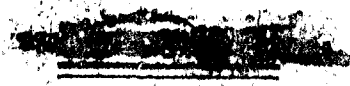


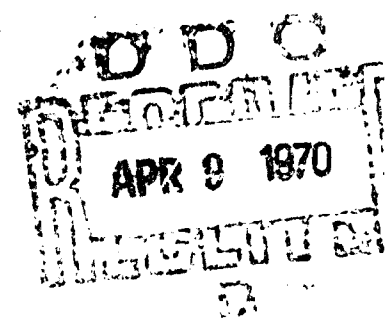
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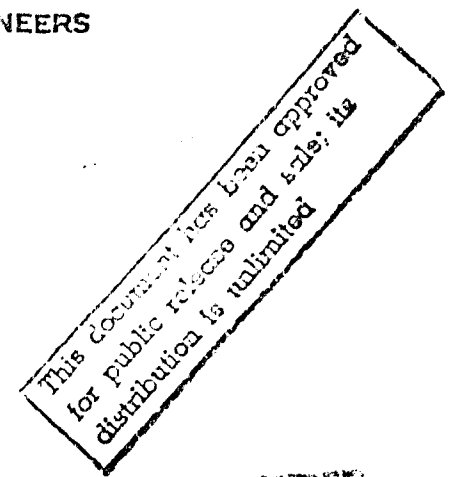
COMPREHENSIVE REPORT
INVESTIGATION OF MILITARY CONSTRUCTION
IN ARCTIC AND SUBARCTIC REGIONS
1945-1948

APPENDIX III
DESIGN AND CONSTRUCTION STUDIES
AT FAIRBANKS RESEARCH AREA



PREPARED BY
ST. PAUL DISTRICT
CORPS OF ENGINEERS
FOR
OFFICE OF THE CHIEF OF ENGINEERS
AIRFIELDS BRANCH
ENGINEERING DIVISION
MILITARY CONSTRUCTION

JUNE 1950



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COMPREHENSIVE REPORT
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APPENDIX III
DESIGN AND CONSTRUCTION STUDIES
AT FAIRBANKS RESEARCH AREA

I Introduction

1. **PURPOSE.** The Fairbanks Research Area was constructed for the purpose of providing an opportunity to observe various types of structures erected on permafrost under conditions that would be known and recorded from the beginning to the conclusion of operations.

2. **GENERAL CONDITIONS.** The Research Area is located about 2-1/2 miles northeast of Fairbanks, Alaska. See Plate III-1 and Figure III-1. The terrain on which it is built is characterized by a comparatively smooth gentle slope which generally provides good surface drainage. Geologically, the Research Area is located on the lower colluvial slopes between the valley fill and the upper colluvial slopes of the rock upland known as Birch Hill. The mean annual temperature at Fairbanks is about 26° F with extremes of +88° F and -58° F. The total annual precipitation is about 12 in., including the annual snowfall of about 4 ft. Mean daily air temperatures, meteorological data and degree-days above and below 32° F during 1947-48 are shown on Plates III-2, III-3, and III-4, respectively. The natural soil underlying the Research Area to a depth of 50 ft is



Figure III-1. General view. Area 1 at top center. Area 2 at left. Area 3 at lower center. July 1948.

principally silt with some fine sand and occasional layers of peat. Analyses of typical soil samples are shown in Table III-1. Under natural conditions, permafrost is encountered at depths of 3 to 6 ft below the surface. The site of the investigation has been divided

TABLE III-1

ANALYSES OF TYPICAL SOIL SAMPLES
FAIRBANKS RESEARCH AREA
AREA NO. 1 -- SECTION B

Sample Number	699	671	677
Depth below ground surface, ft	2.0	6.0	30.0
Physical Constants			
1. All fractions of sample			
Moisture -- field condition, %	74	28	35
Natural density, lb per cu ft	56	91	81
Specific gravity	2.72	2.76	2.70
Natural porosity, %	67	47	52
Natural voids ratio	2.03	.89	1.08
2. Fraction passing no. 40 sieve			
Plasticity index	Non-plastic	Non-plastic	Non-plastic
Mechanical Analysis			
Percent of total sample			
Grain size in in. or US standard sieve no.			
Gravel: no. 10 to 3 in.	0	0	0
Coarse, 1 to 3 in.	-	-	-
Medium, no. 4 to 1 in.	-	-	-
Fine, no. 10 to no. 4	-	-	-
Sand: no. 200 to no. 10	4	10	4
Coarse, no. 40 to no. 10	0	0	0
Fine, no. 200 to no. 40	4	10	4
Silt or clay: Less than no. 200	96	90	96
Effective size in mm	.0067	.023	.0058
Uniformity coefficient	3.4	1.9	5.0
Class (textural)	Silt	Silt	Silt
USED soil group	ML	ML	ML
Miscellaneous Characteristics			
Color	Brown	Gray	Gray
Frozen or unfrozen	Frozen	Frozen	Frozen
Ice lenses present	Yes	Yes	Yes

into three areas as shown on Plate III-1 and in detail for each area on Plates III-5, III-6, and III-7. The general plan is as follows:

- a. Area No. 1 -- ground and pavement surface studies. In this area, the effect of the climatic factors such as air temperature, solar radiation, wind velocity, humidity, cloudiness, and precipitation, on ground temperature, especially where permafrost is present, will be determined under various ground and pavement surface conditions. See Plate III-5 and Figures III-2 to III-5, inclusive (pages 4 and 5),
- b. Area No. 2 -- runway foundation studies. The effect of the construction of runway sections with various pavement types, insulators and base courses on ground temperatures will be determined. See Plate III-6 for the various combinations of fill, insulation and surfaces constructed and under observation. See Figures III-6 through III-15 (pages 9, 10 and 11).
- c. Area No. 3 -- building foundation studies. Buildings have been constructed with various ground exposures and insulators to determine the effect of these types of foundation construction on ground temperatures, especially on the permafrost. Pile foundations will be studied to a limited extent from several piles installed at various depths of embedment in the permafrost to determine whether the freezing and heaving of the active layer will displace the piles. See Plate III-7 and Figures III-16-III-30 (pages 13, 15, 16-19).

3. CONSTRUCTION HISTORY. Construction at the Fairbanks Research Area was started in April 1946 by the Alaskan Department with inspection and supervision by Permafrost Field Office personnel of the St. Paul District Office, CE. Core and churn drilling operations were started in February 1946 and all scheduled borings were completed in June 1946. Continuous samples were taken from all core borings with nearly 100 percent recovery in the frozen silt. Installation of all special equipment, such as ground temperature observation equipment, was made by employees of the Permafrost Field Office. Clearing and stripping operations were started in March and April 1946, respectively. General construction operations were started in April 1946 and completed in the fall of 1947 following changes in plans involving the construction of additional structures. Facilities are provided in each test area for the observation of ground temperatures, groundwater levels, vertical movement of ground surface, pavement surface, or structure, probing to ground frost level, and obtaining soil density samples. Observations of ground temperatures are presently being made at weekly intervals. Groundwater levels are also observed at 2-week intervals during the open season. Vertical movements are observed at one-month intervals. Probing is made at 2-month intervals to check the ground frost levels. Test pits are excavated and soil density tests are made as required at critical places in the foundations. Climatic factors are being observed daily in the area.

II Area No. 1 -- Ground and Pavement Surface Studies

4. DESCRIPTION. Test sections in this area for determining the effect of climatic factors such as air temperature, solar radiation, wind velocity, humidity, cloudiness, and precipitation on ground temperatures include the following four sections, as shown on Plate III-5 and Figures III-2 through III-5.

- a. Section "A" consists of an undisturbed area, 200 ft by 200 ft, with natural

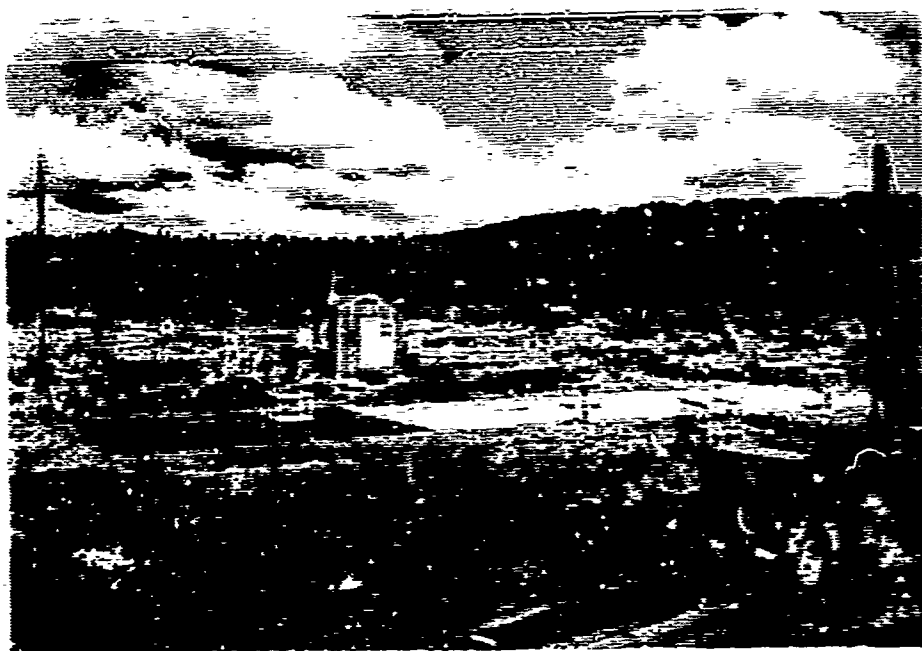


Figure III-2. Area 1, Sections A, B, C and D from left background to foreground. July 1948

cover of vegetation including moss, brush, and spruce trees. See Figure III-5.

b. Section "B" consists of an area, 200 ft by 200 ft, in natural surface condition but with trees and brush removed. See Figure III-4.

c. Section "C" consists of an area, 200 ft by 200 ft, with vegetation removed and with surface soil stripped off to a depth of about 16 in. See Figure III-3.

d. Section "D" consists of an area, 30 ft by 90 ft, stripped to a depth of 12 in. and backfilled with sand to the original ground line and paved with 6 in. of concrete. One-third of the surface area of the pavement is colored black, one-third is colored white, and the remaining third is an uncolored concrete surface. A temperature equipment test area has also been established in Area No. 1 for the comparison of various types of equipment and installations for measuring ground temperatures. The soil in Area No. 1, as is the case throughout the Research Area, is composed of silt and peat. See Plate III-8 and Figure III-2.

5. INSTALLATION OF OBSERVATIONAL FACILITIES. Facilities are provided in Area No. 1 for the observation of ground temperatures, groundwater levels, vertical movement of ground surface, depth to permafrost by probing, and the taking of soil samples for natural moisture and density determinations. The location of each of these facilities is shown on Plate III-5. Ground temperatures are observed at intervals to a depth of 30 ft by means of thermocouples placed in oil-filled 3/4-in. steel pipe or directly in the ground. Continuous temperature records are made in each of Sections DB, DN, and DW with Leeds and Northrup "Micromax" recording equipment using resistance thermometers placed at depths varying from the pavement surface to 6 ft below the pavement surface. Vertical movement observation points in unsurfaced areas consist of 3/4-in. steel rod, 7-3/4 in. long, anchored by welding to a slotted 1/2-in. steel plate one foot square. In concrete paved areas, they consist of 1/2- by 2-in. bolts grouted into the pavement flush with the surface. In asphalt surfacing and gravel surfacing in Area No. 2, they consist of 25d and 80d nails, respectively, driven so that the head just protrudes above the surface. Groundwater elevations are observed by means of a graduated rod in groundwater wells consisting of 2-in. steel pipe casings fitted with 80 mesh well points. Vertical movement points and groundwater wells are similar throughout the Research Area. Probing is accomplished by means of a 1/4-in. or 1/2-in. steel rod and, if results appear indefinite, they are supplemented by borings with a 1-1/2-in. soil auger. During October 1947, a temperature equipment test area was established in Area No. 1 to test various types of installations for the measurement of ground temperatures. The equipment tested included



Figure III-3. Area 1, Section C
Close-up of stripped surface.
September 1946



Figure III-4. Area 1, Section B
Close-up of cleared surface.
September 1946



Figure III-5. Area 1, Section A. Close-up of undisturbed natural
cover. October 1946

thermocouples, resistance thermometers, and mercury thermometers. Nine installations were made; of which six included thermocouples, two included resistance thermometers, and one included mercury thermometers. The thermocouples were installed in direct contact with the ground, in steel pipes filled with oil, and in steel pipes filled with sand.

6. **GROUND TEMPERATURE OBSERVATIONS.** The period of record for observations of ground temperatures in Sections A, B and C, which represent natural vegetative cover, a cleared area, and a stripped area, respectively, extends from July 1946 to 31 October 1948. Observations were made daily from the beginning date until March 1947 and weekly from that date. The change in schedule was made when it became apparent that the rate of change of ground temperatures was not great enough to make daily readings necessary. For Section D in which the effect of different surface colors in concrete pavement on underlying ground temperatures is to be observed, thermocouple installations were not made until late in the fall of 1947. Ground temperatures in hole D, located beside Section D, have been periodically observed from 11 November 1946. Continuous records of ground temperatures have been made in each of the 3 colored sections in Section D since about 20 June 1948. These records are of short duration but voluminous and have not been completely analyzed. Plates III-9 to III-17, inclusive, show ground isotherms at the centers of Sections A, B, and C, for the period July 1946 to 31 October 1948. Plates III-18 to III-20, inclusive, show similar information for Sections DN, DB, and DW for the period 1 October 1947 to 31 October 1948.

7. **GROUNDWATER OBSERVATIONS.** Groundwater levels are observed at 6 points in Area No. 1. Observations are possible only during the period from May to October, inclusive, because the wells are frozen during the remainder of the year. Present data cover the periods from 8 June to 25 September 1946, from 27 May to 29 October 1947, and from 26 May to 27 October 1948, at weekly intervals.

8. **VERTICAL MOVEMENT OBSERVATIONS.** Vertical movement was observed in Area No. 1 at 9 points, to determine the extent of heave and settlement of the ground surface and to compare this movement as it occurs under varying conditions of vegetative surface cover. Three points are located in each of Sections A, B and C. Twenty observations were taken at each point at intervals during the period from 22 November 1946 to 2 October 1948. Data are plotted on Plates III-10, 11, 13, 14 and 16 to 20, inclusive.

9. **DETERMINATION OF NATURAL MOISTURE CONTENT AND DRY DENSITY OF SOILS.** In order to obtain undisturbed soil samples for the determination of natural moisture content and dry field density, test pits were dug in Sections A, B, and C of Area No. 1 on 23 and 24 September 1947 and 4 September 1948. Results of natural moisture tests on samples obtained from the initial core borings and a typical test pit in Section B are shown on Plate III-21.

10. **APPLICATION OF PROBING METHODS COMPARED TO GROUND TEMPERATURE DATA.** Probing for the determination of the depth to the permafrost table and the frost table are taken periodically at 9 frost observation points in Area No. 1, 10 frost observation points in Area No. 2, and 3 frost observation points in Area No. 3, as shown on Plates III-5, III-6, and III-7, respectively. Observations have been taken at intervals of two months since 17 October 1946. All probe points in Areas Nos. 1 and 3, and 4 of the 10 points in Area No. 2 are located adjacent to ground temperature holes, so that a direct comparison of data obtained by the two methods can be made. Results were found to be quite similar for all points. Therefore, points A, B, and C in Area No. 1 have been selected as typical for purposes of this discussion. These points are located at the centers

of Sections A, B, and C. Plates III-22 and III-23 show the 0°C isotherm as plotted from ground temperature observations and the locations of permafrost and seasonal frost levels as plotted from probing data for points A, B, and C during the period of record. It is noted that differences up to two feet between the two methods occurred during 1948. In this connection it has been observed that, after seasonal frost begins to penetrate from the surface in the fall, ground temperatures tend to appear somewhat erratic, probably due to the rapidly changing temperature gradients. By the thermocouple installations, positive location of the 0°C isotherm can be determined only when it coincides with the depth to one of the thermocouples. The plotting of isotherms between these depths is a matter of interpolation which tends to reduce the accuracy of plotting, especially when temperature gradients are changing rapidly. For this reason, it is not considered proper to regard all variation in results between the two methods as being due to inaccuracies in probing. The accuracy of the ground temperature method would be greatly increased by spacing the thermocouples closer together. For future installations, it is considered advisable to place thermocouples at one-foot intervals to the probable depth of thaw. The reliability of probing data is affected by the nature of the soil involved. In fine-grained soils which offer little resistance to the probing rod when thawed and wet, the point of contact with a frozen layer is quite definite. Sand has a tendency to bind the rod, which makes it harder to drive, so that the point of contact with frozen soil is not as easily detected. It has been noted from past experience, however, that the resistance of the rod to twisting affords a criterion. A torque is applied to the rod by means of a pipe wrench when it is suspected that the point has entered frozen soil. If the point is embedded in frozen ground, it will be held rigidly and the upper end of the rod will spring back to its original position upon release. If the point is still in thawed soil, the entire rod will turn and much less spring will be noted upon release. Probing in gravel is unreliable, and in coarse gravel it is impossible due to obstruction by the larger stones. When probing results appear indefinite, they may often be checked by borings with soil augers. In general, the two methods when applied in fine-grained soils give results which compare closely enough so that neither method can be discarded as unsatisfactory for observations at depths up to 10 ft. Valuable supplementary data can be obtained by probing, especially during the "freeze-up" season in the fall. The greatest value of probing is for reconnaissance and for investigation of sites where continuous records over long periods of time are not contemplated. Recent field tests indicate that probing by pneumatic drills is more effective in frozen ground than hand-driven probe rods.

III Area No. 2 -- Runway Foundation Studies

11. DESCRIPTION. Test sections in Area No. 2 were constructed to determine the effect of various pavement types, insulators, and base courses on ground temperatures. They include the 26 runway test sections listed in Table III-2 and shown on Plate III-6 and Figures III-6 and III-7.

12. INSTALLATION OF OBSERVATIONAL FACILITIES. Facilities are provided in Area No. 2 for observation of ground temperatures, groundwater levels, vertical movement of ground and test section surfaces, depth to permafrost by probing, and the taking of soil samples for moisture determinations. The location of each of these facilities is shown on Plate III-6. Sampling sleeves provided in paved test sections, for taking soil samples under pavement, have proved to be ineffective for sampling through the gravel backfill to the subgrade due to the caving action of the gravel. For this reason, the sampling sleeves have not been used. As an alternate method, test pits have been dug adjacent to the edge of

pavement in certain representative sections. Typical moisture-density relations for the sand and gravel fills of Areas Nos. 2 and 3 are shown in Plate III-24. Plate III-25 shows grain-size accumulation curves which are typical of the backfill materials in Areas Nos. 2 and 3. Observational facilities in Area No. 2 are, in general, similar in construction to those installed for similar purposes in Area No. 1, as described in this report.

TABLE III-2
FAIRBANKS RESEARCH AREA NO. 2
RUNWAY TEST SECTIONS

Sec. No.	Con-struction Season	Surface	Base Course		Total Thickness, ft
			Type	Insulation	
RN-1	Summer	5-in. Asphalt	Sand and Gravel	None	12
RN-2	Summer	5-in. Asphalt	Sand and Gravel	None	8
RN-3	Summer	Gravel	Sand and Gravel	None	4
RN-4	Summer	5-in. Asphalt	Sand and Gravel	None	4
RN-5	Summer	5-in. Asphalt	Sand and Gravel	3-in. P.C. Foamglas	4
RN-6	Summer	5-in. Asphalt	Sand and Gravel	6-in. P.C. Foamglas	4
RN-7	Summer	5-in. Asphalt	Sand and Gravel	6-in. Cell Concrete	4
RN-8	Summer	5-in. Asphalt	Sand and Gravel	6-in. Cell Concrete (low density)	4
RN-9	Summer	5-in. Asphalt	Sand and Gravel	12-in. Cell Concrete	4
RN-10	Winter	5-in. Asphalt	Sand and Gravel	6-in. Cell Concrete (low density)	4
RN-11	Winter	5-in. Asphalt	Sand and Gravel	6-in. P.C. Foamglas	4
RN-12	Summer	5-in. Asphalt	Sand and Gravel	None	2
RN-13	Summer	6-in. Concrete	Sand and Gravel	None	2
RN-14	Summer	5-in. Asphalt	Sand and Gravel	None	10
RN-15	Summer	5-in. Asphalt	Sand and Gravel	None	6
RN-16	Summer	6-in. Concrete	Sand and Gravel	None	6
RN-17	Summer	6-in. Concrete	Sand and Gravel	None	4
RN-18	Summer	12-in. Concrete	Sand and Gravel	None	4
RN-19	Summer	5-in. Asphalt	Sand and Gravel	6-in. Compacted Spruce Logs and Branches	4
RN-20	Summer	5-in. Asphalt	Sand and Gravel	6-in. Compacted Moss	4
RN-21	Summer	5-in. Asphalt	Sand and Gravel	6-in. Zonolite Concrete	4
RN-22	Summer	5-in. Asphalt	Sand	None	4
RN-23	Summer	6-in. Concrete	Sand	None	4
RN-24	Spring	6-in. Concrete	Sand and Gravel	None	4
RN-25	Spring	5-in. Asphalt	Sand and Gravel	None	4
RN-26	Spring	Gravel	Sand and Gravel	None	4

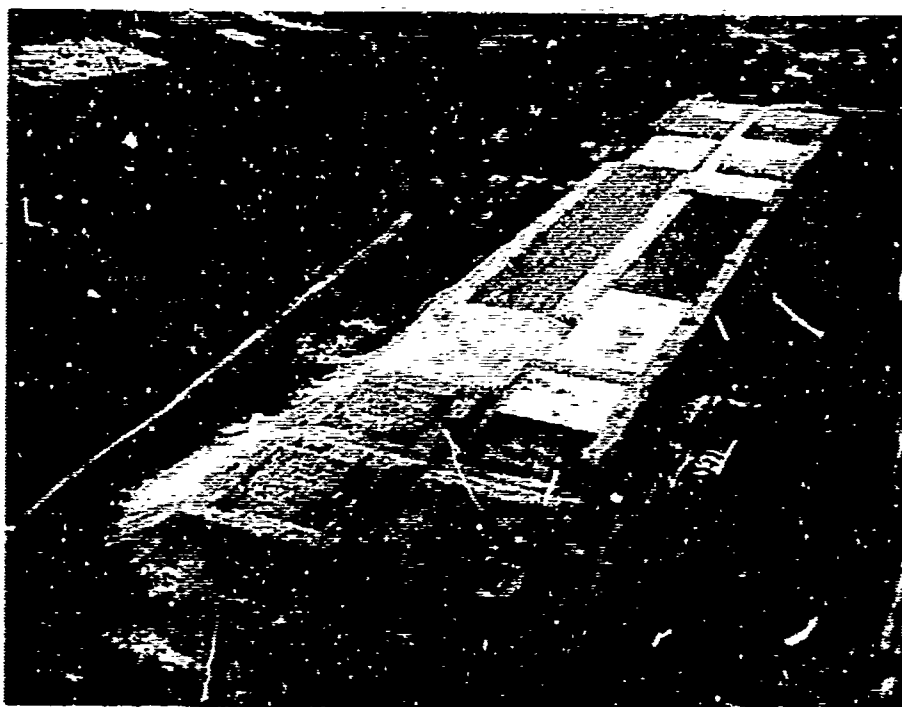


Figure III-6. Area 2 General view looking south, Area 2 in foreground, Area 3 in upper right background. October 1946

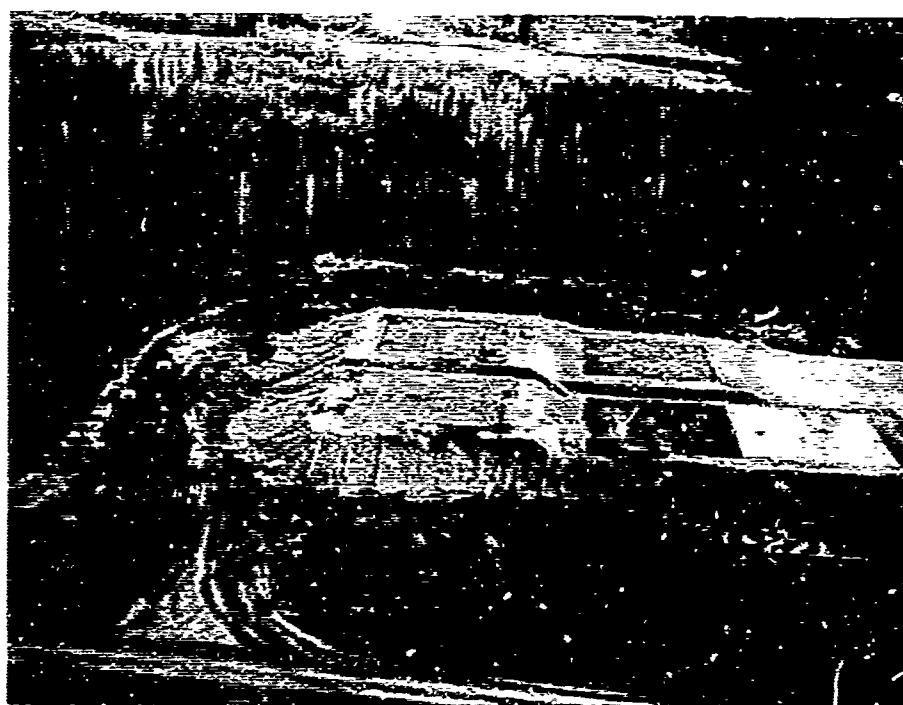


Figure III-7. Area 2 Close-up of thick gravel fills, Sections RN-1, RN-2, RN-14, RN-15 and RN-16. October 1946

Figure III-8. Area 2
Stripping thawed surface
material with dragline
July 1946



Figure III-9. Area 2
Hand drainage of subgrade
during spring thaw.
June 1946

Figure III-10. Area 2
Junction between Sections
RN-19 at left and RN-20 at
right. Note spruce boughs
and moss insulating layers
prior to compaction by
placement of gravel fill.
July 1946

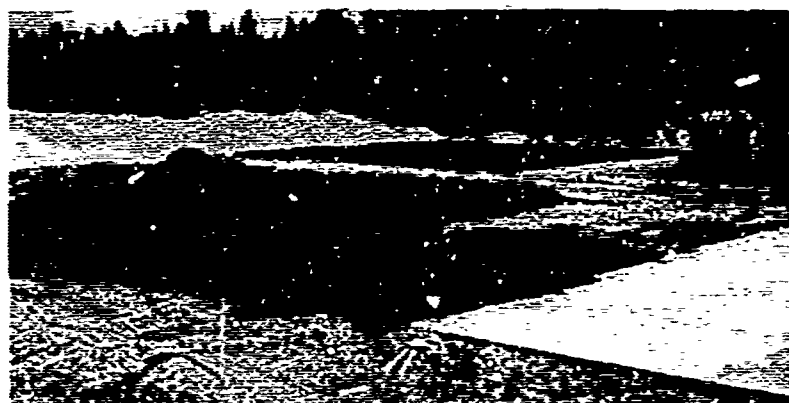


Figure III-11. Area 2
Insulating layers in place.
Foamglas in Section RN-5,
near center; spruce boughs
in Section RN-19, left fore-
ground; moss in Section
RN-20, middle foreground; and
zonolite concrete in Section
RN-21, right foreground.
July 1946



Figure III-12. Area 2
Placing cell concrete insulation
in Section RN-7.
July 1946



Figure III-13. Area 2
Base course construction. Due to
unstable condition of subbase during
spring construction, backfill was
extended by end dumping. July 1946



Figure III-14. Area 2
Six-inch layer of Foamglas insulation
consisting of two courses each 3 in.
thick sealed with tar. March 1947

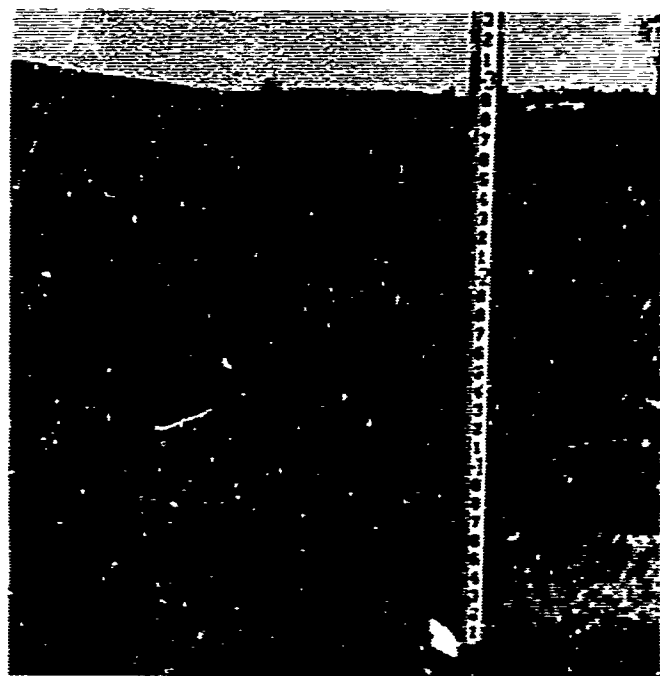


Figure III-15. Area 2
Sectional view of Section RN-9. Note
insulating layer of cell concrete
exposed below gravel fill. October 1946

13. GROUND TEMPERATURE OBSERVATIONS. Ground temperatures are observed in Area No. 2 at the center of each of the 26 test sections and at 4 points on the center line of the area. Observations were started at holes R-201 and R-204 in February of 1946, at holes RN-10 and RN-11 in July of 1947, and at all other holes in Area No. 2 in September or October 1946, so that, in general, a record of observations covering a period of two years is available. Readings were taken daily until April 1947 and weekly from that date to 31 October 1948. Ground frost data collected in Area No. 2 have been summarized and are presented graphically in Plates III-25 to III-36, inclusive, which show the position of the 0°C isotherm under each section at various times during the period of record. On the basis of observations and tests performed, it is considered that natural variables such as climatic conditions, moisture content, and density of soil remain quite uniform throughout the small area involved in Area No. 2, so that any differences in heat flow observed may be considered to result from the differences in design of the various sections. Since each section represents an individual test, the 0°C isotherm has been plotted separately for each section as a straight line rather than attempting to draw a continuous isotherm for the full length of the area. Ground temperatures are observed in each section at only one point which is located at the center of the section in each case. Holes R-201 and R-204, located in natural ground on the center line at either end of Area No. 2, provide a comparison with ground temperatures under natural conditions. The position of the 0°C isotherm for these holes is given by note on each of the sectional drawings.

14. GROUNDWATER OBSERVATIONS. Groundwater levels are observed at weekly intervals at 6 points in Area No. 2. Present data are for periods from 8 June to 25 September 1946, from 27 May to 29 October 1947, and from 26 May to 27 October 1948.

15. NATURAL MOISTURE CONTENT AND DRY DENSITY OF BACKFILL AND SUBGRADE. Test pits were dug during late September and early October 1947 adjacent to the surfacing in Sections RN-2, RN-6, RN-9, RN-18 and RN-21 in order to secure undisturbed samples for the determination of natural moisture content and dry field density of soils underlying the test sections. The test pits were located and dug in such a manner as to make it possible to secure samples as far under the sections as possible without disturbing the surfacing. The data obtained were used in making calculations of depth of thaw. For all samples from the five test pits in Area No. 2, the moisture content of the gravel fill ranged from 1.9 to 6.0 percent, with the greater number falling between the limits of 2.5 to 4.5 percent. The dry density of the gravel varied from 133 to 144, and averaged 141 lb per cu ft. The highest density of the natural soil was encountered immediately beneath the gravel fill in each case. This condition is apparently due to compaction by overlying gravel fill and is noted to depths of from 1.5 to 2.0 ft below the bottom of the gravel. The average dry density in this zone for all test pits was 95.1 lb per cu ft, the maximum density of 108.2 occurring in Section RN-9. Below this zone, the density decreased and averaged 78.7 lb per cu ft for all samples taken. The average moisture content of the gravel fill for all test pits was found to be 3 percent, indicating that the material is quite free draining. A marked increase in moisture content is noted upon entering the natural soil subgrade, but no definite trend toward increase or decrease of moisture content can be detected at increasing depths into the subgrade. Two samples, one in Section RN-2 with a moisture content of 57.8 percent and one in Section RN-18 with a moisture content of 81.9 percent, are noted to depart radically from the general average. All other samples range between 22.5 and 54.0 percent and average 37.9 percent. Peat in varying stages of decomposition has been noted to have varying qualities of absorption due to the cellular structure of organic material which may hold free water within the cells. Both of the samples noted above for widely divergent moisture content contained peat which might explain this condition. It is noted that the cell concrete insulating layer in Section RN-9 had a moisture

content of 40.4 percent, and the concrete layer in Section RN-21 had a moisture content of 74.0 percent. Both are embedded in gravel having a moisture content of about 4.6 percent.

16. VERTICAL MOVEMENT OF GROUND AND TEST SECTION SURFACES. Vertical movement was observed in Area No. 2 at three points in natural ground along the eastward side of the area and at five points on the surface of each test section to compare the extent of vertical movement which may be expected in several types of construction and in the natural ground surface. The various types of construction studied in the test sections are also shown on Plate III-6. Level observations were made of each point at intervals during the period from 23 November 1946 to 10 November 1948.

IV Area No. 3.-- Building and Piling Studies

17. DESCRIPTION. Test facilities in this area for testing the effect on ground temperatures of buildings with various types of foundation construction, and the stability of piles placed to varying depths into permafrost, include the buildings shown in Table III-3, on Plates III-7 and III-37, and in Figure III-16. Buildings 1 to 8, inclusive, are 16 by 16 ft and are used as test buildings only. Buildings 9 and 10, 32 ft square, are utilized as residences and Building 11, also 32 ft square, as a garage, in addition to serving as test buildings. Test piles to determine the depth necessary to prevent seasonal heaving and settlement are placed at varying depths to penetrate permafrost to distances 0, 1/2, 1, 1-1/2, 2, 2-1/2, and 3 times the thickness of the active layer. See Figures III-17 through III-26.

18. INSTALLATION OF OBSERVATIONAL FACILITIES. Facilities are provided in Area No. 3 for the observation of ground temperatures, groundwater levels, depth to permafrost by probing, the taking of soil samples for moisture determinations, and vertical movement of buildings, piles and ground surface. The locations of these facilities are shown on Plate III-7. Vertical movement observation points for buildings consist of 20d spikes driven into the sills at each corner. For piles, the observation point consists of a 20d spike driven into the pile at about 14 in. above the normal ground surface. Ground temperatures pertinent to the buildings are observed at switch panels within the buildings in lieu of switches in junction boxes used for taking readings at other thermocouple installations. The spacing and depths of each thermocouple installation, together with other information pertinent to the installations, are shown in Plate III-37. During September 1947, swellometers to observe the

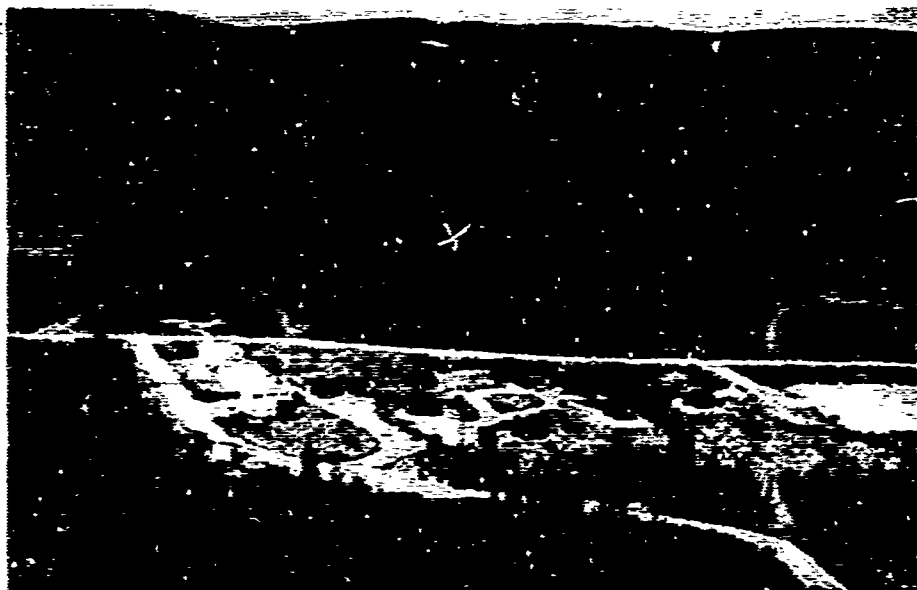


Figure III-16. Area 3
General view of test buildings. July 1948

TABLE III-3

FAIRBANKS RESEARCH AREA NO. 3
TEST BUILDINGS 1 to 11

Bldg. No.	Type of Floor	Foundation		Insulation in Foundation
		Material	Thickness	
1	Concrete	Sand and Gravel	4 ft	None
2	Insulated wood	Sand and Gravel	4 ft	None
3	Insulated wood	Sand and Gravel	2 ft	None
4	Insulated wood	Sand and Gravel	6 ft	None
5	Insulated wood	Sand and Gravel	4 ft	6-in. Cell Con- crete
6		Post and Pads	Beams 2 ft above ground surface, with- out skirting	None
7	Insulated wood	Post and Pads	Beams 2 ft above ground surface, with skirting	None
8	Insulated wood	Mud Sills	On the natural ground surface	None
9	Insulated wood on upper con- crete slab	2 Concrete slabs separ- ated by concrete piers 3 ft high on 5-ft gravel fill		None
10	Insulated wood with air space	Piling approx- imately 4 ft above natural ground		None
11	Concrete slab containing layer of continuous hollow tile open to air at both ends	Gravel Fill	5 ft	None



Figure III-17. Area 3
Foundation of Test Building
No. 1 with 4-in. concrete floor
slab in place. Building No. 2 in
background. September 1946

Figure III-18. Area 3
Test Building No. 5 with wood
sills placed directly on gravel
fill and wood floor completed.
Typical construction for Build-
ings Nos. 2, 3, 4 and 5.
September 1946

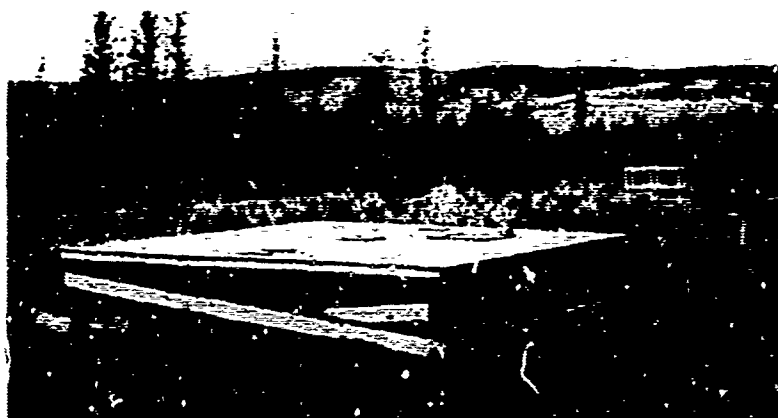


Figure III-19. Area 3
Test Building No. 6 with wood
floor completed. Not to be
skirted. September 1946

Figure III-20. Area 3
Test Building No. 7 with floor
completed and skirting in place
September 1946



Figure III-21. Area 3
Test Building No. 8 with pads and
sills in place on ground surface.
August 1946



Figure III-22. Area 3
Rock wool insulation in floor of
test building. Typical construction
for Buildings Nos. 2, 3, 5, 6 and 7.
July 1946

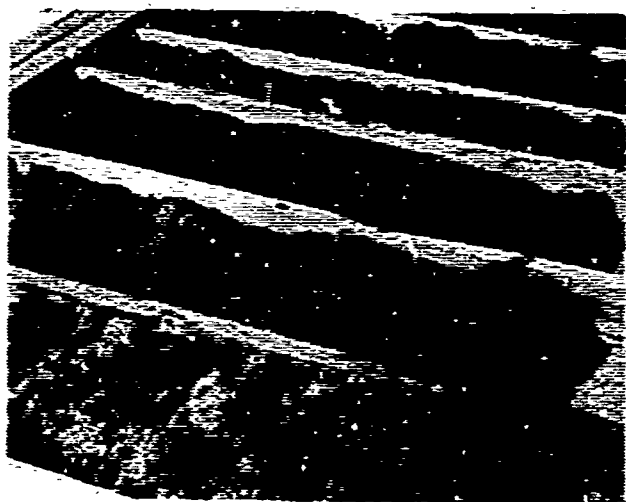
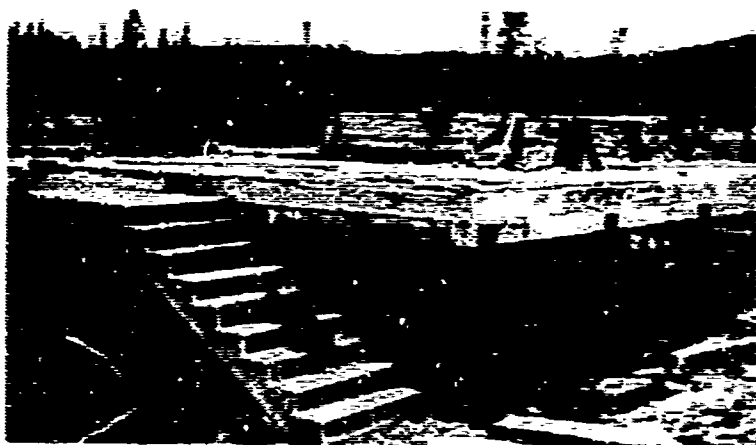


Figure III-23. Area 3
General view of Buildings 9, 10
and 11 from rear to foreground.
July 1946



Figure III-24. Area 3
Test Building No. 10 with
rough floor in place. Note
composite wooden beams and
treated wood piling supports.
August 1947



upward vertical movement of the soil at various depths below the ground surface, were installed in Area No. 3 at three locations as shown on Plate III-7. Essentially, a swellometer consists of eight wood rods, 1-in. square and of varying lengths, each of which is firmly fastened to a hollow prismatic base one foot long and a ninth rod set in a solid base. See Plate III-38.

The rods are assembled and lubricated in such a way that they may slide freely in a vertical direction and move independently of each other. The base of each rod is left unlubricated and may be roughened to promote a good bond with the surrounding soil so that the movement of each rod is determined by the movement of the soil at the depth to which the base is placed. The movement of each rod is observed at its upper end which protrudes above the ground surface.

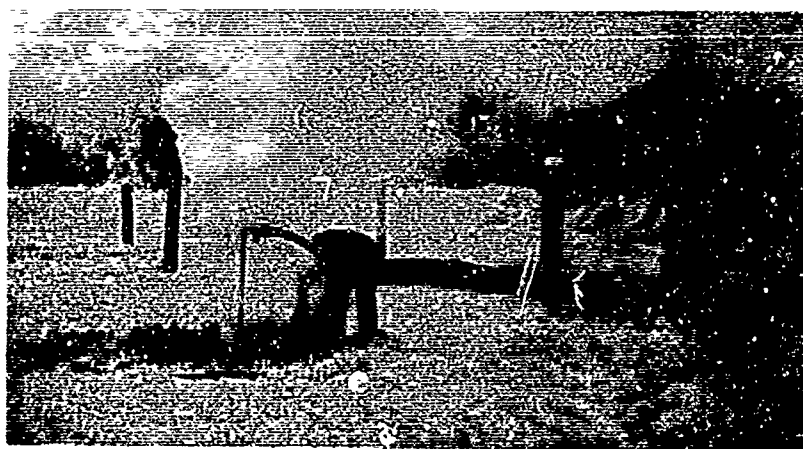


Figure III-25. Area 3

General view of test pile section showing a steam point in operation. The Herman Nelson heater on the right was used to warm up a dragline engine.

December 1946



Figure III-26. Area 3

Placing a test pile into a thawed hole. December 1946

19. GROUND TEMPERATURE OBSERVATIONS. Ground temperatures are observed weekly at 3 points in natural ground and under Test Buildings 1 to 8 in Area No. 3 at points indicated on Plate III-7. Beginning on 14 October 1946, these buildings were heated to maintain a continuous temperature of $+15^{\circ}\text{C}$ until 1 January 1947, at which time it had become apparent that no thaw would be caused under certain of the buildings by this temperature and the inside temperature was raised to $+20^{\circ}\text{C}$. Ground isotherms have been plotted for each of the buildings to show a north-south section at the center line of the building. Plates III-39 to III-49, inclusive, show these data plotted at various times during the period of record.

20. GROUNDWATER OBSERVATIONS. Groundwater levels are observed weekly at 4 points in Area No. 3, as shown on Plate III-7. Present data cover periods from 8 June to 25 September 1946, from 27 May to 29 October 1947, and from 26 May 1948 to 3 November 1948. It may be noted on the plates showing ground temperature observations during the winter months that thawed zones which had about the same shape each year developed under certain buildings such as 7 and 8, and, in some cases, increased in size during the season. While no definite proof is available, it is believed that these thawed zones carry groundwater when soil layers just under the surface are frozen. The movement of water through

cracks, crevices and polygon channels tends to keep the ground from freezing.

21. NATURAL MOISTURE OF SUBGRADE. In order to secure information concerning the natural moisture content of soils in Area No. 3, auger borings were made in late September 1947 from the surface down to permafrost at points B-101, B-103 and B-104 in natural ground as shown on Plate III-7, and from the top of subgrade down to permafrost under each of Buildings 1 to 8 inclusive. For buildings on gravel fill, a 20-in. metal pipe was placed to extend from the top of fill through the fill to the top of subgrade to permit access to the subgrade for auger borings. Typical moisture data for the building area are shown in Plate III-50.



Figure III-27. Area 3
Placing fabricated steel vertical
movement observation point in the
ground adjacent to a test pile.
February 1947

shown on Plate III-7. The objective of these observations is to determine the vertical movement of the natural ground surface, to compare the vertical movement of buildings on various types of foundations, and to test the stability of piles placed to varying depths into permafrost. Twenty-one observations on points B-101, B-102, B-104 and B-105 in natural ground were made at intervals between 22 November 1946 and 8 November 1948, fifteen to twenty-one observations were made on test buildings between 8 November 1946 and 8 November 1948, and twenty observations were made on and beside the test piles between 14 January 1947 and 6 November 1948. Observations of ground temperatures and settlement of the Navy monotube test piles were made as required.

23. NAVY MONOTUBE TEST PILING. In order to assist the Bureau of Yards and Docks, Department of the Navy, in the testing of monotube steel piling placed in frozen ground, space was made available for a test installation of this type of piling. The installation was performed by personnel and equipment of the Post Engineer, Ladd Air Force Base, under supervision of the Department of the Navy. The Permafrost Field Office assisted in the

22. VERTICAL MOVEMENT OF GROUND AND TEST STRUCTURES. Vertical movement is observed in Area No. 3 at four points in natural ground, on the sills at each of the four corners of Test Buildings 1 to 11, on 17 piles supporting Building 10, at 15 points on the concrete floor of Building 11, on each of 24 test piles, and at a point on the ground beside each of the 24 test piles. Locations of the observation points are

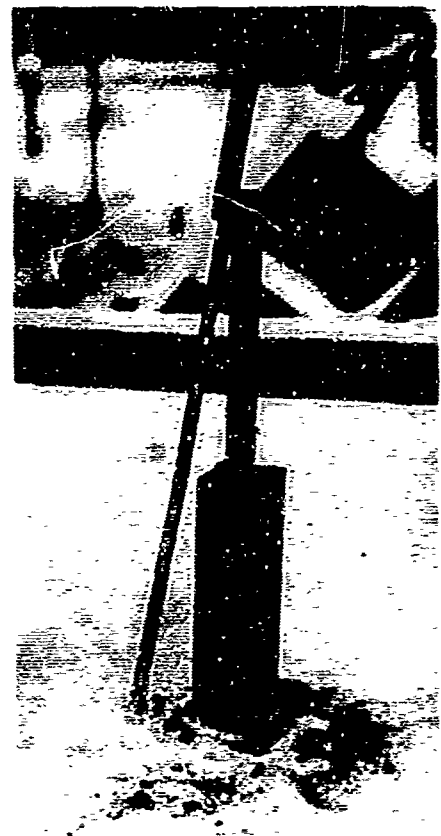


Figure III-28. Area 3
One of the 3 swellometers
installed in the test pile
area. December 1947



Figure III-29. Area 3
Detail of monotube Navy test pile
and temperature pipe.
November 1947

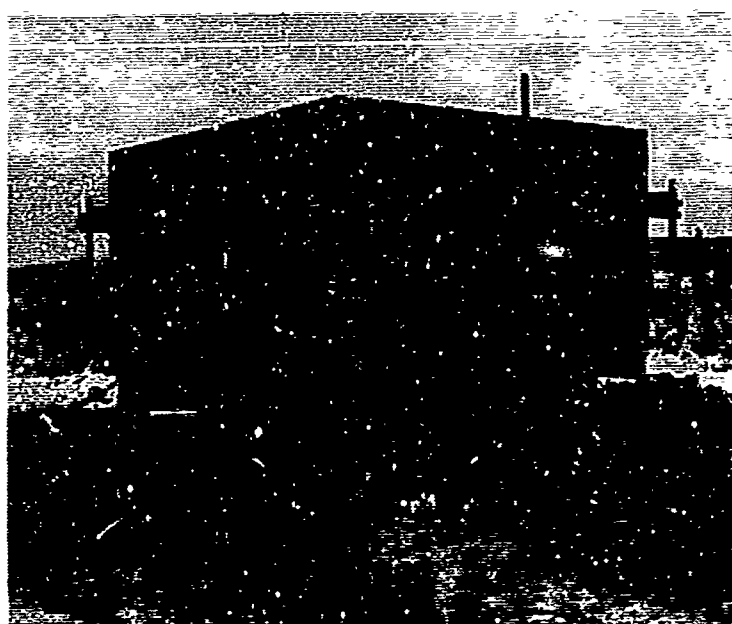


Figure III-30. Area 3
Loading platform for monotube test
piling. December 1947

planning and field supervision of the work and thermocouple assemblies were made up by the St. Paul District for the observation of ground temperatures adjacent to the piles. The installation consists of one 60-ft and one 40-ft pile which jointly support a bin in which a gravel load is placed. The piles were placed in holes preformed by steam thawing, during the period 13 to 20 November 1947. The loading bin with supporting steel members was completed on 12 December 1947. A total load of 114 tons was placed on the piles in December 1947. Installations for observing vertical movement of the ground surface and for observing groundwater levels in Area No. 3 are similar to those in Area No. 1, as described previously in this report. The following quotation is the report of 3 June 1948 from the Field Operations Branch of the Permafrost Division, St. Paul District, Corps of Engineers to the Officer in Charge, Arctic Test Section, U. S. Navy, Fairbanks, Alaska concerning test loading of monotube piles:

"1. Reference is made to previous correspondence on the above subject describing the installation of two Monotube Test Piles in the Permafrost Research Area near Fairbanks, Alaska by the Post Engineer, Ladd Air Force Base and the Permafrost Division in cooperation with your organization. The following is a summary of the sequence of operations and observations obtained in connection with the loading of these piles. The data is assembled on the inclosed drawing 'Vertical Movement and Ground Temperature Chart-Monotube Test Piles'. (Plate III-51 of this report.)

"2. Two piles, one 40 feet and the other 60 feet long, were placed in holes preformed by steam thawing, during the period 13 to 20 November 1947. On 12 December 1947, after the test piles had been installed for 23 days, the construction of the bin to carry the test load of gravel was completed, with the bin and the supporting steel beams in place atop the two piles. Calculated total weight of the bin and supporting members was 7.11 tons. At the time of

completion of the bin, ground temperatures adjacent to both piles were returning to normal after having been raised during the installation of the piles by steam jetting. Temperatures above zero degrees Centigrade were recorded between 2 and 45 feet at the 60-foot pile and between 4 and 21 feet at the 40-foot pile. Total settlement up to this date was 0.079 foot for the 60-foot pile and 0.031 foot for the 40-foot pile. There was no appreciable change in elevation following erection of the bin. By 19 December 1947, the piling appeared to be stabilized, with no settlement observed between 12 December and 19 December 1947. Temperatures of 0° C. or colder were recorded for the lower 30 feet of the 60-foot pile and for the entire depth of the 40-foot pile. On 20 December 1947, approximately 107 tons of gravel were placed in the bin, bringing the total load to 114 tons. Upon release of the screw jacks in place at the four corners to prevent tipping of the bin, immediate settlement was observed, and by 22 December 1947 the 60 and 40-foot piles had settled 0.126 foot and 0.106 foot respectively and were exerting pressure on the jacks at the corners of the bin. The jacks were lowered again on 22 December and an immediate settlement of 0.009 foot and 0.012 foot on the 60-foot and 40-foot piles respectively was observed. This rapid settlement continued until 26 December when the total observed settlement since loading was 0.214 foot on the 60-foot pile and 0.197 foot on the 40-foot pile. At this point, ground temperatures had not stabilized and normal cooling was continuing. After the jacks were released on 22 December 1947, they were not disturbed until 16 March 1948. Between these dates, a portion of the load was transferred to the jacks, through settlement of the piles, at an indeterminate rate. Between 26 December 1947 and 16 January 1948, total settlement was 0.023 foot and 0.018 foot on the 60-foot and 40-foot piles respectively, and ground temperatures continued a gradual decline on both piles until they reached a minimum on 10 January 1948, after which time a gradual daily increase in temperatures was noted. No temperatures above 0° C. were recorded between 30 December 1947 and 21 January 1948 at the 60-foot pile and at the 40-foot pile between 20 December 1947 and 21 January 1948. Between 16 and 30 January, accelerated settlement accompanied by a general rise in ground temperatures was observed at both piles and resulted in the distribution of a greater proportion of the load to the jacks at the corners. Total settlement between these dates was 0.065 foot and 0.053 foot at the 60-foot and 40-foot piles respectively. Elevations of both piles remained virtually constant during the period 30 January 1948 to 12 March 1948, although minor temperature variations were noted. The load remained constant and no change was made in the jacks which were carrying an undetermined portion of the load.

"3. Since it was apparent that the two piles could not sustain the load of 114 tons and hydraulic jacks with indicator dials or other suitable means were not available to apply and measure the load in increments, it was decided to remove the major portion of the gravel and to reload in increments one foot in depth over the bin's floor surface and thereby determine the maximum load capable of being supported by the piles. Accordingly, on 15 March 1948, all but 2 feet of the gravel was removed from the bin, decreasing the load to approximately 34 tons. Since 7 March, a gradual lowering in ground temperatures had been noted, and on the date the gravel was removed, the ground temperatures observed were generally +0.1° C., with the exception of the upper and lower extremities of the piles where the

temperatures were below zero. On 16 March, approximately 24 hours after the gravel had been removed, a slight but definite drop in temperature was noted and, in every case, ground temperatures were at zero or minus 0.1°C . throughout the center portions of the piles. No appreciable change in elevation occurred upon decreasing the load. On 16 March 1948, the jacks were lowered for the first time since 22 December 1947; there were no significant changes in temperature or elevation. No portion of the load was supported by the jacks between 16 March and 6 April 1948. On 18 March 1948, the gravel was brought to a depth of 3 feet, increasing the load to 47 tons. No appreciable change in temperature or elevation resulted. Elevations on 19 March showed an equal settlement of 0.008 foot on both piles and no further increments were added until 23 March 1948. Between 20 March and 22 March 1948, a temperature rise from minus 0.1°C . to plus 0.2°C . took place in the central sections of both piles. The small vertical movement observed during the period 19 March to 23 March is likely due to the limitations of accuracy of level observations. On 23 March 1948, an increment of one foot of gravel brought the total load to 60.5 tons. Application of this additional load was accompanied by a slight rise in temperature throughout the length of both piles; there was no observed change in elevation. On 25 March, the gravel was brought to a depth of 5 feet or a total load of 74 tons. No significant changes in elevation or temperature were noted until 23 March when temperature at both piles showed an increase of from 0.1 to 0.3°C . Largest increases were in the central portion of the 40 foot pile. After 26 March, a gradual decrease in temperatures took place in both piles. On 29 March, eight inches of gravel were added bringing the total load to approximately 83 tons. Settlement in the order of 0.004 foot occurred immediately, but both piles remained fairly stable until 1 April when a rapidly accelerating settlement started and continued to its maximum on 12 April when the steel beams came to rest on the ground surface at the south side of the bin and on two jacks which had not been removed from the north side. Temperature observations continued since 12 April indicate a general and progressive rise in ground temperatures. At present, temperatures are above 0°C . throughout the length of both piles.

"4. The observations outlined above indicate that the piles were relatively stable and, for the short period of observation, successfully supported a load of approximately 74 tons. with the application of additional load bringing the surcharge to 82.8 tons, the yield point was reached and progressive accelerating settlement occurred. The bond between the soil material and the pile surface (tangential adfreezing strength) has been computed for 1 April 1948 when the piles were supporting a total load of 82.8 tons; there had been no appreciable movement for three days following application of the final load increment and, with the exception of approximately seven feet on the 40 foot pile, ground temperatures adjacent to the piles were at 0°C . or colder. The quantity of soil moisture which remained thawed at the range of temperature observed adjacent to the piles is indeterminate from the available data, however, for the purpose of calculating tangential adfreezing strength, ground at 0°C . or colder is considered to be frozen.

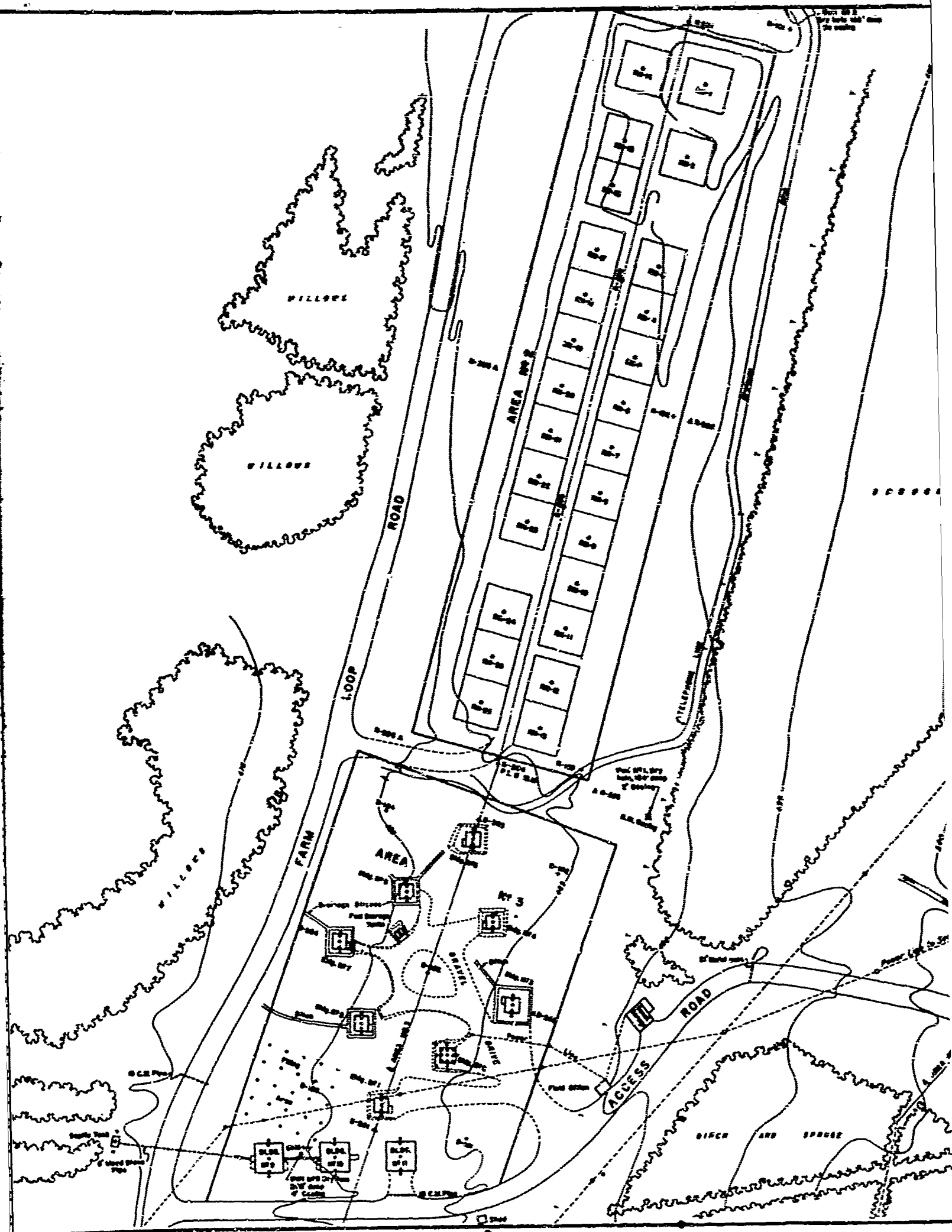
Weight of bin in place	14220 lb.
Weight of 1513.90 cu. ft. gravel at 100 lbs/cu.ft.	151390 lb.
Total Load	165610 lb.

Surface area of 40' pile in frozen ground	89 sq. ft.
Surface area of 60' pile in frozen ground	194 sq. ft.
Total Area	283 sq. ft.

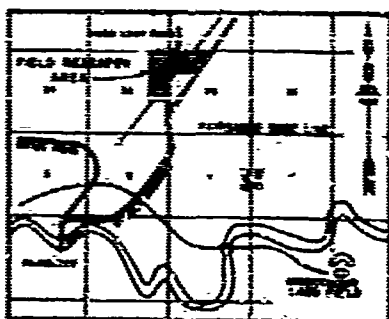
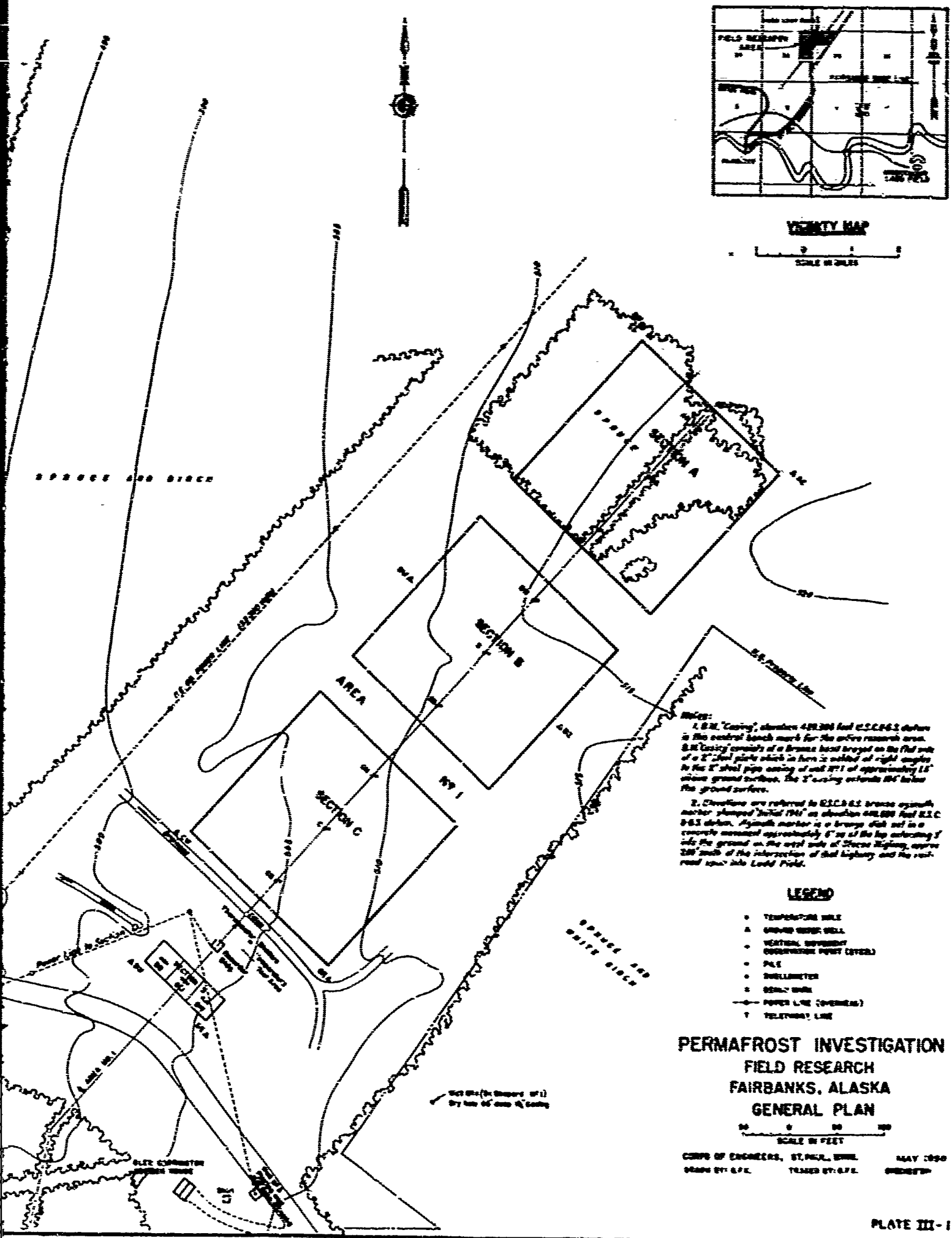
Tangential adfreezing strength	4.06 lb. per sq. in.
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"5. Except for the observation that pile failure and rapid settlement occurred at a specific loading only general conclusions can be drawn from the data collected thus far. The soils underlying the test site are frozen to a depth of approximately 150 feet and, in the natural state, contain large amounts of moisture, mostly in the form of ice which serves to bond the soil particles into a dense mass capable of great resistance to penetration. Piling placed in permafrost must, therefore, be placed in holes preformed by thawing or some other method. If thawed, the thermal regime is disturbed and the soil loses its structure becoming more or less a fluid, depending upon the amount of moisture present. Piling introduced into the ground under these conditions do not develop their full strength until the adjacent soil has refrozen and the thermal regime is re-established. In the vicinity of Fairbanks where the test is being conducted, the temperature of the permafrost at depths below the level of seasonal influence approaches the critical temperature of 0° C. and a considerable time is required for the natural 'reserve of cold' to overcome the effects of heat introduced for the formation of holes into which piles can be placed and to re-establish the thermal regime. Consideration of the inclosed drawing, which depicts graphically the vertical movement of the piles and concurrent ground temperatures, indicates that the pile movement beginning after stabilization of ground temperatures to comparatively normal conditions will generally be accompanied by a rise in temperature which it is thought reflects the thawing of ice forming the surface bond between the pile and the enclosing material. In order to extend and substantiate the data collected to date, it is planned to repeat the above test by reloading the piling with approximately 60 tons and observe their behavior over an extended period of six or more months. Upon completion of this phase of observations, the piles will again be gradually loaded to failure."

A review of this report indicates some doubt as to whether ground around the piling was entirely refrozen at the time of loading. Temperature observation equipment used is not necessarily accurate within 0.3° C and the silt soil in the area can be expected to have 0.1° to 0.2° C depression of the freezing point. Thus, temperatures of -0.4° C do not necessarily indicate that the ground was frozen. This office is now studying the possibility of using pressure cells to indicate ground freezing. The computed tangential adfreezing strength of 4.06 psi is based on the assumption that all of the load was carried by bond between the piling and the adjacent ground and that the ground was frozen. It neglects any point bearing capacity which would be a substantial amount in frozen ground.



A



INSET MAP

SCALE IN MILES

Notes:
1. S.M. "Casing", elevation 449.300 feet U.S.C. 8-63 datum is the central bench mark for the entire research area. S.M. "Casing" consists of a bronze base bored on the flat side of a 2" steel plate which in turn is welded at right angles to the 2" steel pipe casing of wall #1 of approximately 15' above ground surface. The 2" casing extends 80' below the ground surface.
2. Elevations are referred to U.S.C. 8-63 datum against marker stamped "Initial 1941" at elevation 448.800 feet U.S.C. 8-63 datum. Against marker is a bronze disk set in a concrete mound approximately 6" up at the top extending 5' into the ground on the west side of Steese Highway, approx 200' south of the intersection of that highway and the railroad spur into Ladd Field.

LEGEND

- TEMPERATURE WELL
- △ GROUND WATER WELL
- VERTICAL MOVEMENT OBSERVATION POINT COVERED
- PILE
- WELLMETER
- SEAL MARK
- POWER LINE (OVERHEAD)
- TELEPHONE LINE

**PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
GENERAL PLAN**

SCALE IN FEET

CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950
DRAWN BY: S.P.E. TRACED BY: S.P.E. CHECKED BY: S.P.E.

1947

OCTOBER

NOVEMBER

DECEMBER

JANUARY

FEBRUARY

MARCH

5 10 15 20 25

5 10 15 20 25

5 10 15 20 25

5 10 15 20 25

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MEAN DAILY A

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OCTOBER

NOVEMBER

DECEMBER

JANUARY

FEBRUARY

MARCH

A

1948

MEAN DAILY AIR TEMPERATURES

B

1948

JUNE

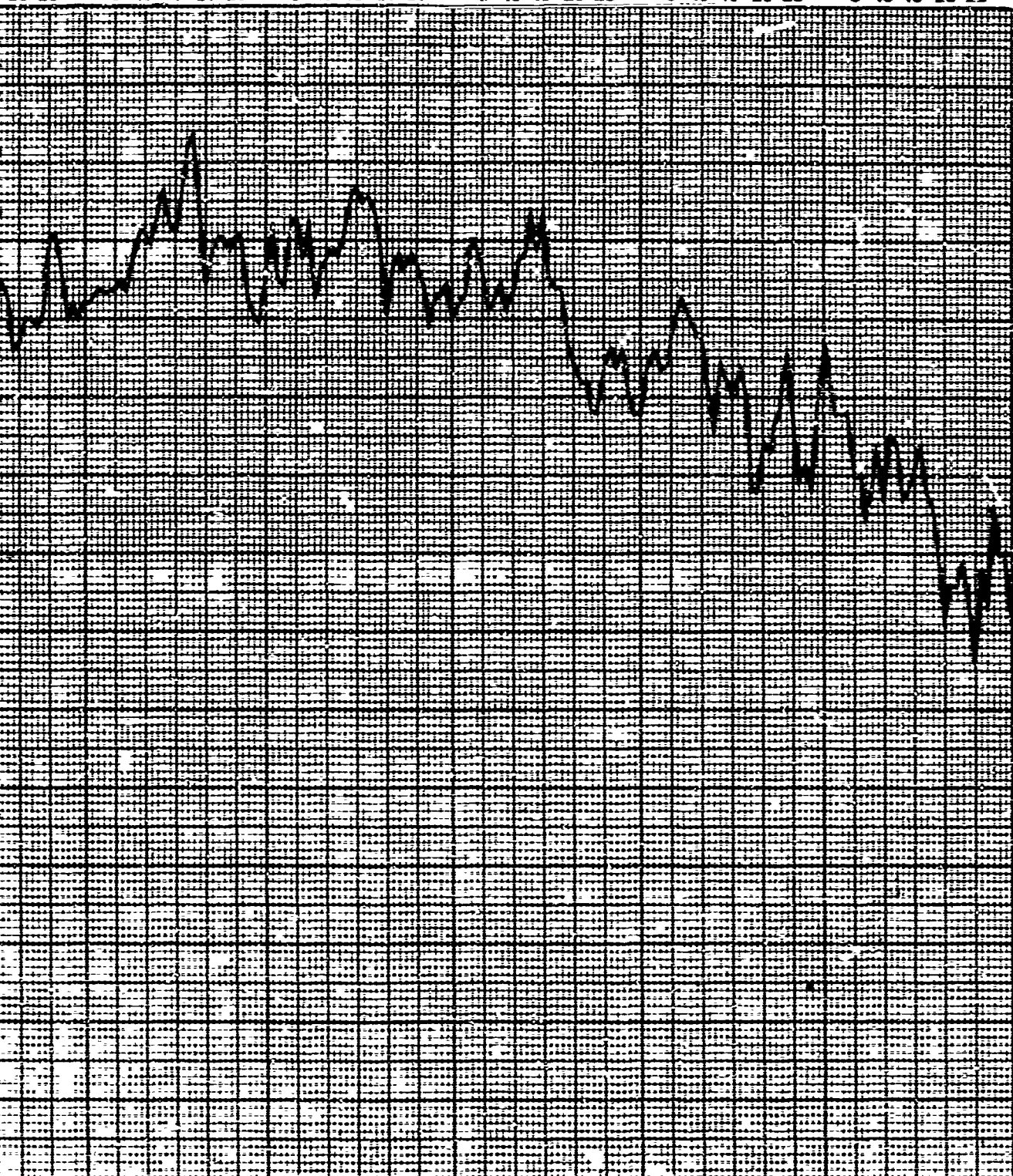
JULY

AUGUST

SEPTEMBER

OCTOBER

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TEMPERATURE-DEGREES CENTIGRADE

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JUNE

JULY

AUGUST

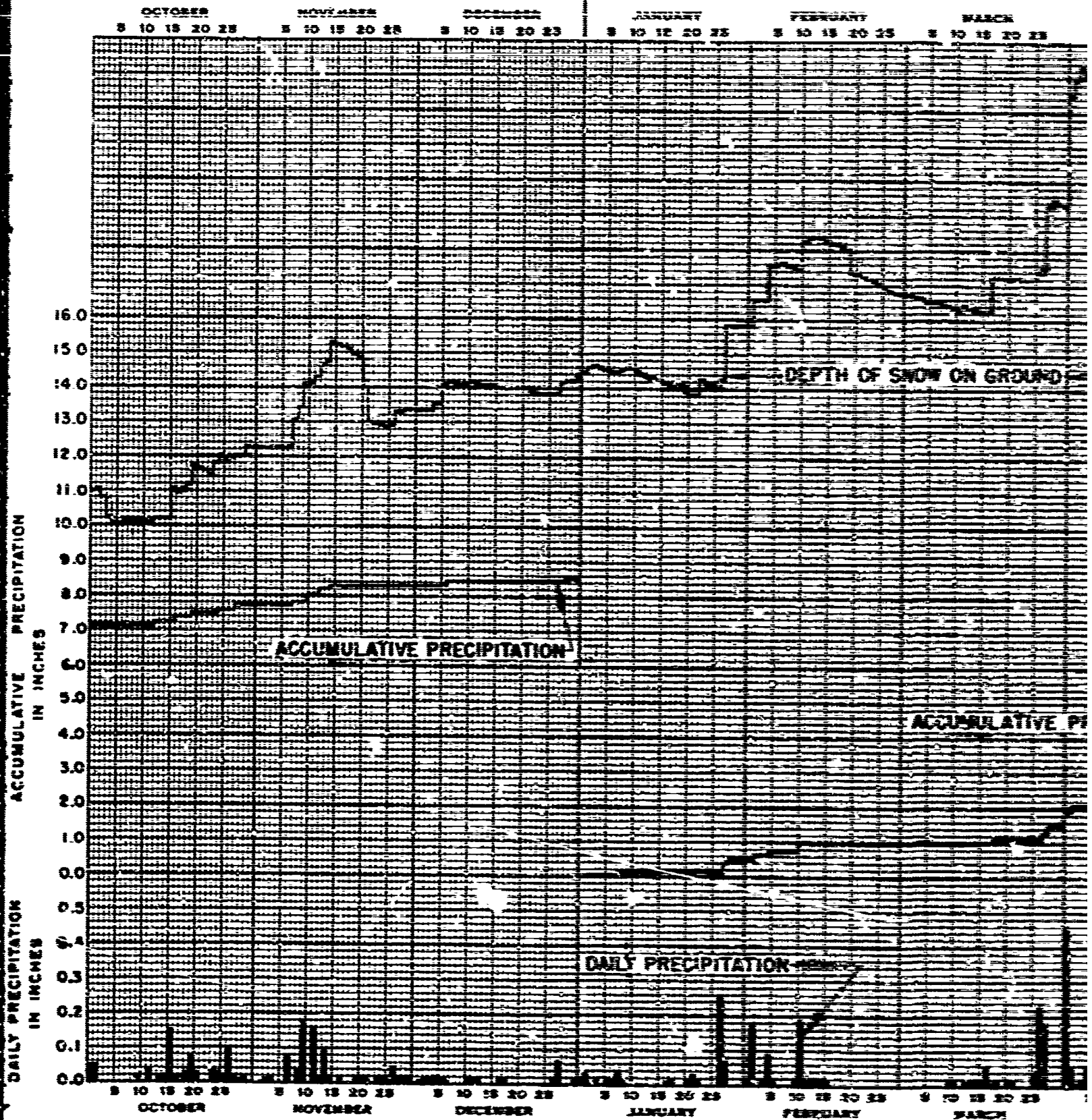
SEPTEMBER

OCTOBER

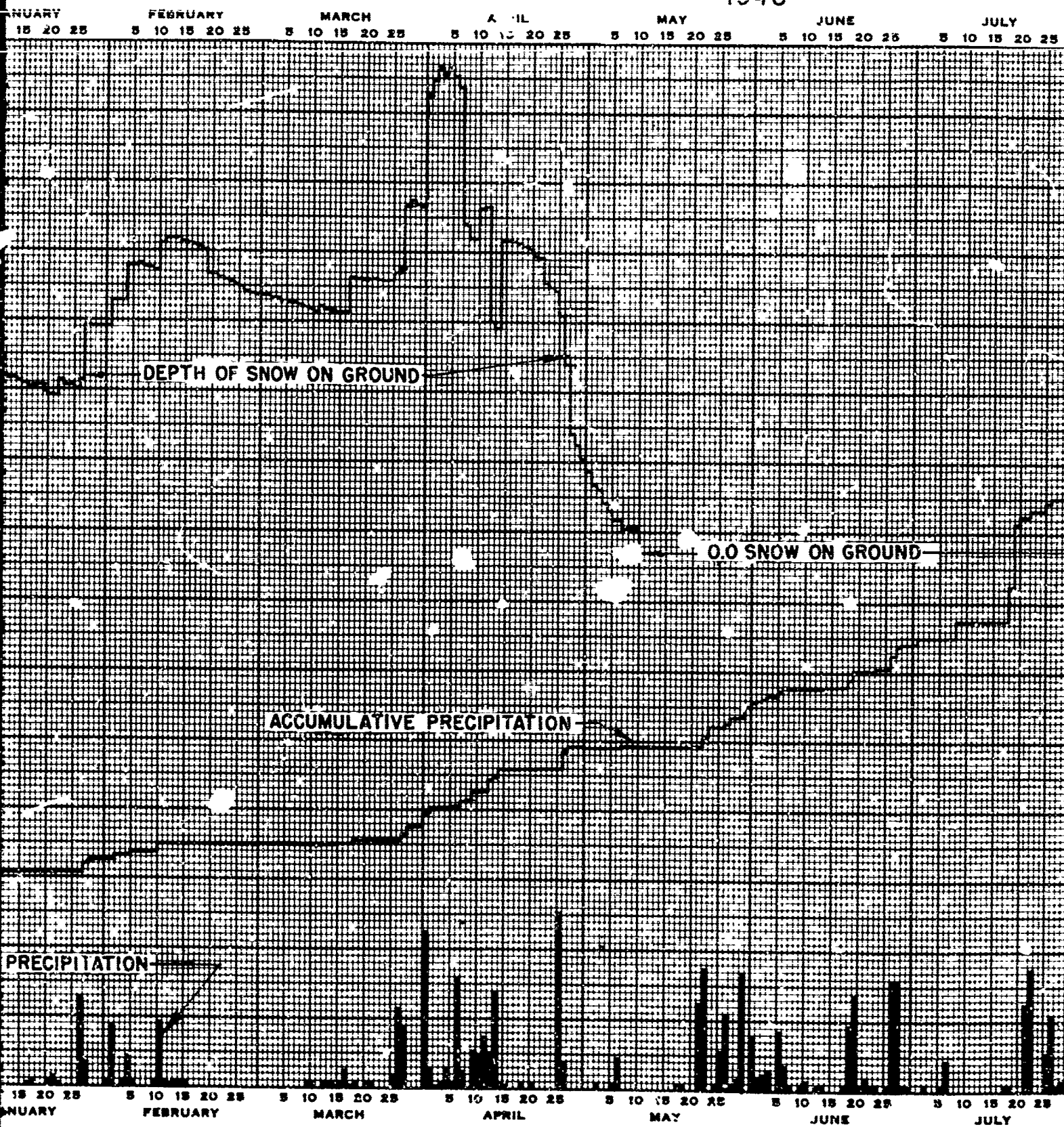
PERMAFROST INVESTIGATION
FIELD RESEARCH FAIRBANKS, ALASKA
AIR TEMPERATURES

CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

1947



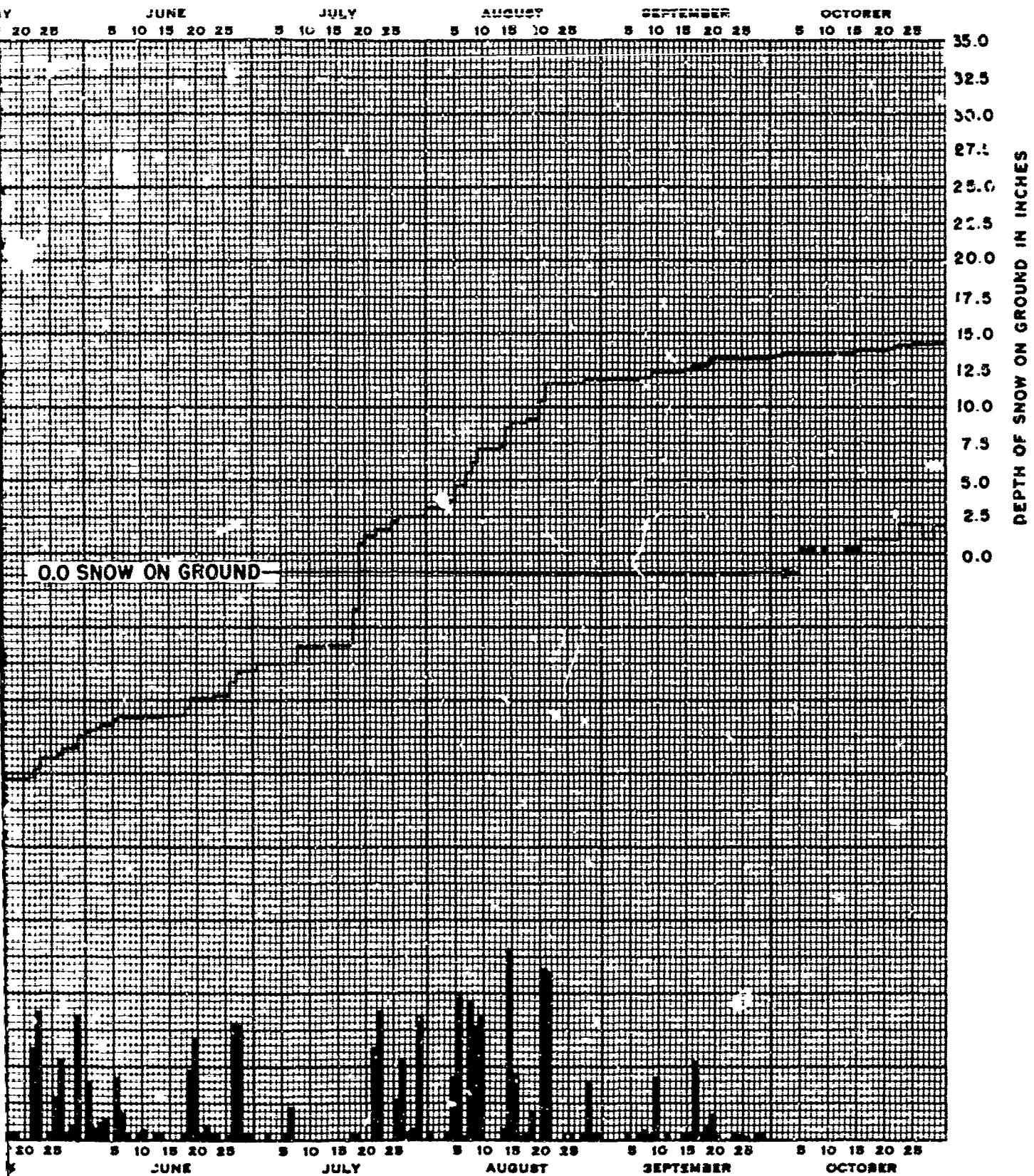
1948



X=TRACE

B

1948



PERMAFROST INVESTIGATION
WEEKS FIELD - FAIRBANKS ALASKA
METEOROLOGICAL DATA
PRECIPITATION & SNOW COVER
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

PLATE III-3

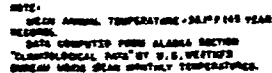
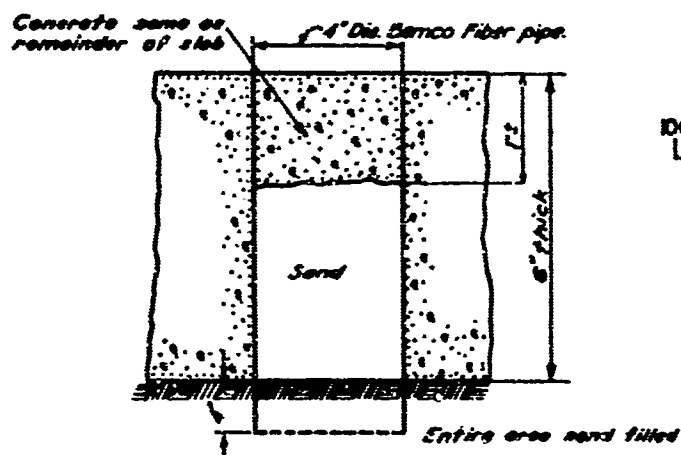
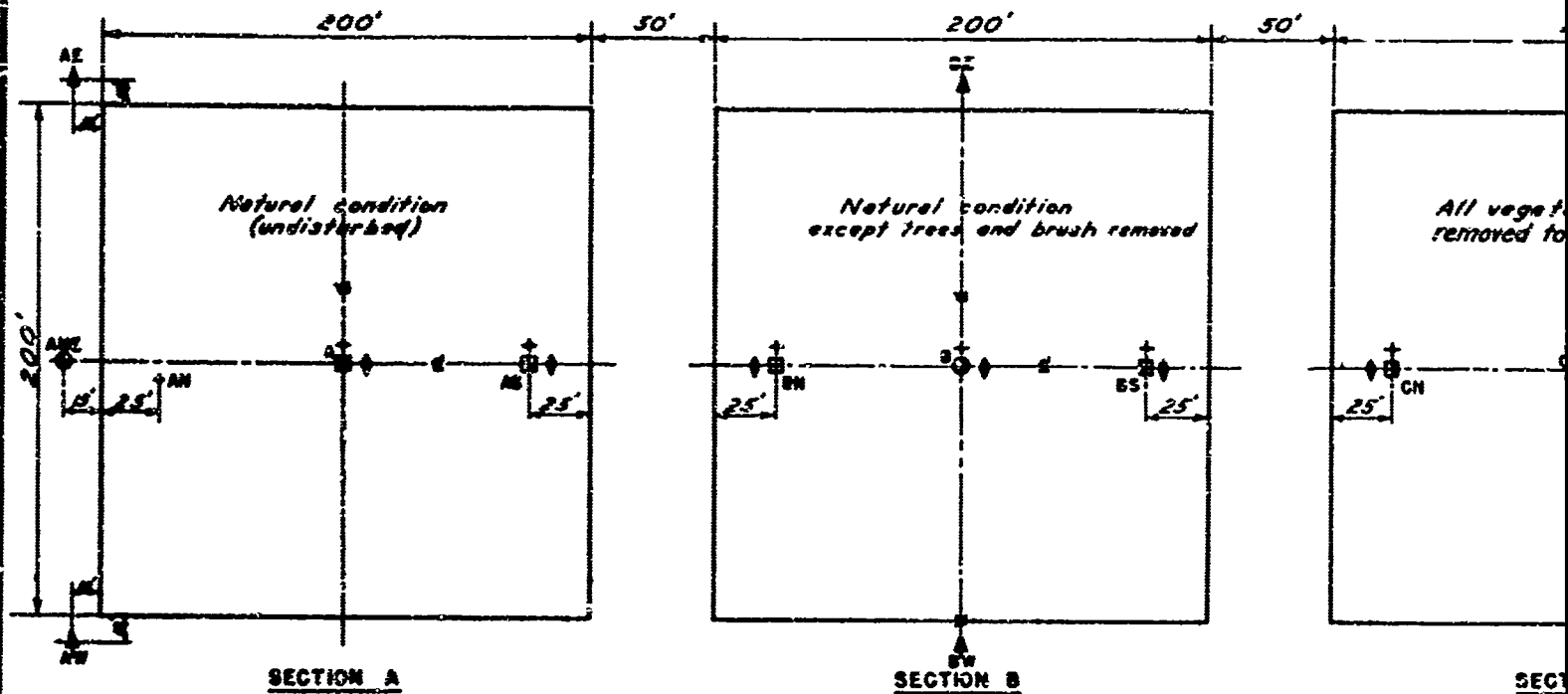
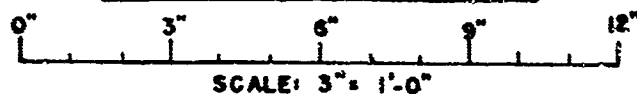


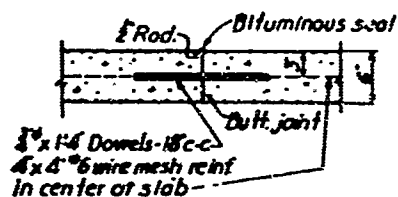
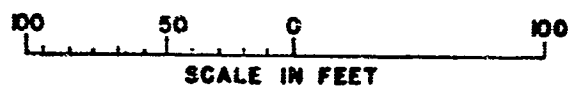
PLATE III-4



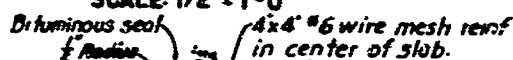
DETAIL OF SAMPLE SLEEVES



PLAN



CONSTRUCTION JOINT



TRANSVERSE DUMMY JOINT



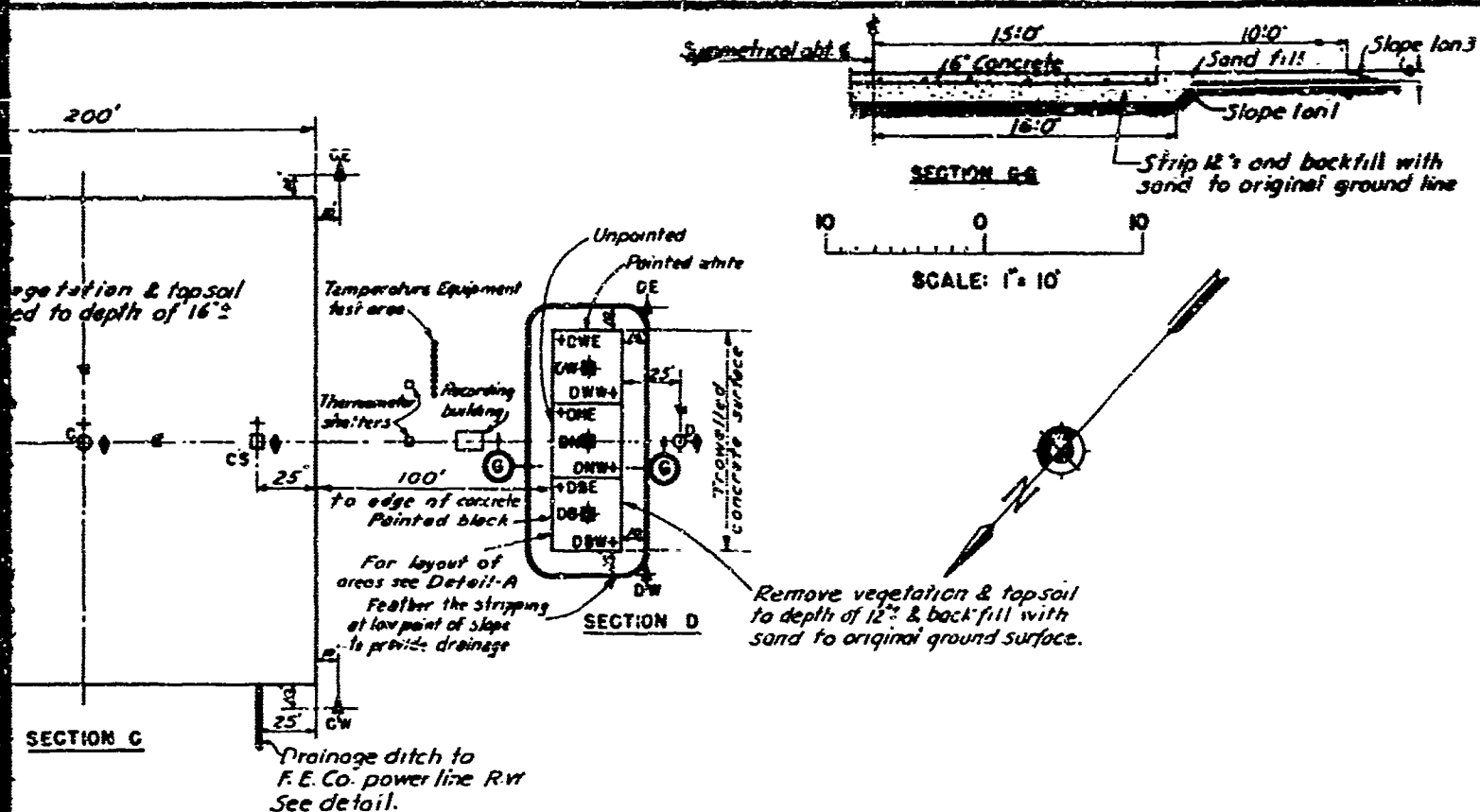
Transverse dummy joint

A



DETAIL OF DRAINAGE DITCH SECTION C

Drainage ditch as shown on section C, location to be established in field.
Cut ditch 12' deep at origin in section C and maintain 0.5% grade to outfall at power line right of way



LEGEND

- ⊕ Core boring 30' deep
 - ⊗ Churn drill boring 30' deep
 - ⊙ Churn drill boring 15' deep
 - ⊙ Churn drill boring 10' deep - Ground water well
 - + Vertical movement observation point
 - ⊗ Bench mark
 - ⊙ Frost observation point
 - Sample sleeves for moisture determination
- } Ground temperature holes

General notes:

Location of thermocouples in ground temperature holes. At the surface and at 3", 6", 9", 12", 15", 18", 21", 24", 27", 30" below the surface.

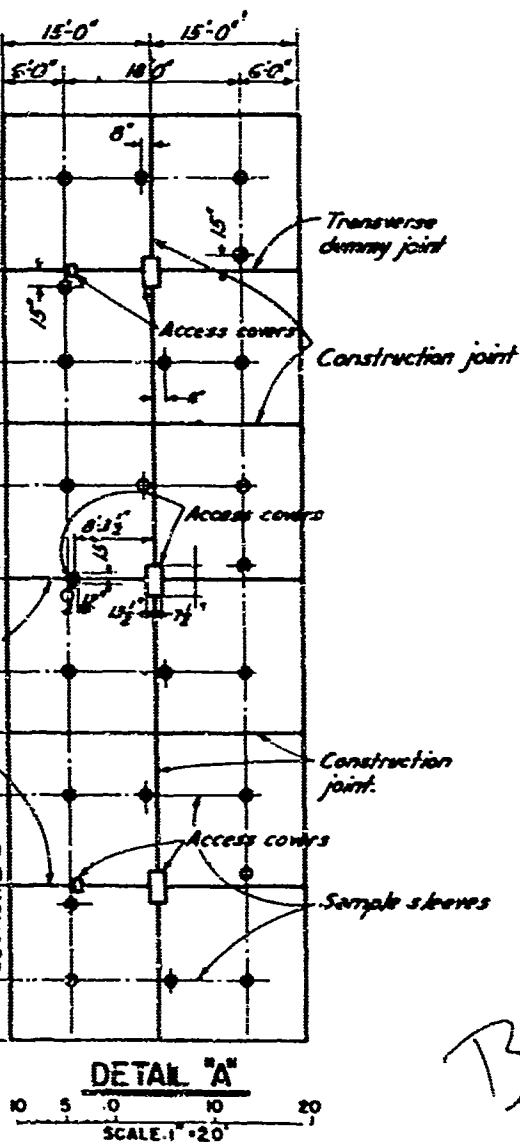
Location of thermocouples in ground temperature holes. At the pavement surface and at 3", 6", 9", 12", 15", 18", 21", 24", 27", 30", 33", 36", 39", 42", 45", 48", 51", 54", and 57" below the pavement surface.

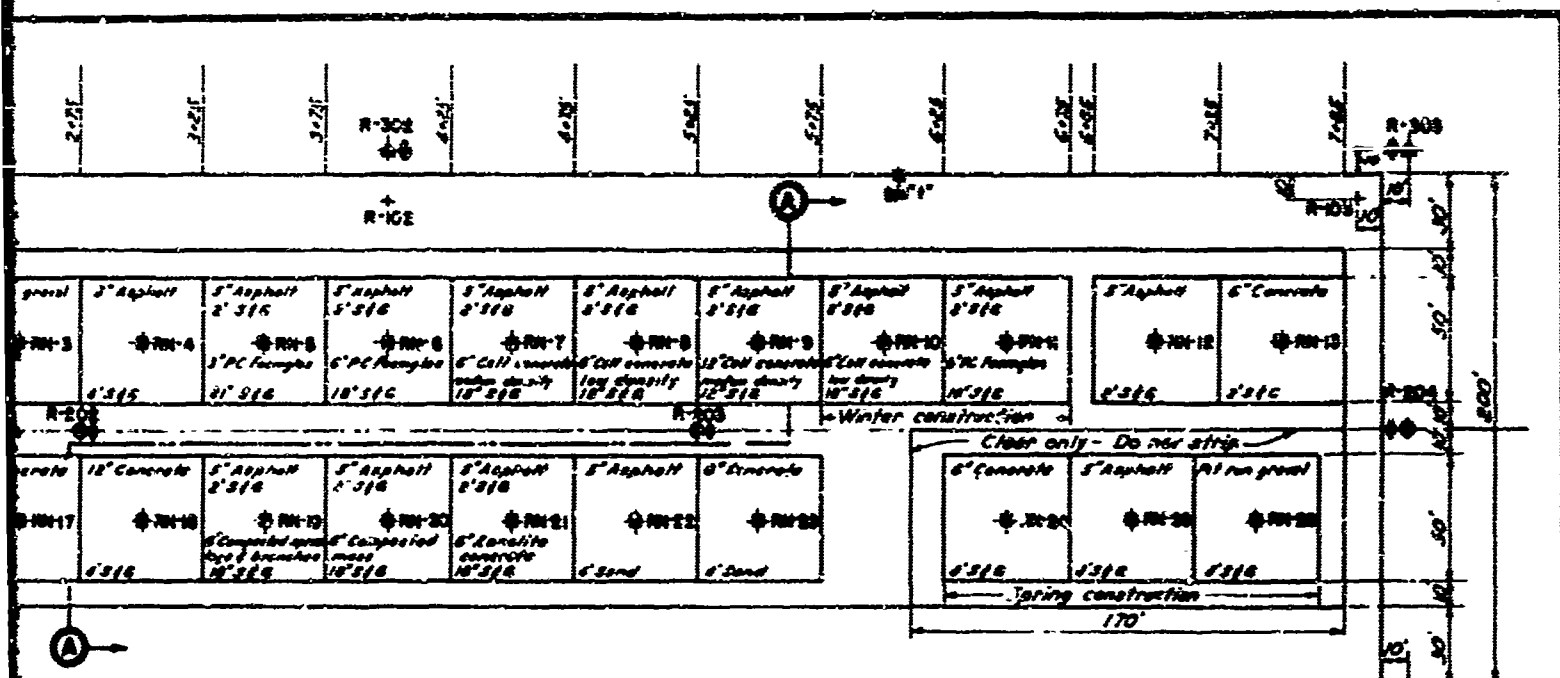
PERMAFROST INVESTIGATION FIELD RESEARCH FAIRBANKS, ALASKA AREA NO. 1 GROUND AND PAVEMENT SURFACE STUDIES

SCALE AS SHOWN

CORPS OF ENGINEERS ST PAUL, MINN MAY 1960

DRAWN BY C.W.B. TRACED BY S.W.J. CHECKED BY:





LEGEND

- Vertical movement observation point.
- 30' Core boring (Ground temperatures observed to 30' depth below top of fill.)
- 10' Churn drill boring (Ground water well)
- 15' Churn drill boring (Ground temperatures observed to 15' depth below top of fill except to 25' depth for holes RN-1 and RN-2, and to 23' depth for hole RN-2.
- Bench mark
- Frost observation point.

Note:

Thermocouples in ground temperature holes RN-1, RN-2 and RN-14 are located at the pavement surface and at 6", 1', 1'-6", 2'-2", 3'-6", 4'-6", 5'-6", 7'-6", 9'-6", 11'-6", 12'-6", 14'-6", 15'-6", 20'-6", and 25'-6" below the surface except that in RN-2 the lowest thermocouple is at 23' instead of 25'.

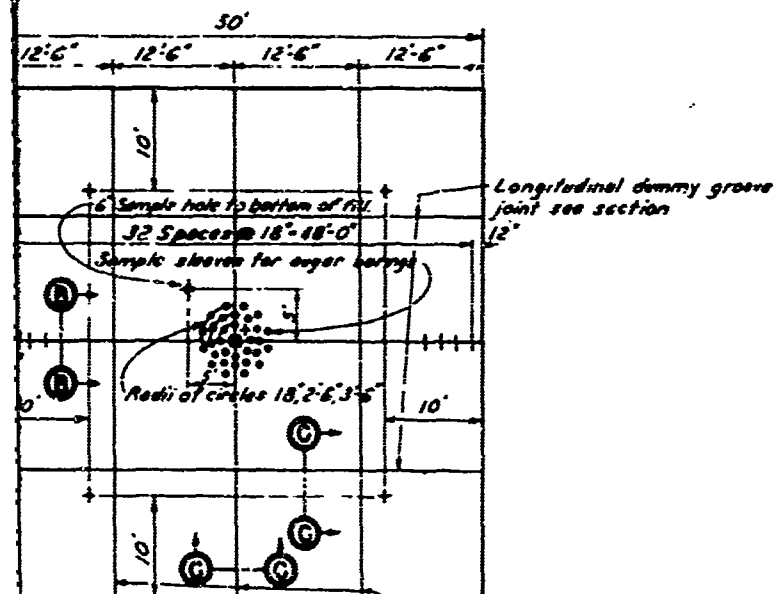
Thermocouples in ground temperature holes RN-3 and RN-26 are located at the gravel surface and at 6", 1'-6", 2'-6", 4'-6", 10'-6", and 15'-6" below the surface.

Thermocouples in ground temperature hole RN-18 are located at the pavement surface and at 1'-6", 2'-6", 3'-6", 5'-6", 7'-6", 11'-6", and 15'-6" below the surface.

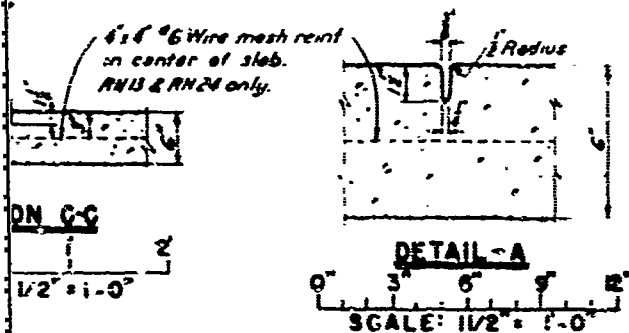
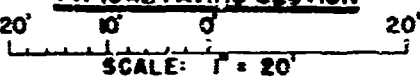
Thermocouples in ground temperature holes R-201, R-202, R-203 and R-204 are located at the gravel surface and at 6", 1'-6", 2'-6", 4'-6", 10'-6", and 15'-6" below the surface.

Thermocouples in all other ground temperature holes in Area 2 are located at the pavement surface and at 6", 1'-6", 2'-6", 3'-6", 4'-6", 6'-6", 10'-6", and 15'-6" below the surface.

Vertical movement observation points for unpaved sections are located the same as those shown in typical paving section except that the center point is not off-set.



TYPICAL PAVING SECTION



PERMAFROST INVESTIGATION

FIELD RESEARCH

FAIRBANKS, ALASKA

AREA NO. 2

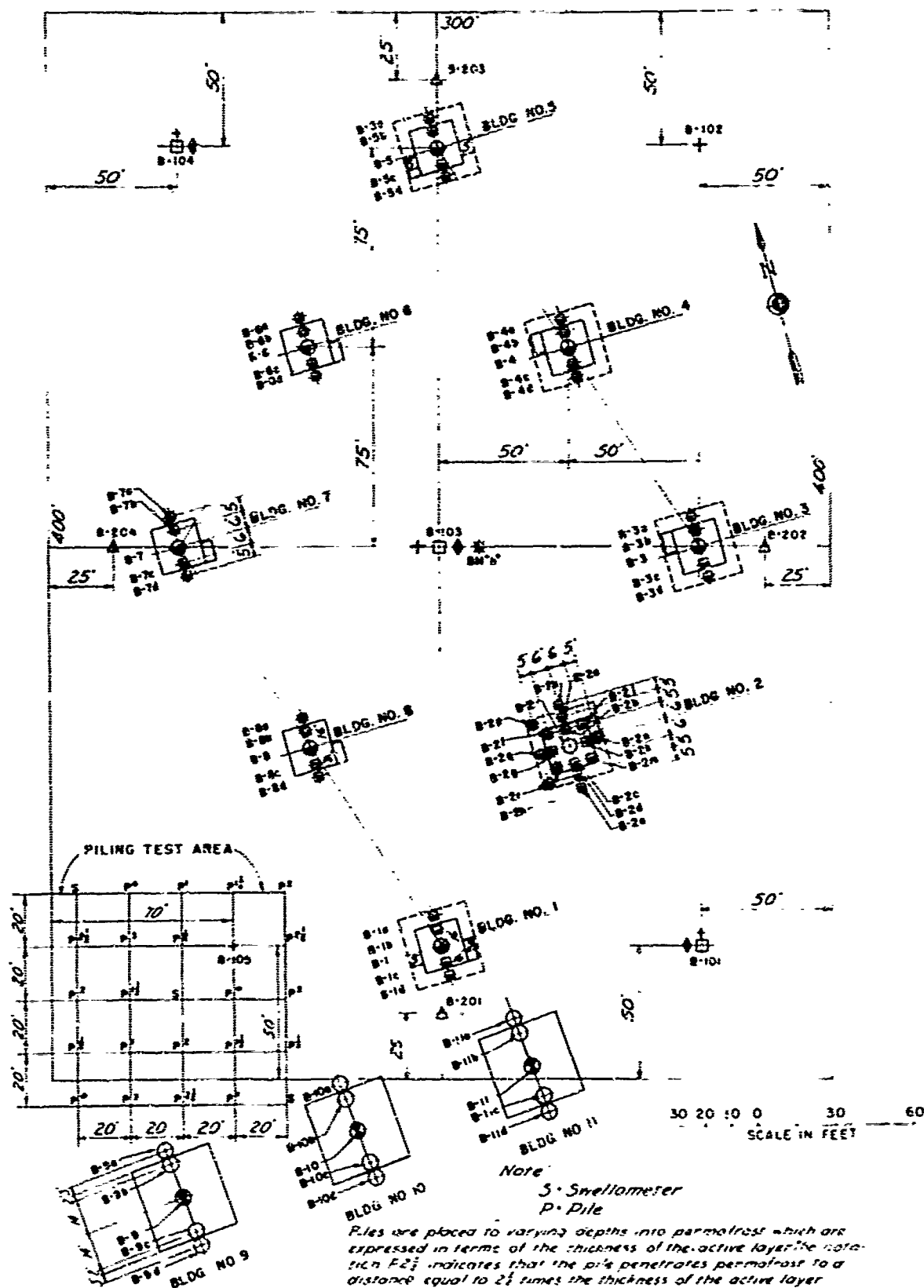
RUNWAY FOUNDATION STUDIES

SCALE: AS SHOWN

CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1960

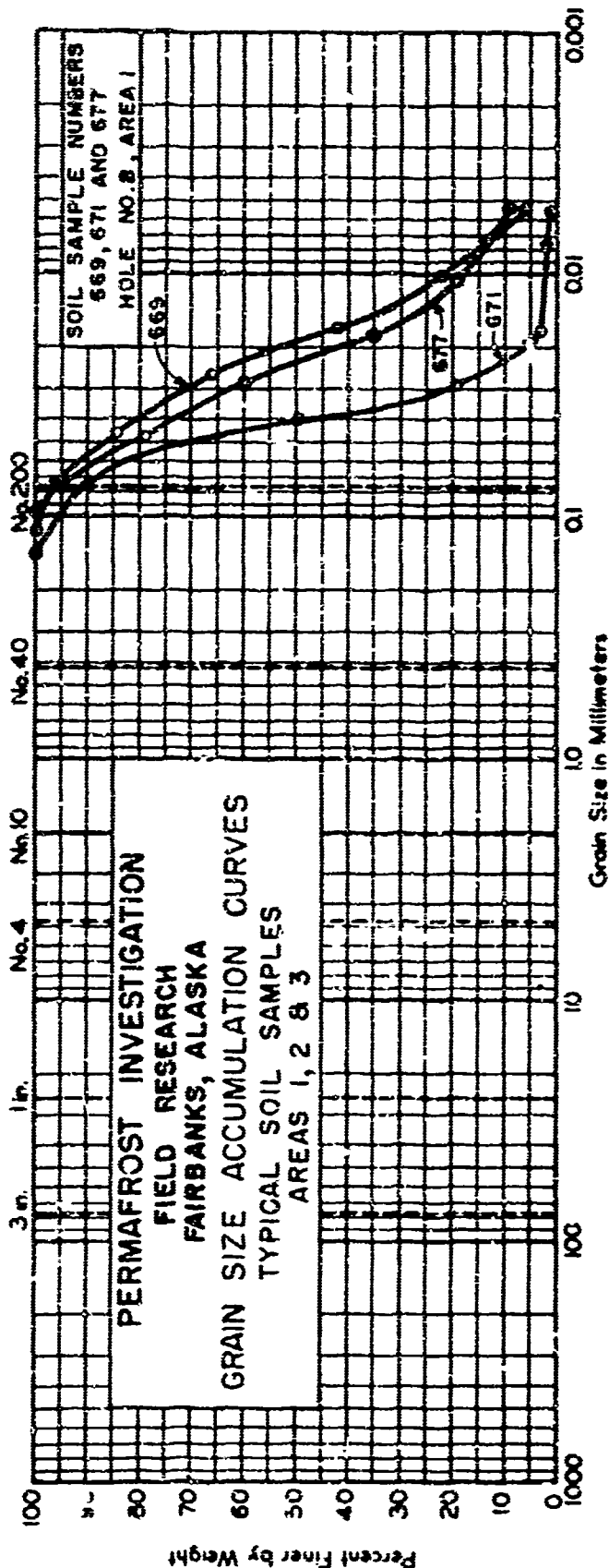
DRAWN BY CWS TRACED BY: S.S.J. CHECKED BY:

PLATE III-6



MECHANICAL ANALYSIS

U. S. Standard Sieve Size



COBBLES	GRAVEL			SAND		SILT or CLAY
	Coarse	Medium	Fine	Coarse	Fine	

Number	Natural Moisture	LL	PL	PI	Classification	Remarks
669	74	-	-	-	SH MH	Nonplastic
671	28	-	-	-	Sandy Silt MH	"
677	35	-	-	-	" "	"

SOILS LABORATORY
PERMAFROST
LADD, AFB, ALASKA

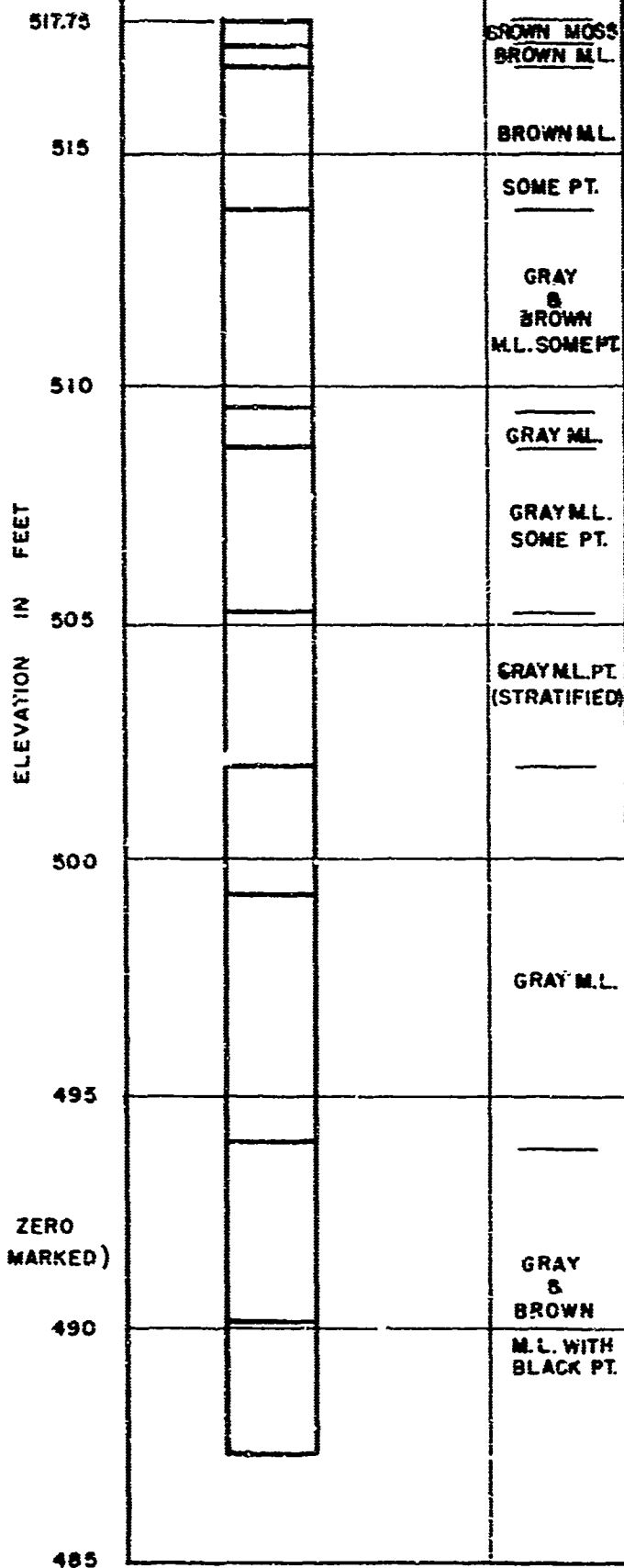
PROJECT Field Research
AREA No. 1
LOCATION Sec. B HOLE NO. Temp. "B"
PLOTTER BY CHECKED BY

305
7 NOV 49

MAY 1950

HOLE NO. A

C. OF E. SOIL CLASSIFICATION



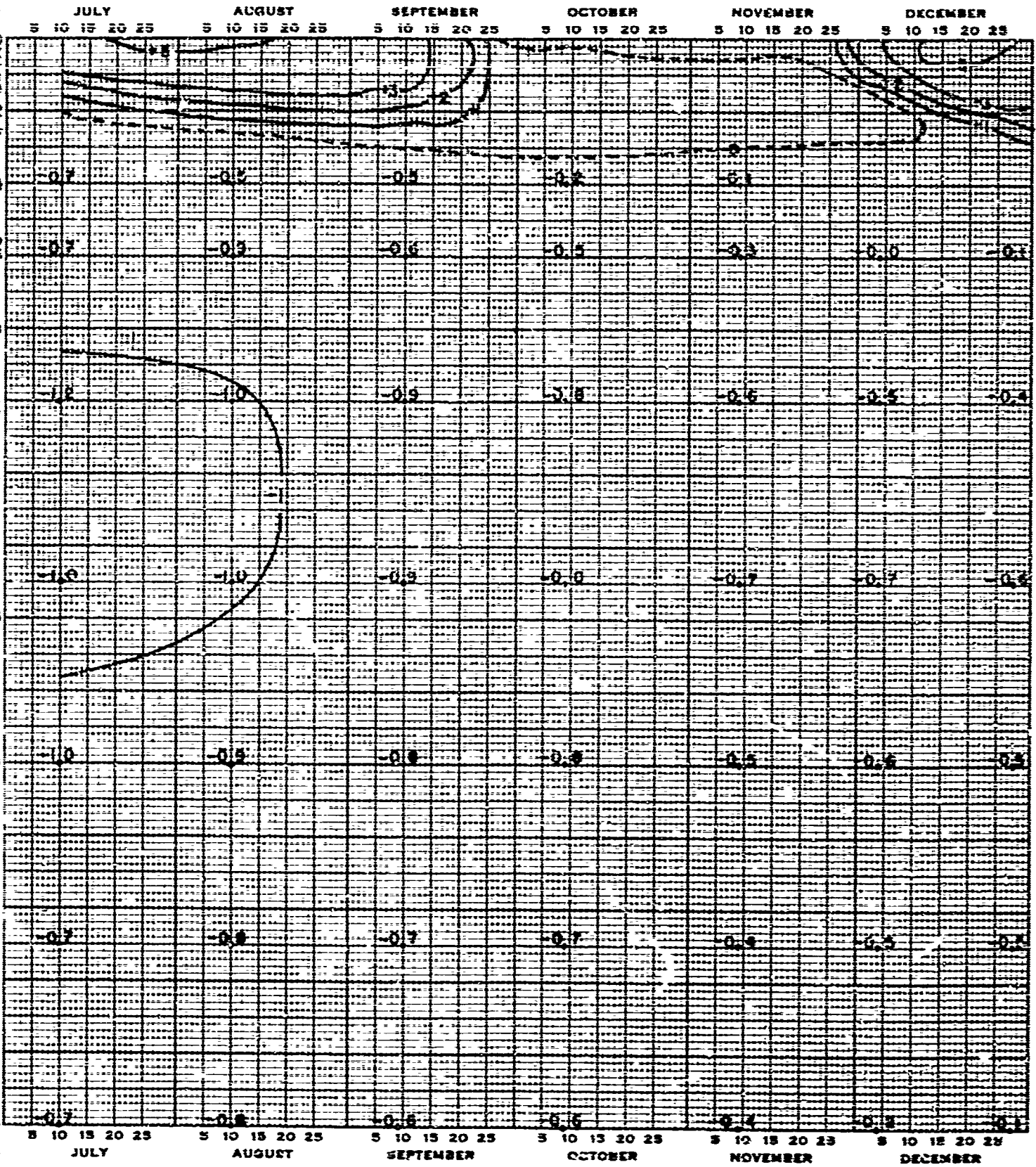
LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO (ABOVE OR BELOW AS MARKED)

A

GROUND SURFACE EL. 517.75

1946



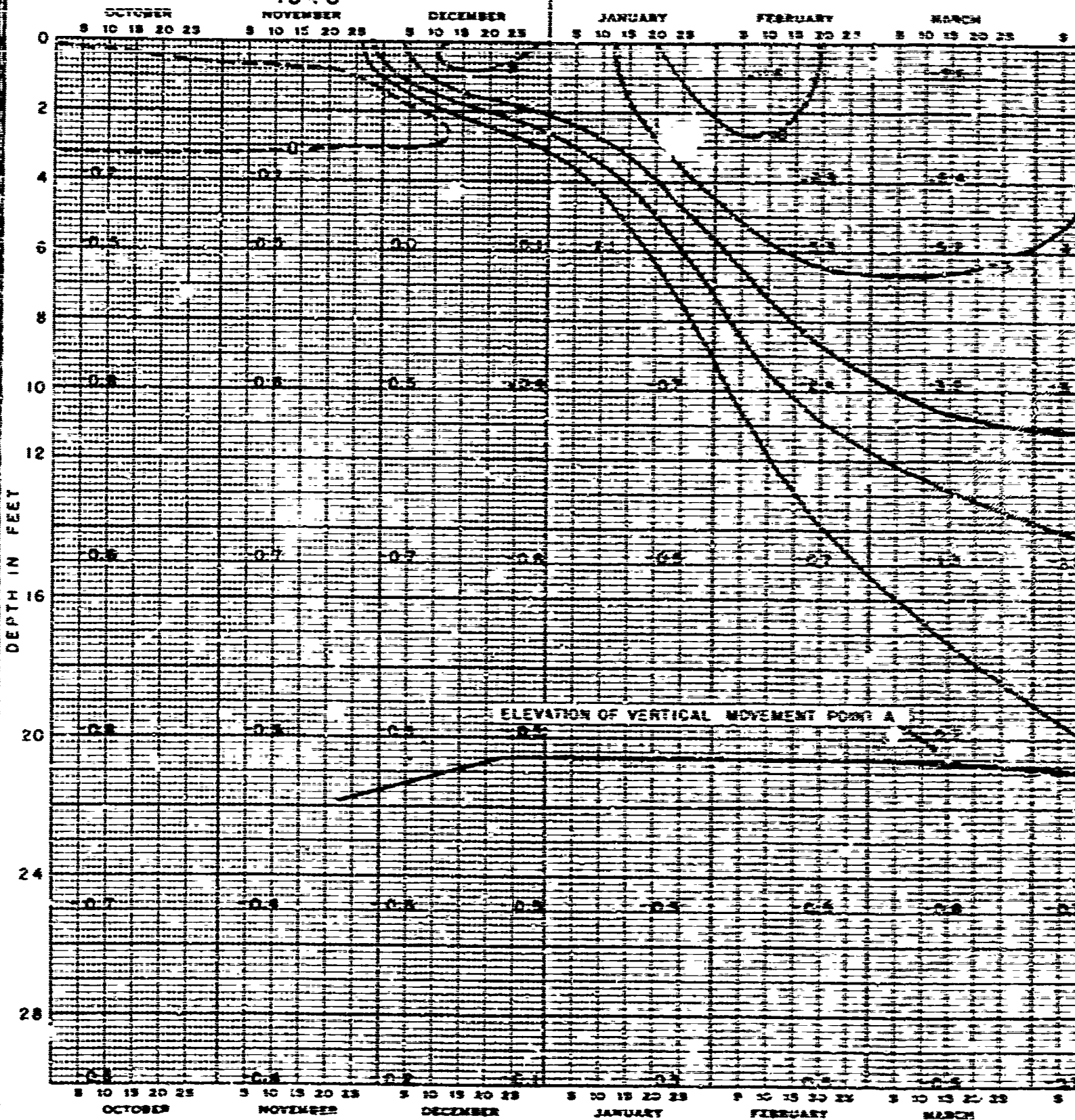
GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
LOCATION OF GROUND TEMPERATURE HOLE
SHOWN ON PLATE III-5

B

PERMAFROST INVESTIGATION
FIELD RESEARCH - FAIRBANKS, ALASKA
AREA NO. 1, HOLE NO. A
GROUND ISOTHERMS
AND SURFACE ELEVATIONS
1946
U.S. ENGR. OFFICE ST. PAUL, MINN. MAY 1950

1946



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

1947

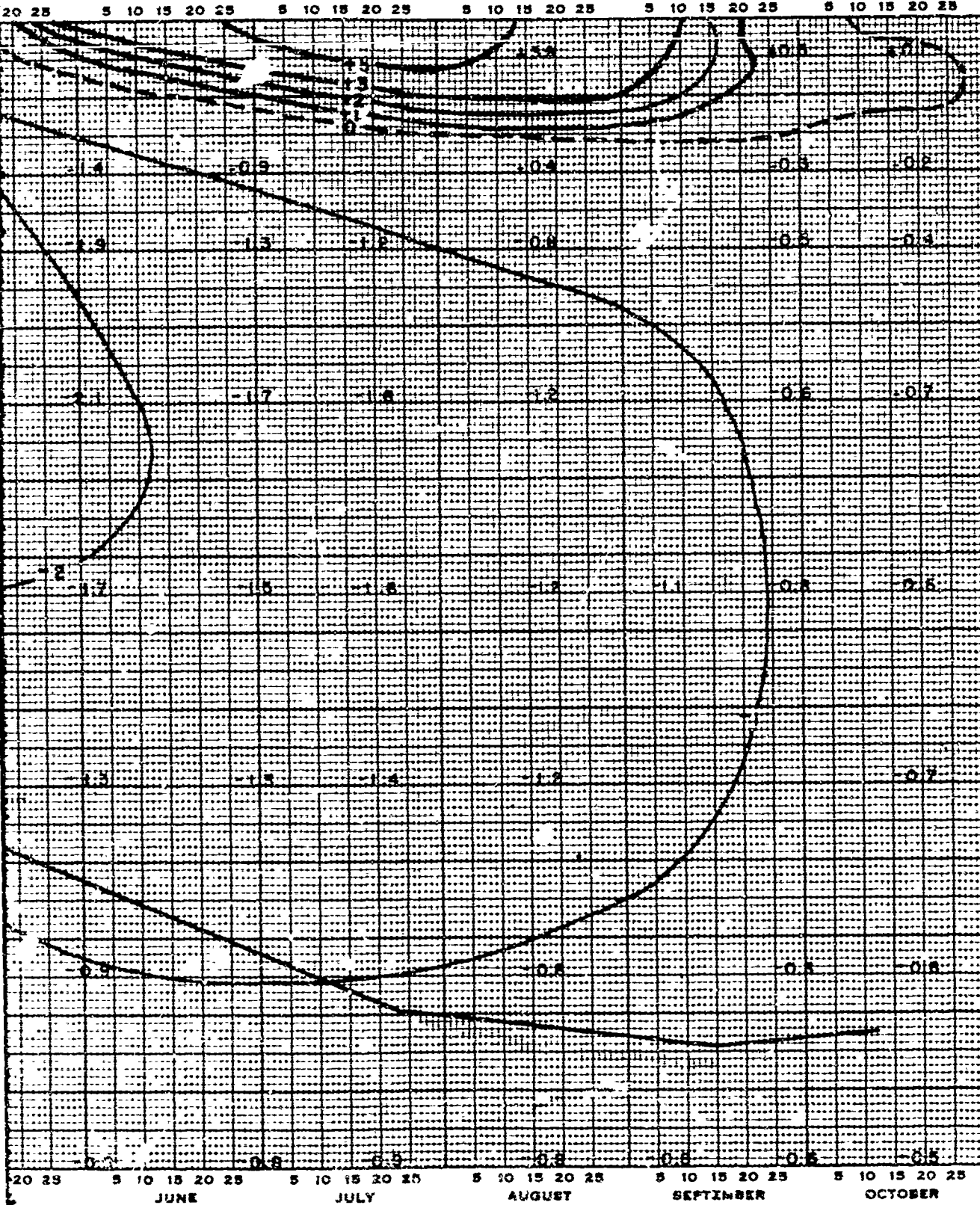
JUNE

JULY

AUGUST

SEPTEMBER

OCTOBER



518.4
518.3
518.2
518.1
518.0

ELEVATION

NOTES:
RMS SHOWN IN DEGREES CENTIGRADE.
N A IS ENTIRELY UNDISTURBED AND
D WITH TREES, BRUSH, AND MOSS.

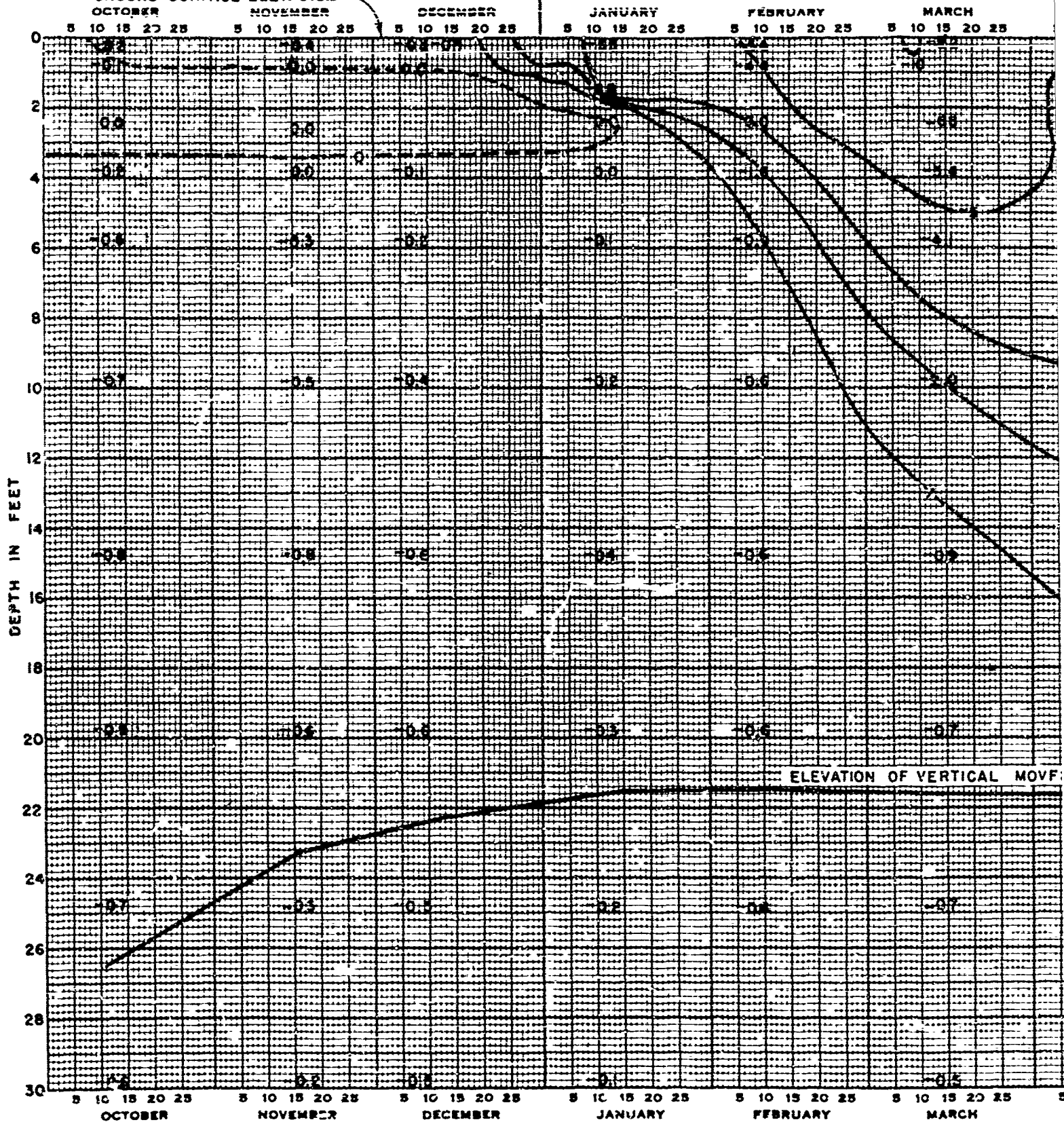
C

PERMAFROST INVESTIGATION
FIELD RESEARCH-FAIRBANKS, ALASKA
AREA NO. 1 - HOLE NO. A
GROUND ISOOTHERMS AND SURFACE ELEVATIONS
1946-47
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

PLATE III-10

1947

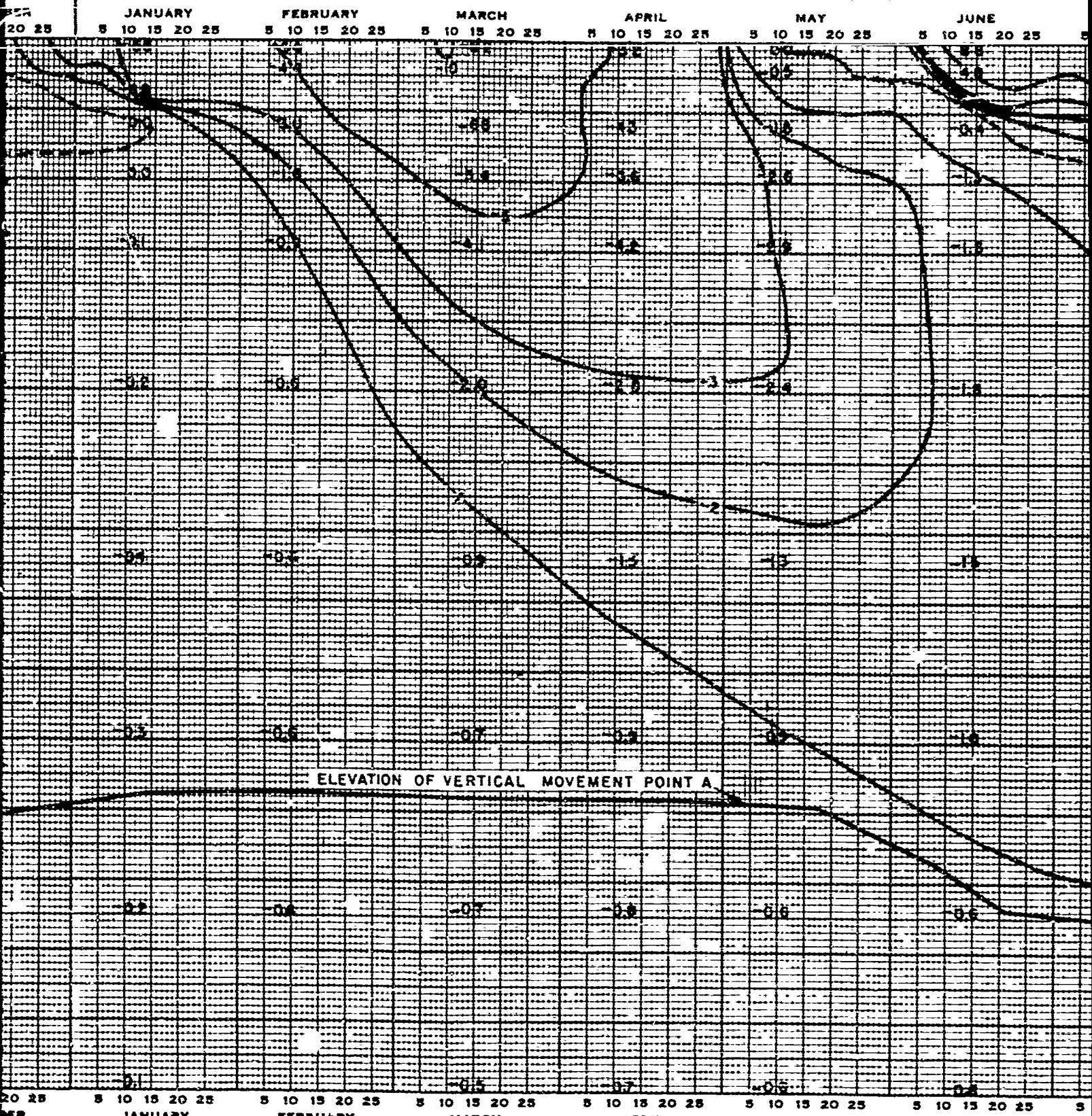
GROUND SURFACE ELEV. 518.3



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

1948



LEGEND

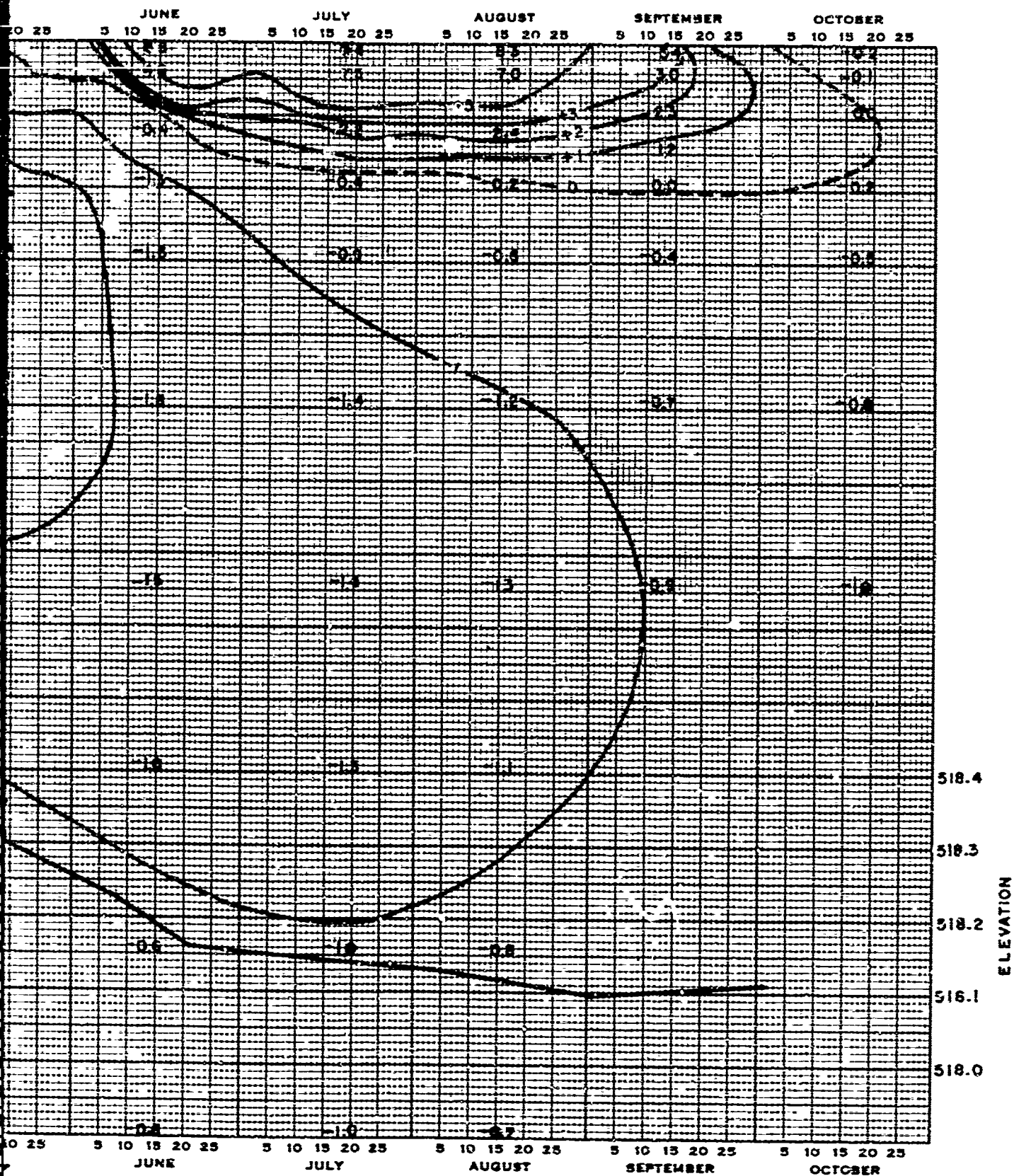
----- ZERO ISOTHERM
 _____ ISOTHERMS OTHER THAN ZERO
 ' ABOVE OR BELOW AS MARKED)

GENERAL NOTE:

ISOTHERMS SHOWN IN DEGREES CENT
SECTION A IS ENTIRELY UNDISTURBE
IS COVERED WITH TREES, BRUSH, AND MO

3

1948



NOTES:

MS SHOWN IN DEGREES CENTIGRADE.
A IS ENTIRELY UNDISTURBED AND
WITH TREES, BRUSH, AND MOSS.

PERMAFROST INVESTIGATION
FIELD RESEARCH - FAIRBANKS, ALASKA
AREA NO. 1 - HOLE NO. A
GROUND ISOTHERMS AND SURFACE ELEVATIONS
1947-48
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

PLATE III-11

HOLE NO. B

EL. 511.60

510

505

500

495

490

485

480

475

SOIL CLASSIF.

C. OF E.

PEAT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

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SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

SANDY SILT

C. OF E.

BROWN PT.

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

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(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

BROWN ML

WITH PT.

(STRATIFIED)

LEGEND

ZERO ISOTHERM

ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

GROUND

0 5 10

0.50

1.5

2

2.5

4.0

4

6.0

6

8

10.0

10

12

14

15.0

16

18

20.0

20

22

24

25.0

26

28

30.0

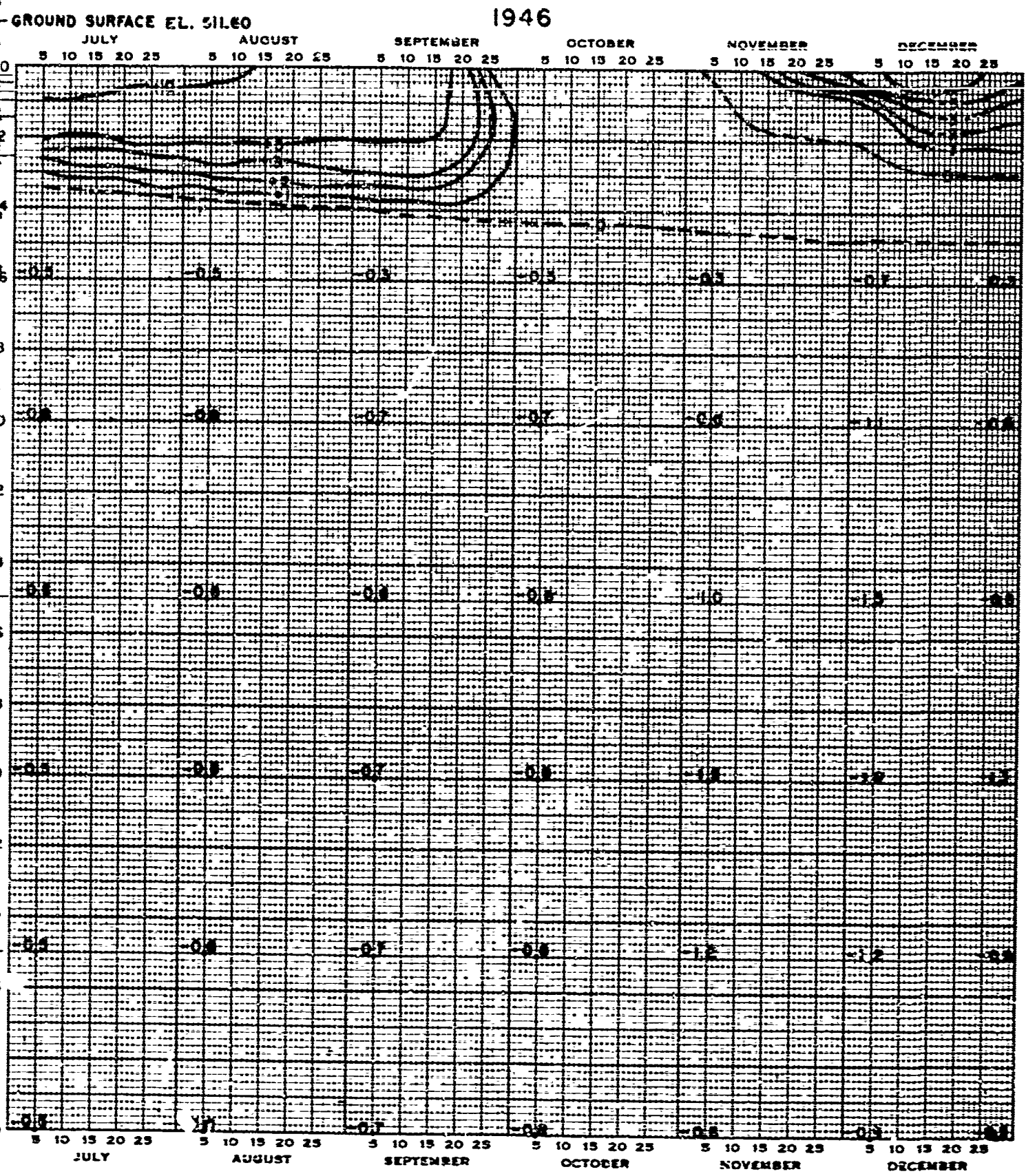
30

8 10

DEPTHS OF THERMOCOUPLES IN FEET

DEPTH IN FEET

H



GENERAL NOTES:

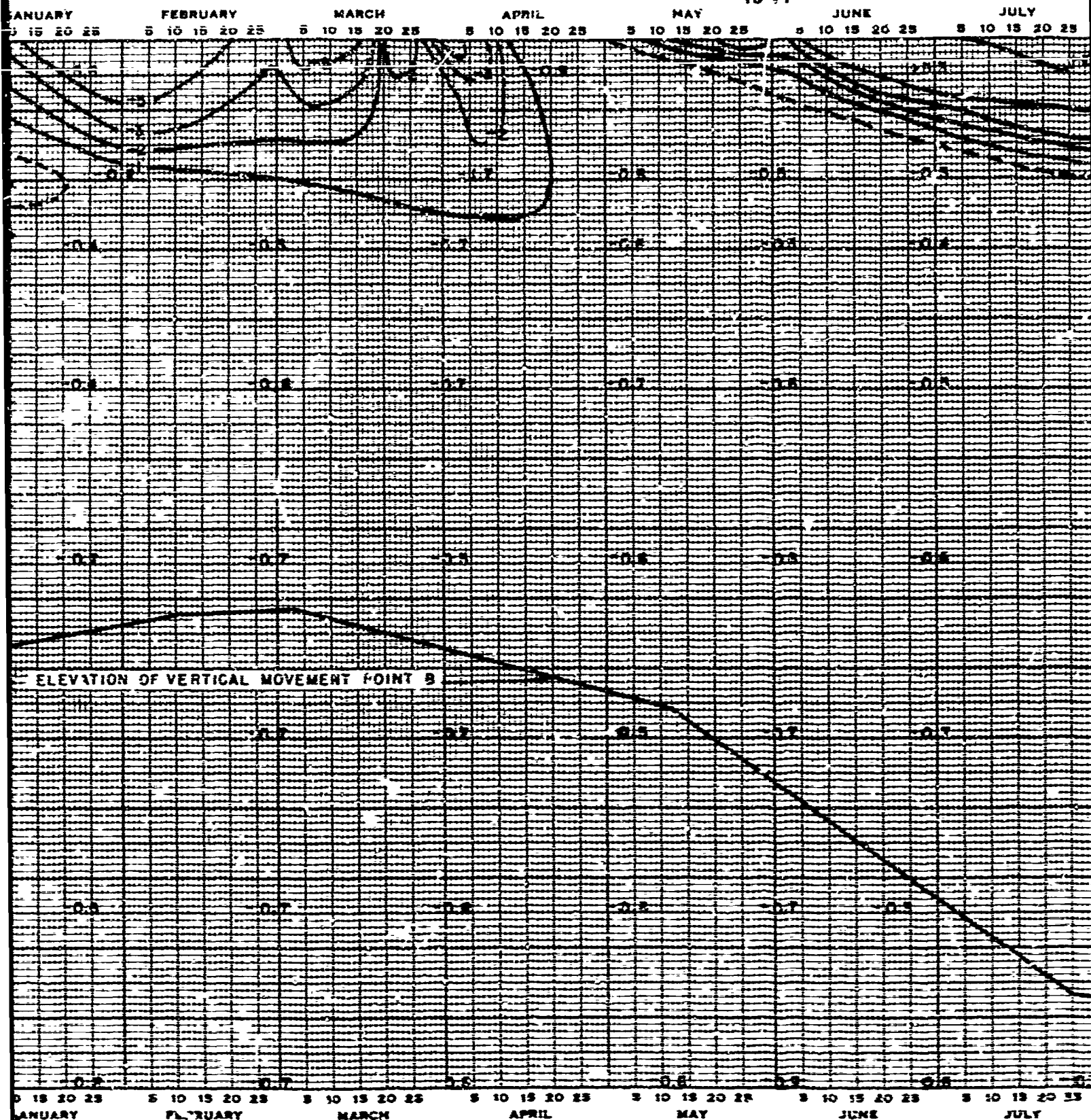
ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
 LOCATION OF GROUND TEMPERATURE HOLE
 SHOWN ON PLATE III - 5

B

PERMAFROST INVESTIGATION
 FIELD RESEARCH - FAIRBANKS, ALASKA
 AREA NO. 1-HOLE B
 GROUND ISOTHERMS AND SURFACE ELEVATIONS
 1946

U.S. ENGR. OFFICE ST. PAUL, MINN. MAY 1950

1947



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
SECTION B HAS BEEN CLEARED OF TREES AND
BRUSH. MOSS COVER NOT REMOVED.

0675310

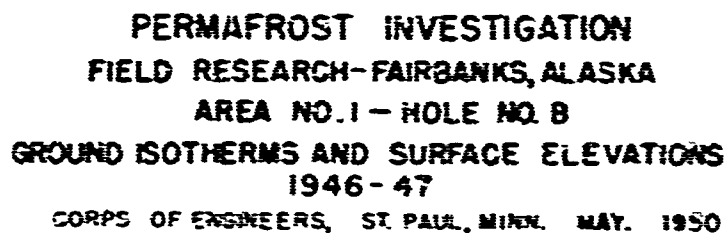
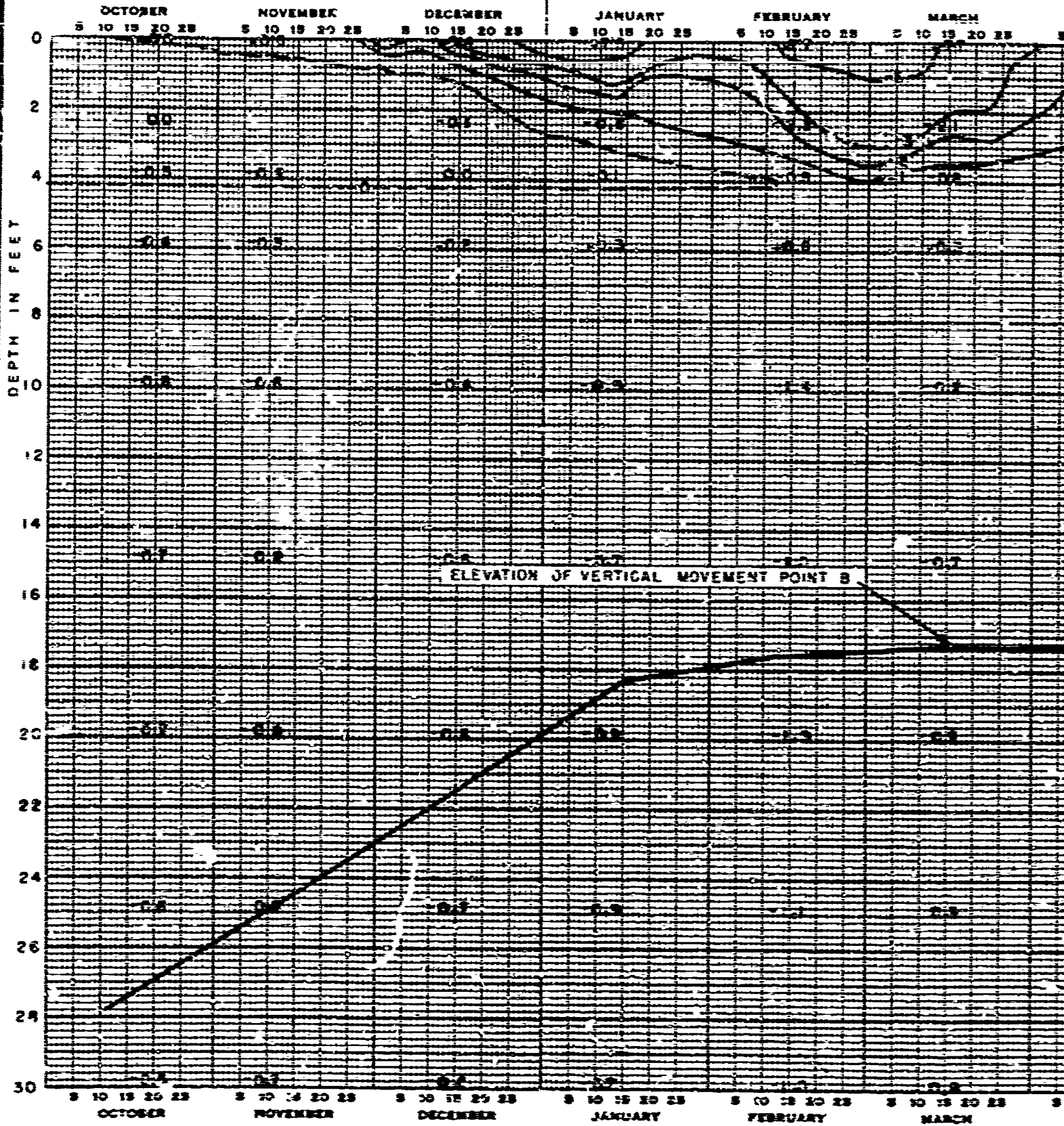


PLATE III-13

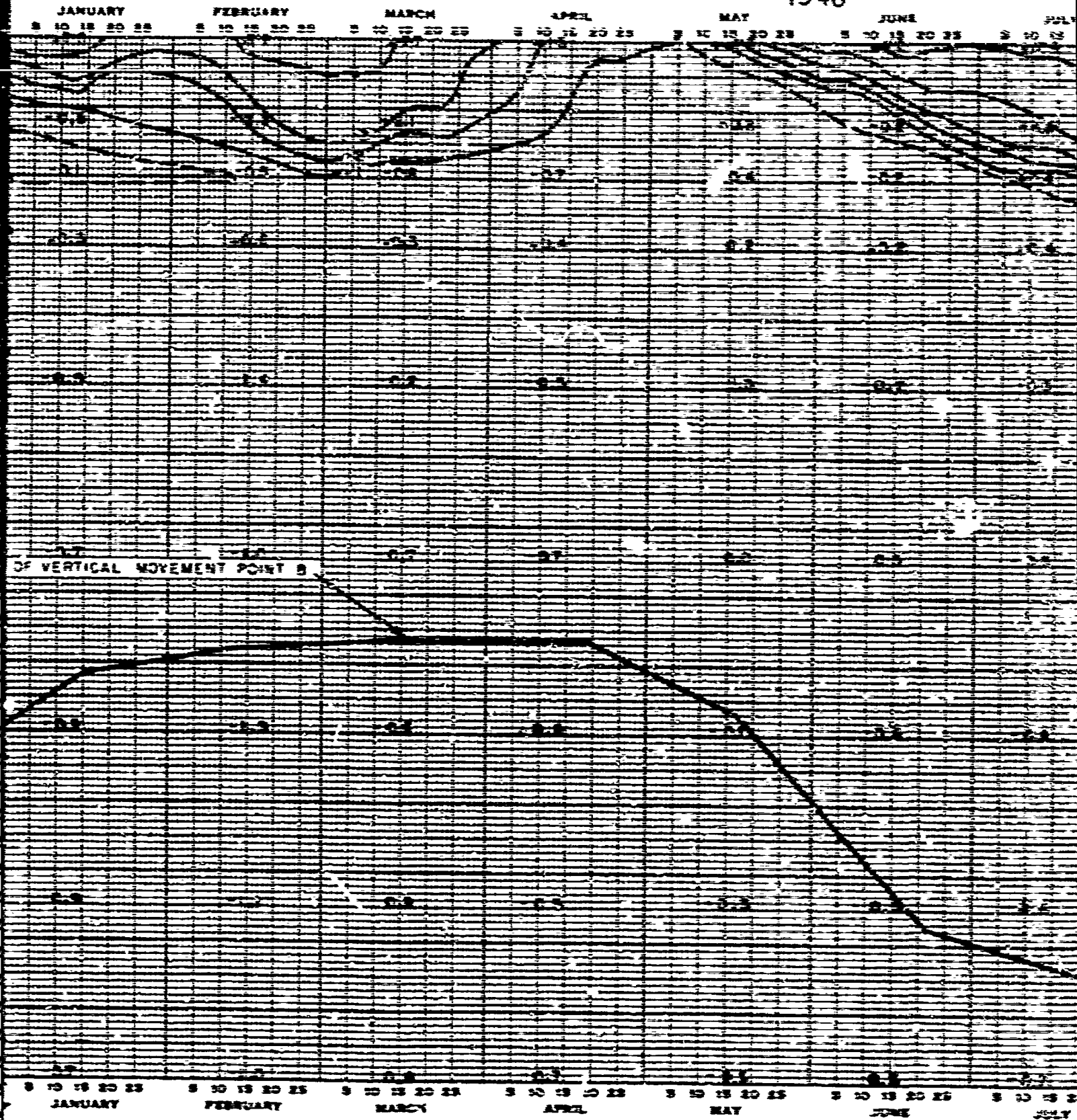
1947



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

1948



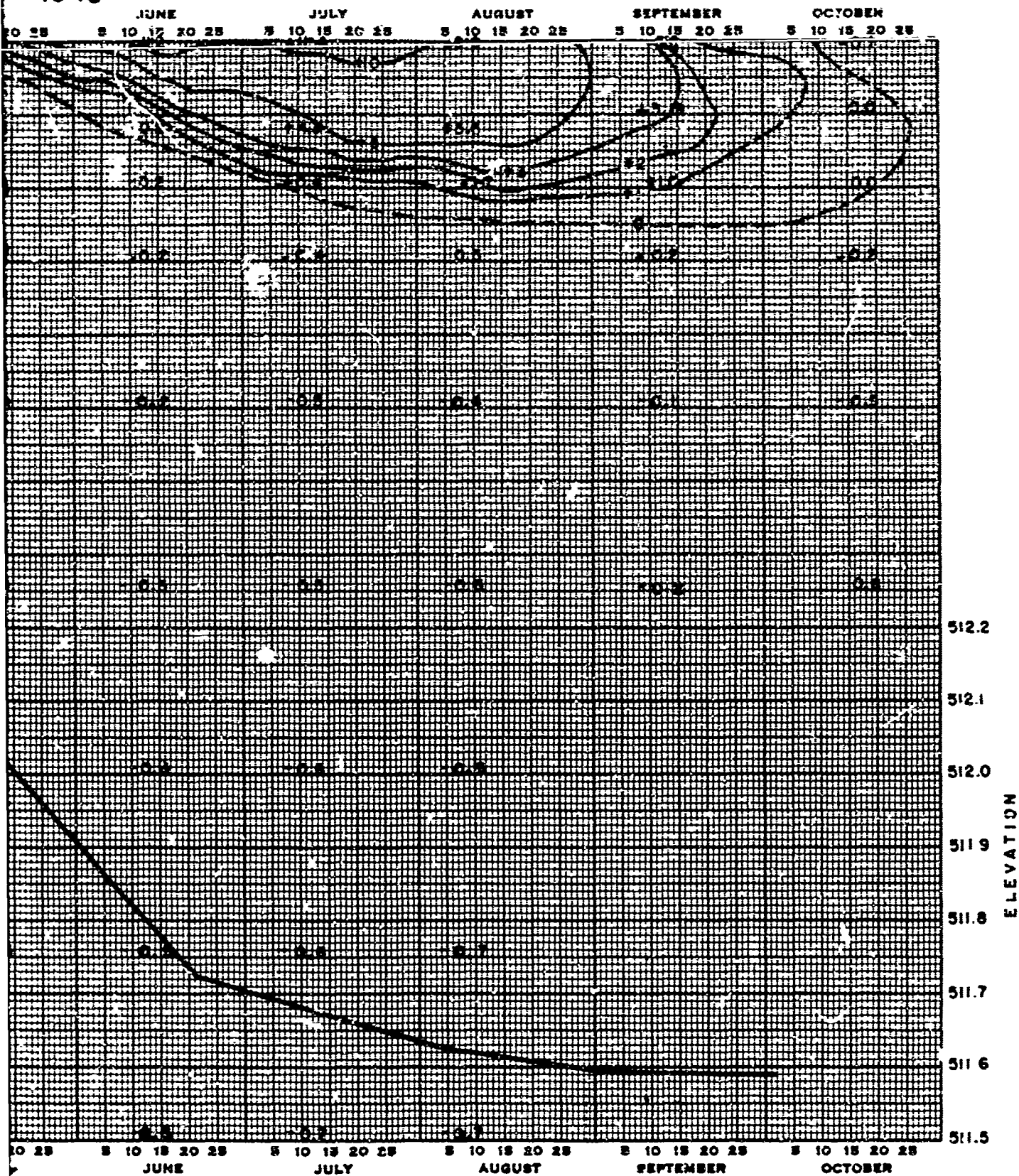
LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO (ABOVE OR BELOW AS MARKED)

GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
SECTION 8 HAS BEEN CLEARED OF TREES &
BRUSH. MOSS COVER NOT REMOVED.

1948



NOTES:

MS SHOWN IN DEGREES CENTIGRADE.
B HAS BEEN CLEARED OF TREES AND
MOSS COVER NOT REMOVED.

PERMAFROST INVESTIGATION
FIELD RESEARCH-FAIRBANKS, ALASKA
AREA NO. 1-HOLE NO. B
GROUND ISOTHERMS AND SURFACE ELEVATIONS
1947-48
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

HOLE NO. C

E1. 507.83

SOIL CLASSIF

G. OF E.
PEAT

G. OF E.
BROWN PT.

505

SANDY SILT

BROWN ML
WITH PT.
(STRATIFIED)

500

SILT

495

SANDY SILT

GREY ML
WITH PT.
(STRATIFIED)

490

SILT

485

SANDY SILT

GREY ML

480

475

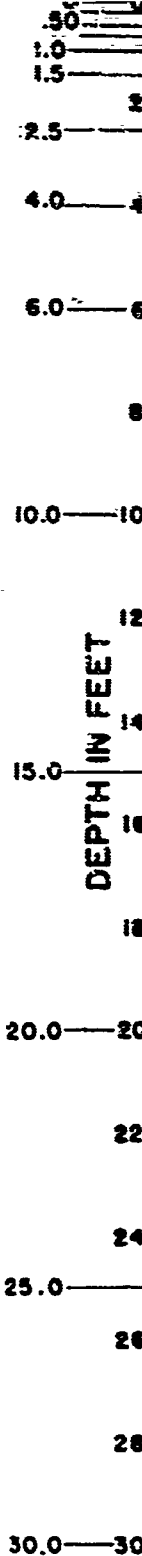
LEGEND

----- ZERO ISOTHERM

———— ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

DEPTHS OF THERMOCOUPLES IN FEET

DEPTH IN FEET



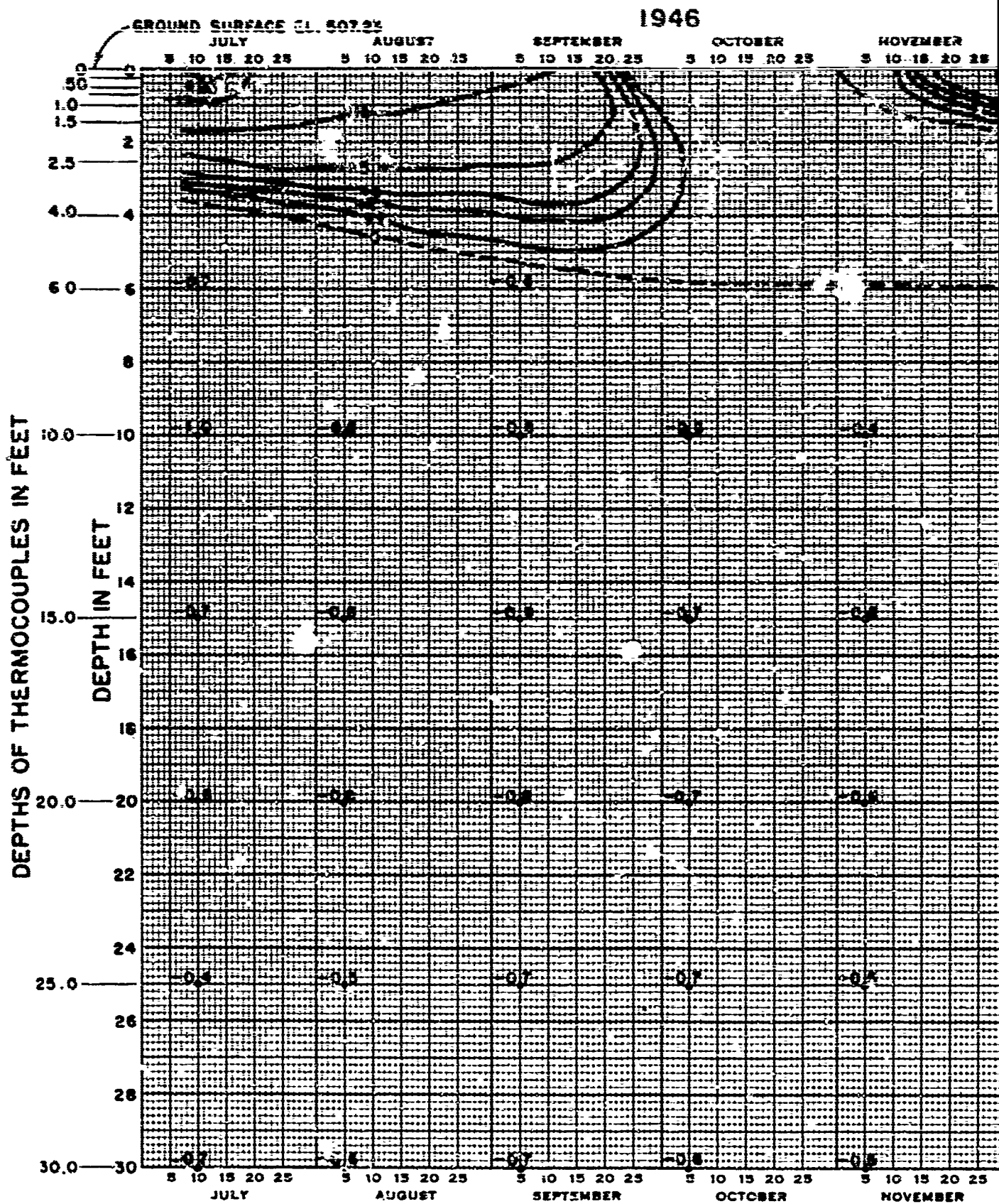
A

BRC, 1 PT.

BROWN ML
(WITH PT.
STRATIFIED)

GREY ML
WITH PT
(STRATIFIED)

GREY ML



PERMAFROST
FIELD RESEARCH-
AREA NO. 1

GROUND ISOTHERMS AND
19

U.S. ENGR. OFFICE ST. PAUL

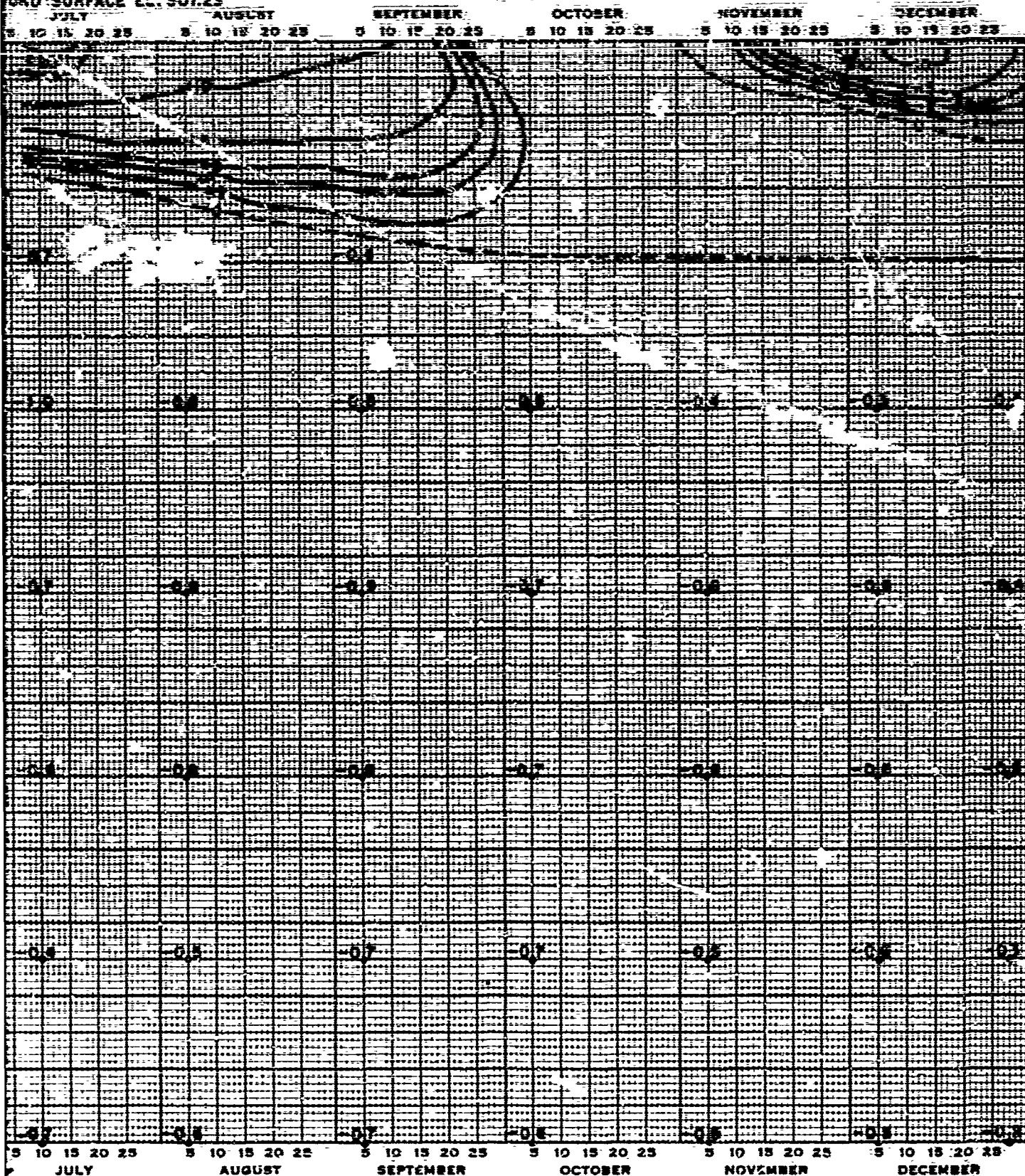
GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
LOCATION OF GROUND TEMPERATURE HOLE
SHOWN ON PLATE III - 5

B

UND SURFACE EL. 507.23

1946



GENERAL NOTES:

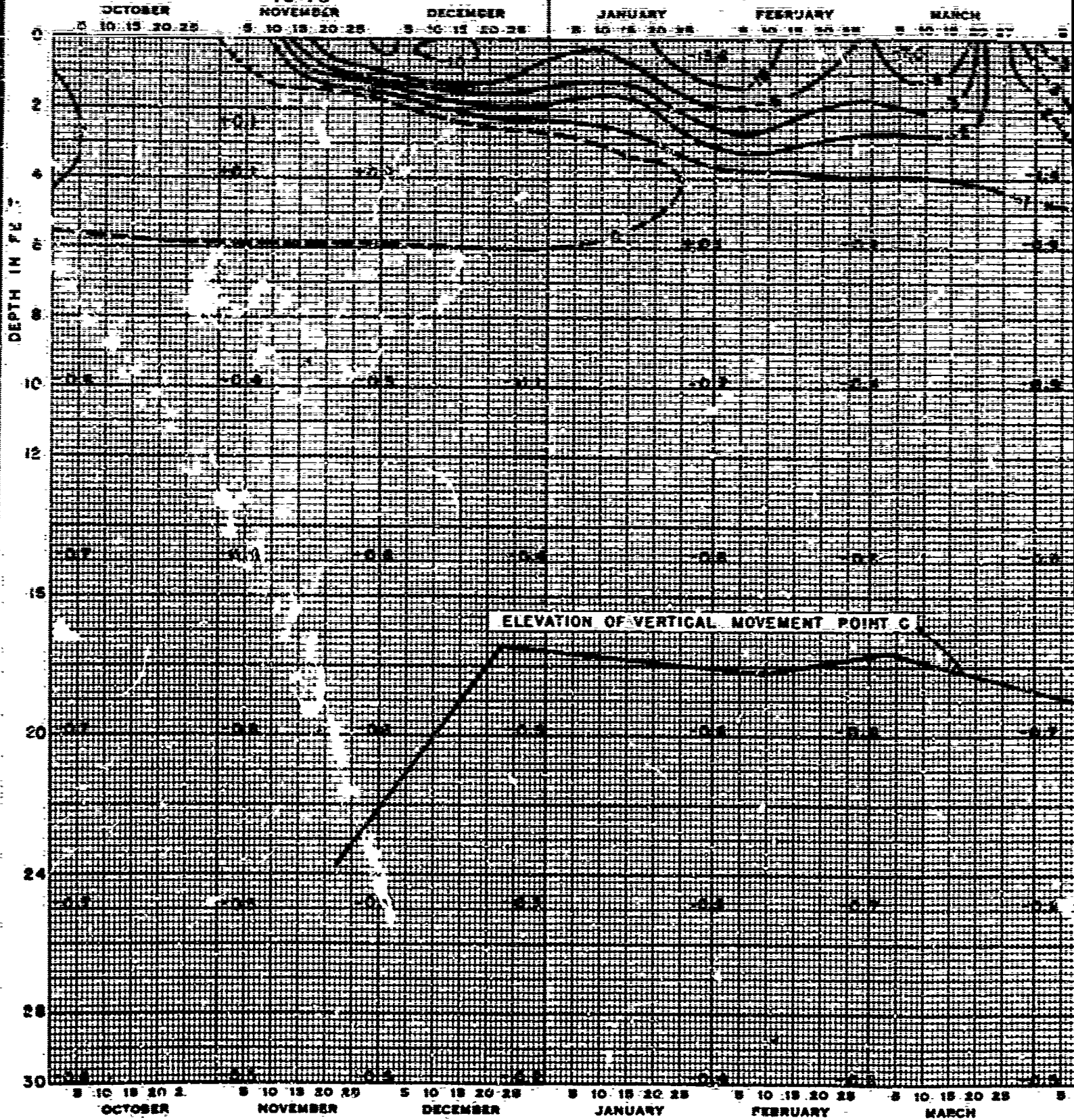
ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
LOCATION OF GROUND TEMPERATURE HOLE
SHOWN ON PLATE III-5

PERMAFROST INVESTIGATION
FIELD RESEARCH- FAIRBANKS ALASKA
AREA NO. I-HOLE NO. C
GROUND ISOTHERMS AND SURFACE ELEVATIONS
1946

U.S. ENGR. OFFICE ST. PAUL, MINN. MAY 1950

PLATE III-15

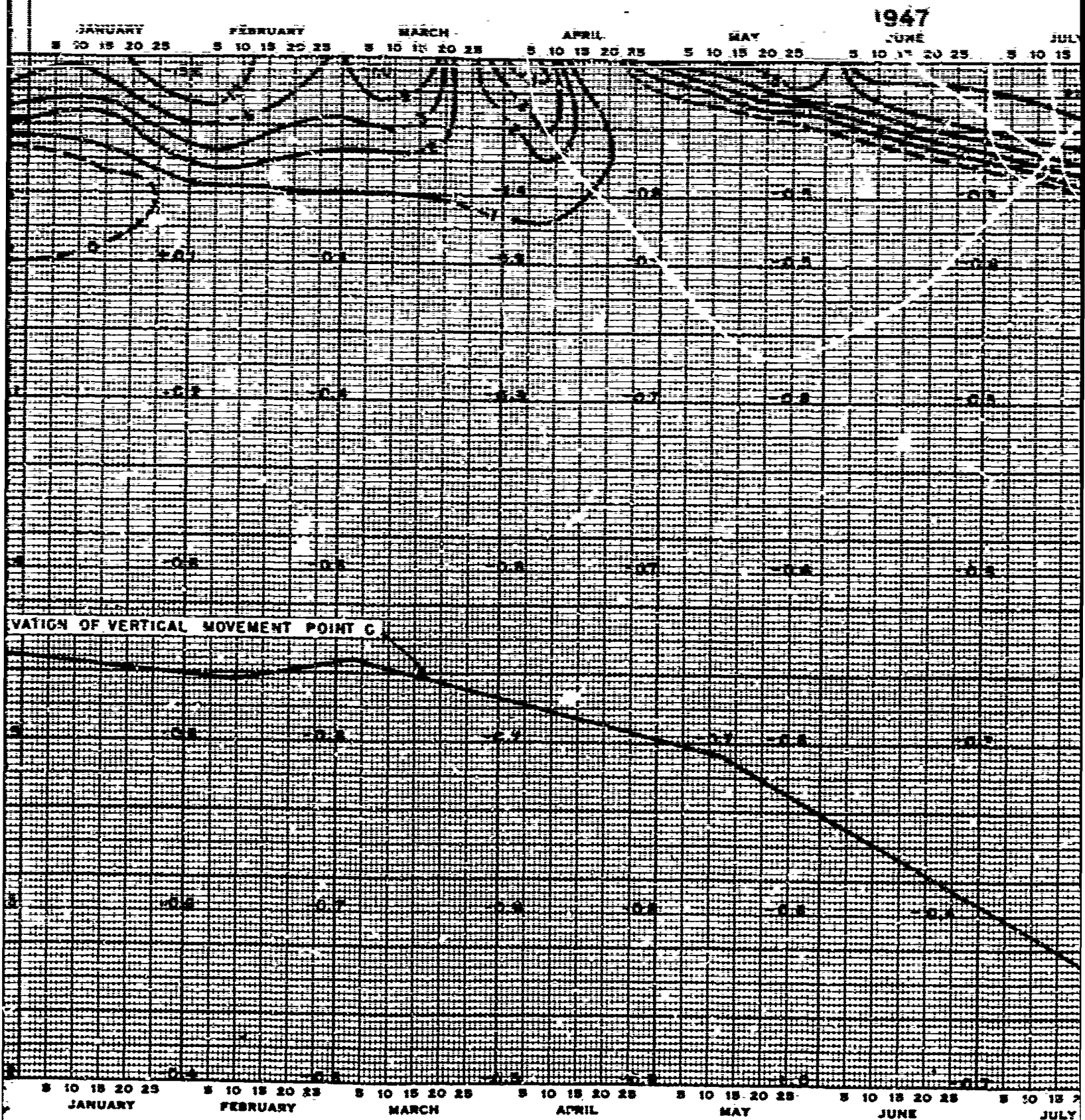
1946



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO (ABOVE OR BELOW AS MARKED)

A



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

B

GENERAL NOTES:

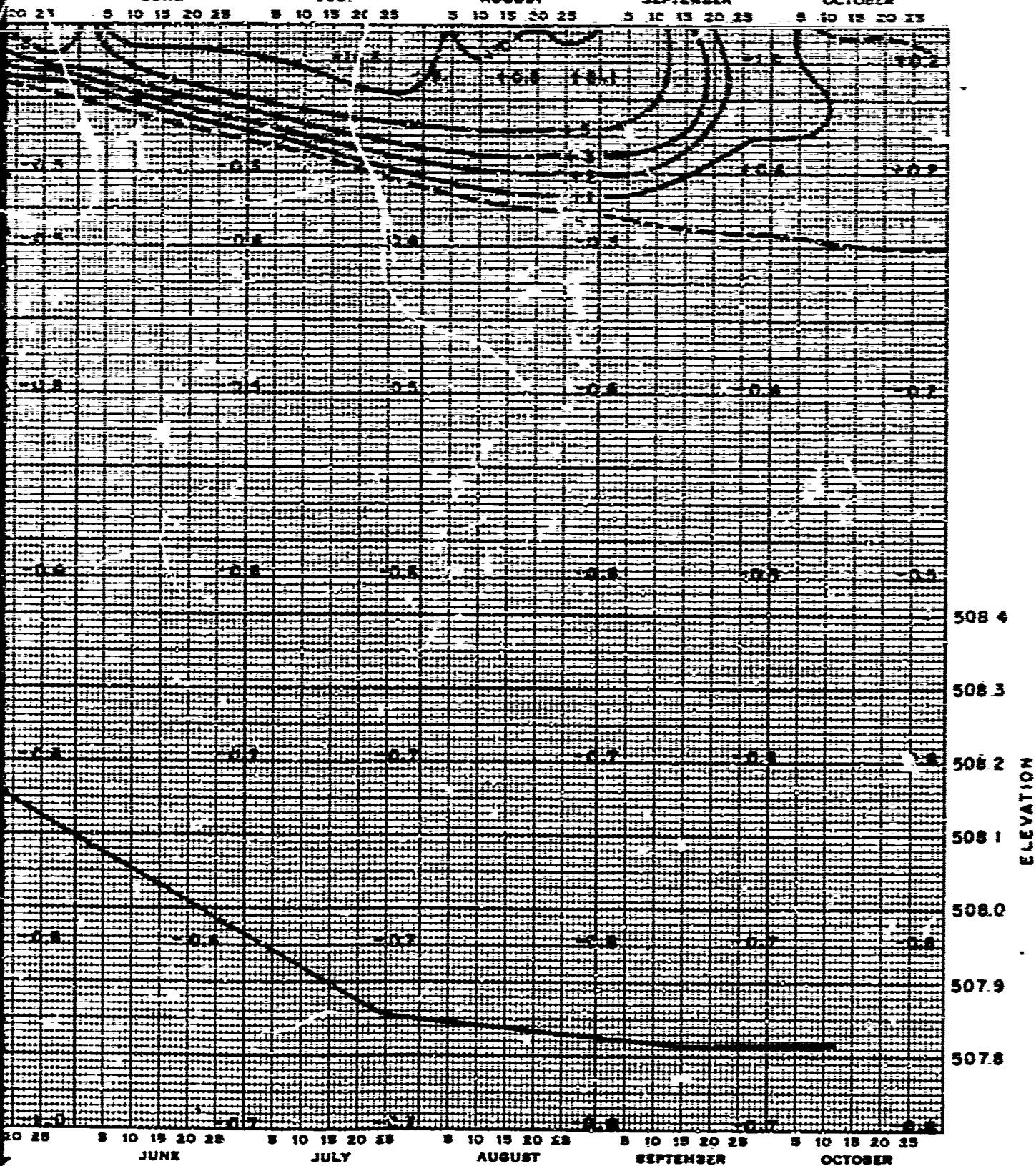
ISOTHERMS SHOWN IN DEGREES CENTIGRADE
SECTION C HAS BEEN CLEARED AND STRIPPED
OF ALL VEGETATION AND TOP SOIL EXPOSING
UNDERLYING SILT.

LINE

三三三

●●●●●

九、研究不足及未来展望



NOTES:

TEMPERATURES SHOWN IN DEGREES CENTIGRADE.
AREA HAS BEEN CLEARED AND STRIPPED
OF VEGETATION AND TOP SOIL EXPOSING
RED SILT.

PERMAFROST INVESTIGATION

FIELD RESEARCH-FAIRBANKS, ALASKA

AREA NO. 1 - HOLE NO. C

GROUND ISOTHERMS AND SURFACE ELEVATIONS 1946-47

CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

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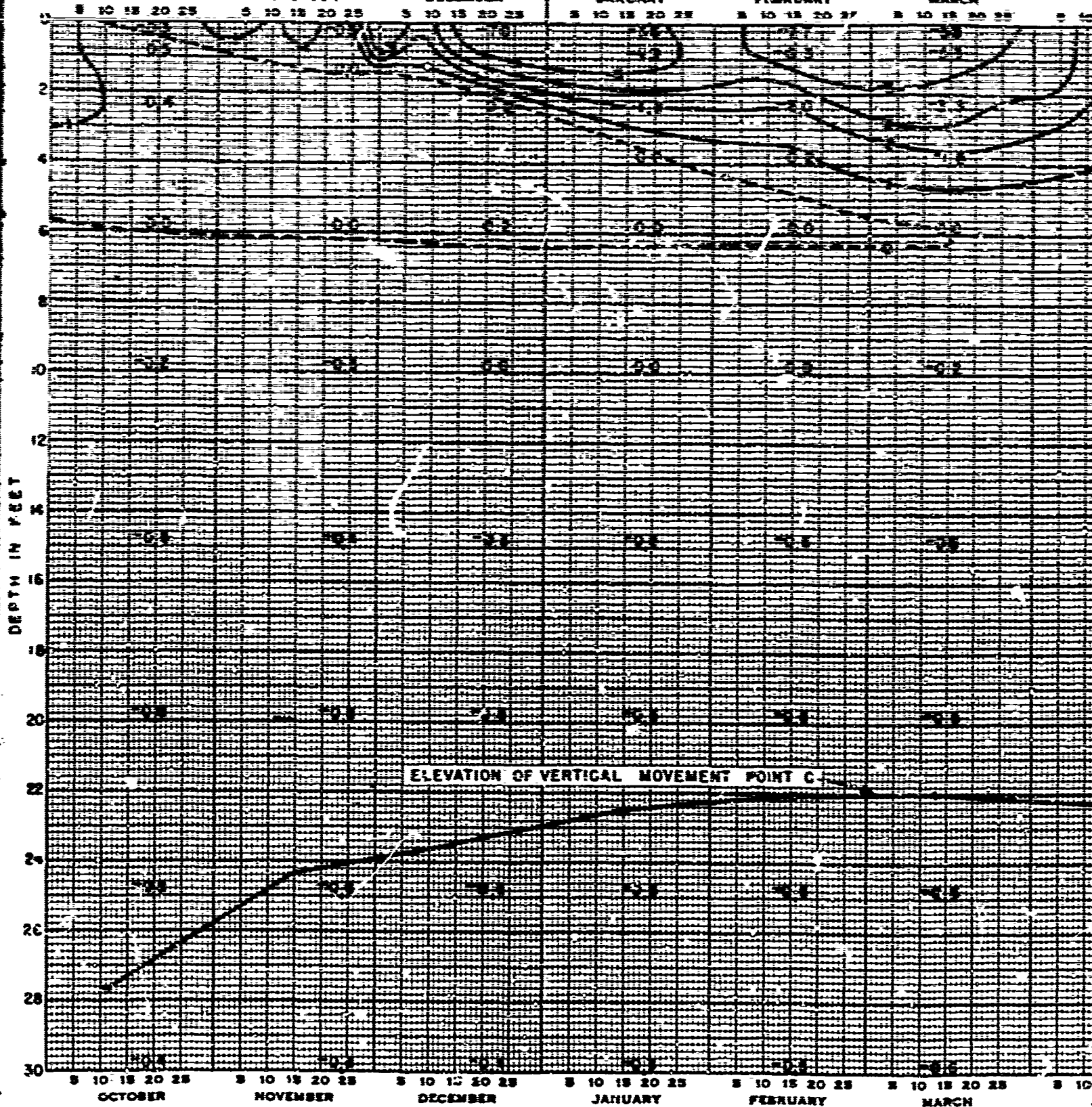
【例题·多选题】

◆ 五、实施策略

[illegible]

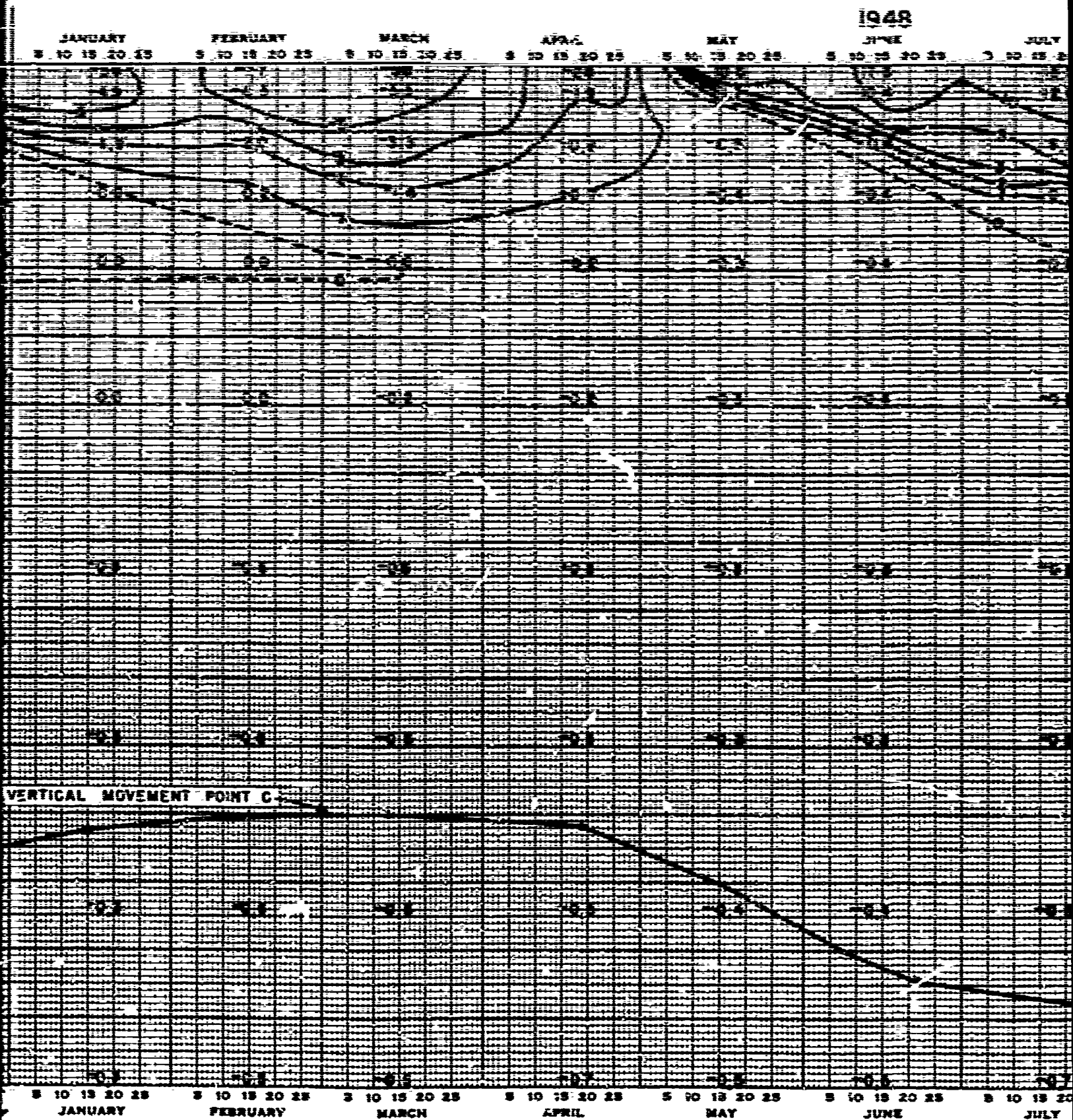
我生天已无我

参考文献



LEGEND

----- ZERO ISOTHERM
 _____ ISOTHERMS OTHER THAN ZERO
 (ABOVE OR BELOW AS MARKED)



LEGEND

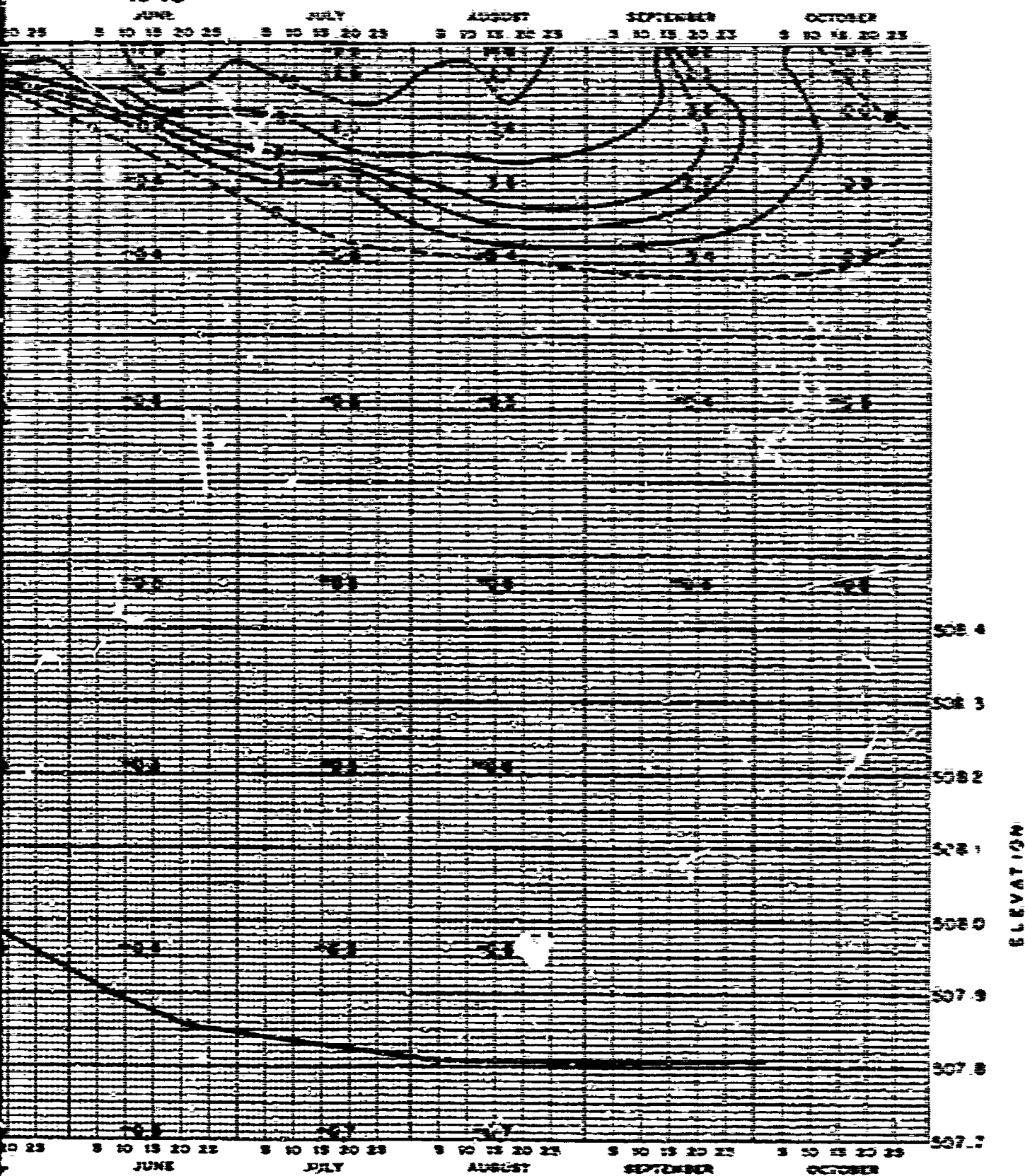
- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
SECTION C HAS BEEN CLEARED AND STRIPPED
OF ALL VEGETATION AND TOP SOIL EXPOSING
UNDERLYING SILT.

B

1948

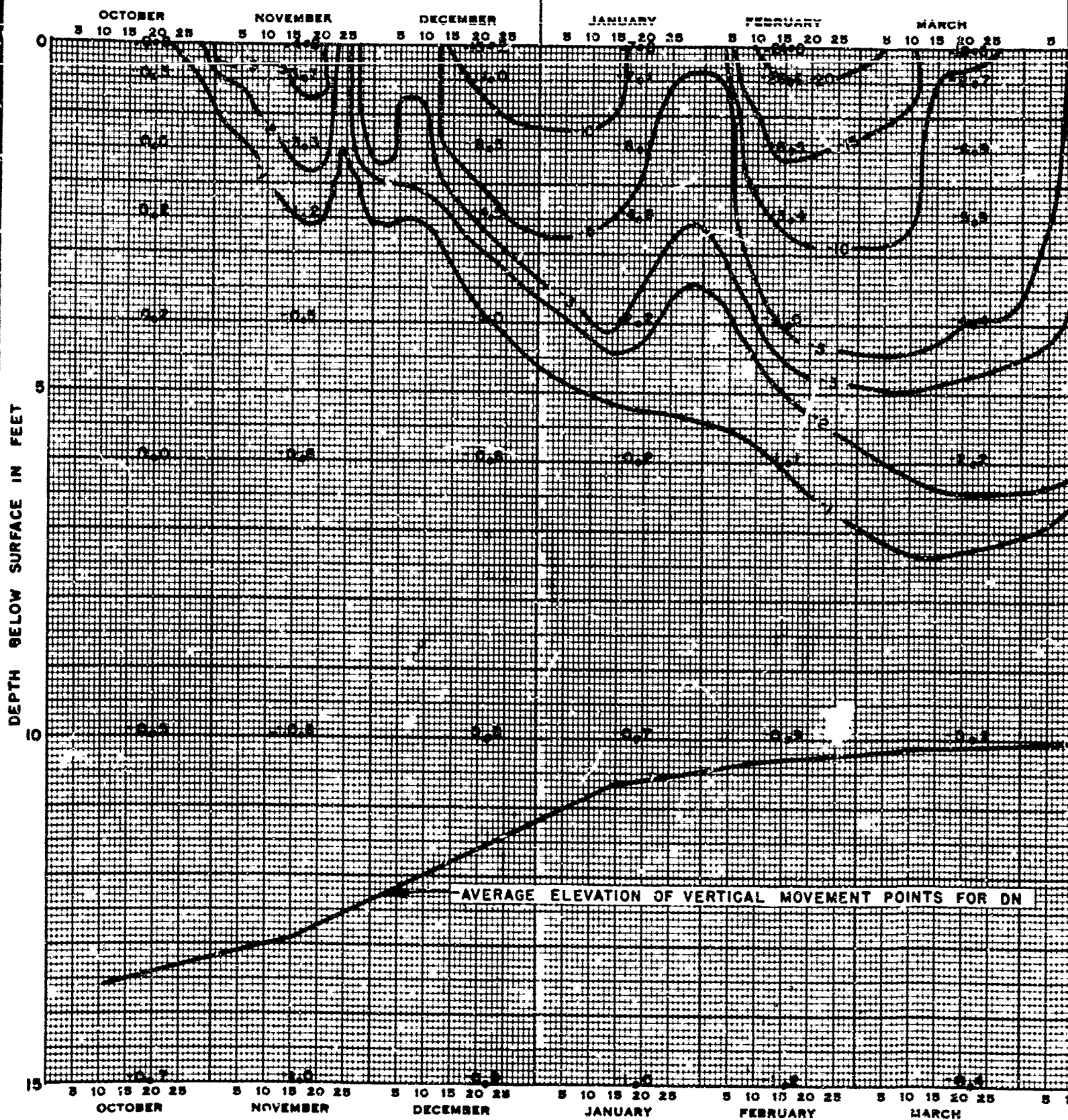


NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE.
 AREA NO. 1 HAS BEEN CLEARED AND STRIPPED
 VEGETATION AND TOP SOIL EXPOSING
 SILT.

PERMAFROST INVESTIGATION
 FIELD RESEARCH-FARSBANKS, ALASKA
 AREA NO. 1 - HOLE NO. C
 GROUND ISOTHERMS AND SURFACE ELEVATIONS
 1947-48
 CORPS OF ENGINEERS, ST. PETERSBURG, ALASKA MAY 1950

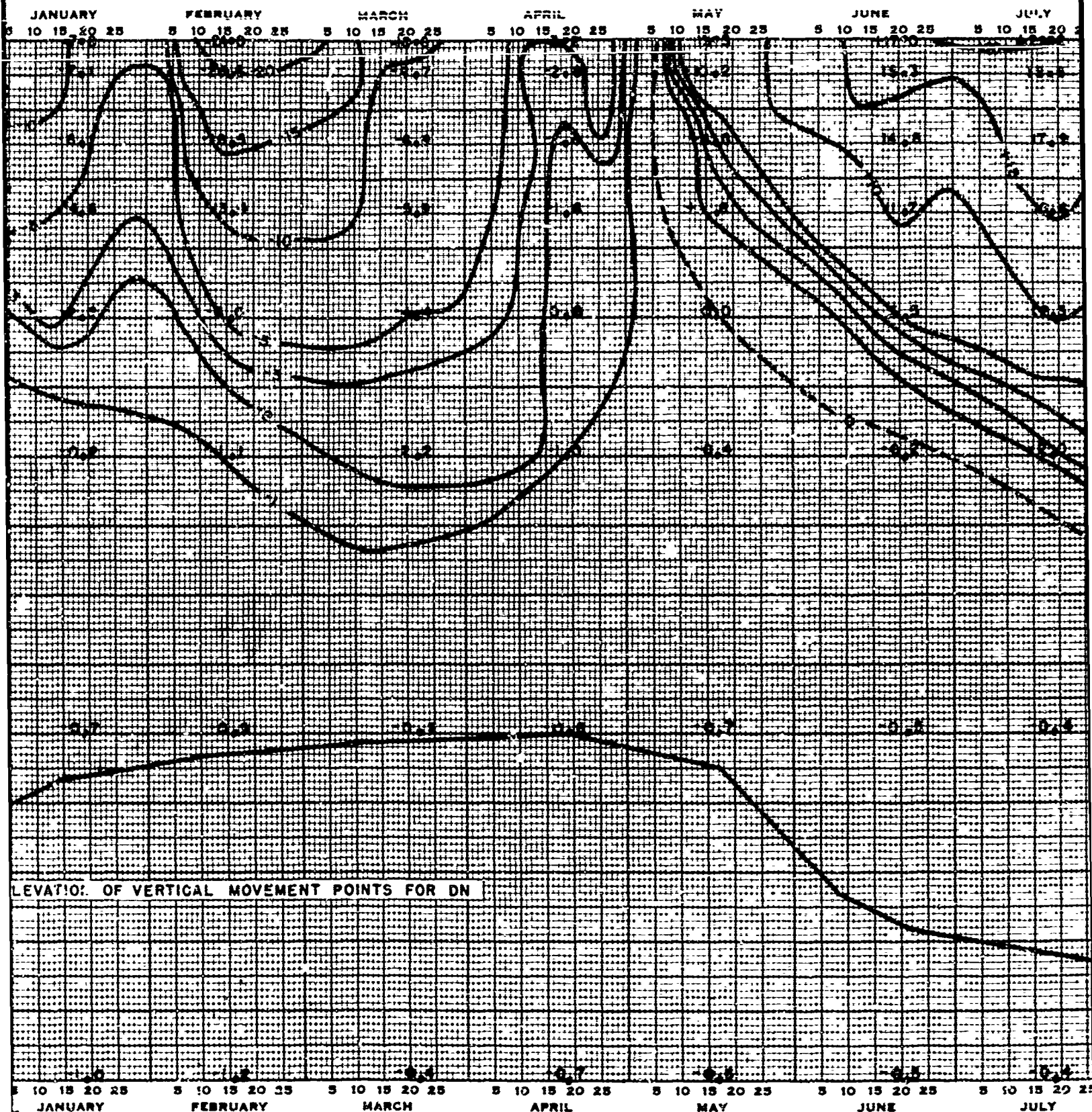
1947



LEGEND

- ZERO ISOTHERM
----- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

1948



LEGEND

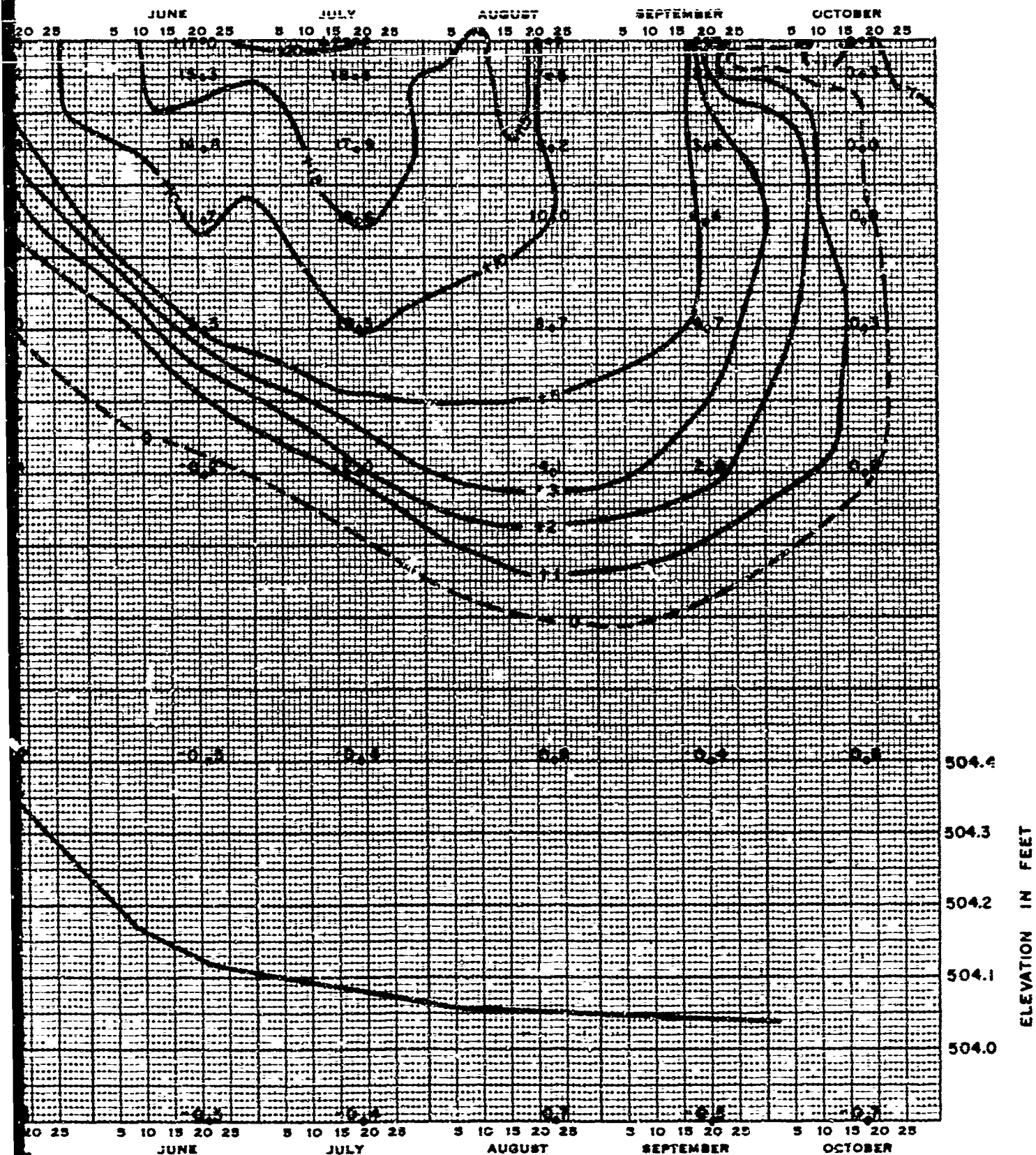
- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRADE

B

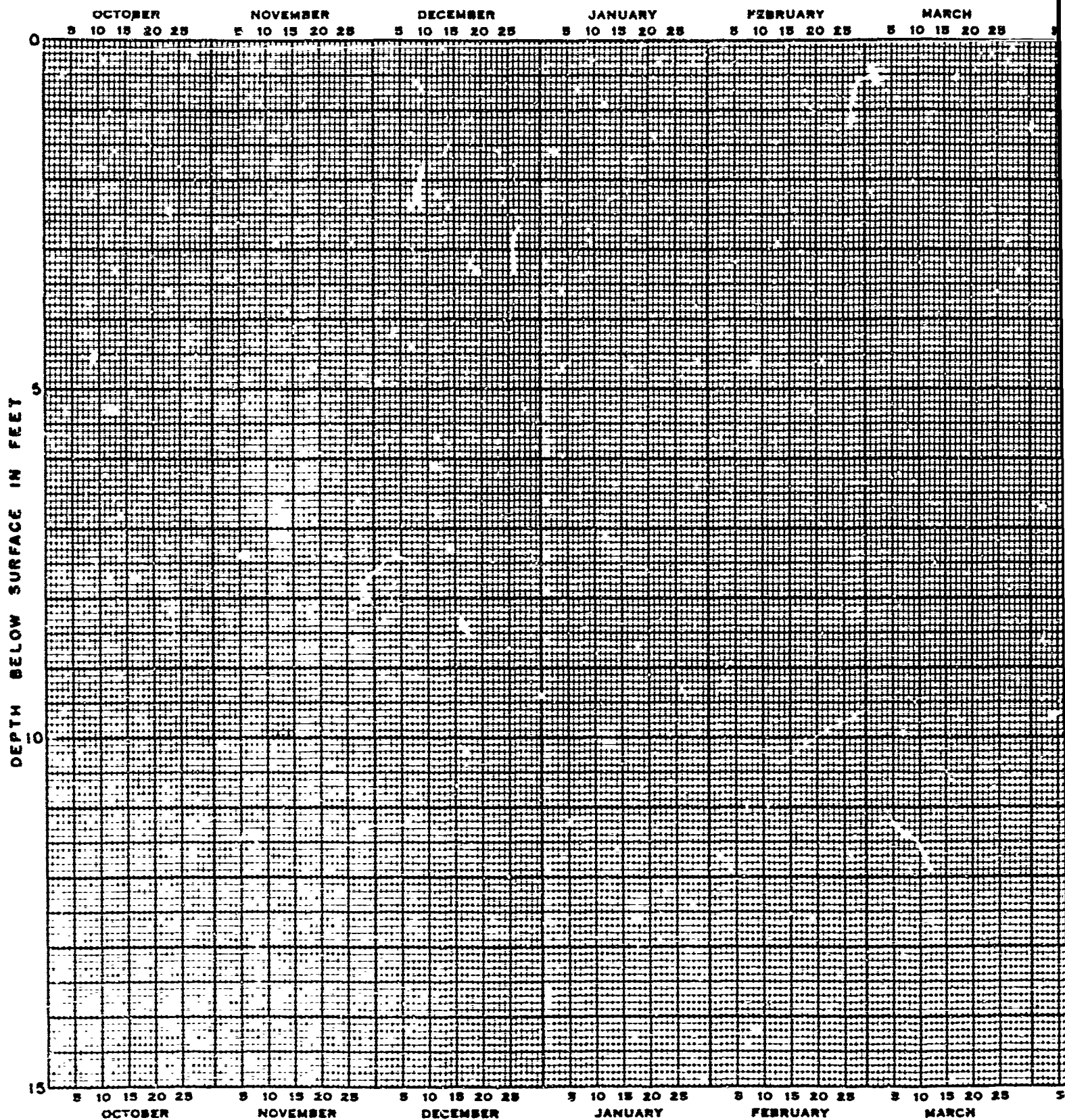
1948



NOTES:
ISOTHERMS SHOWN IN DEGREES CENTIGRADE

PERMAFROST INVESTIGATION
FIELD RESEARCH-FAIRBANKS, ALASKA
AREA NO. 1-HOLE NO. DN
GROUND ISOTHERMS AND SURFACE ELEVATIONS
1947 - 1948
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

PLATE III-18

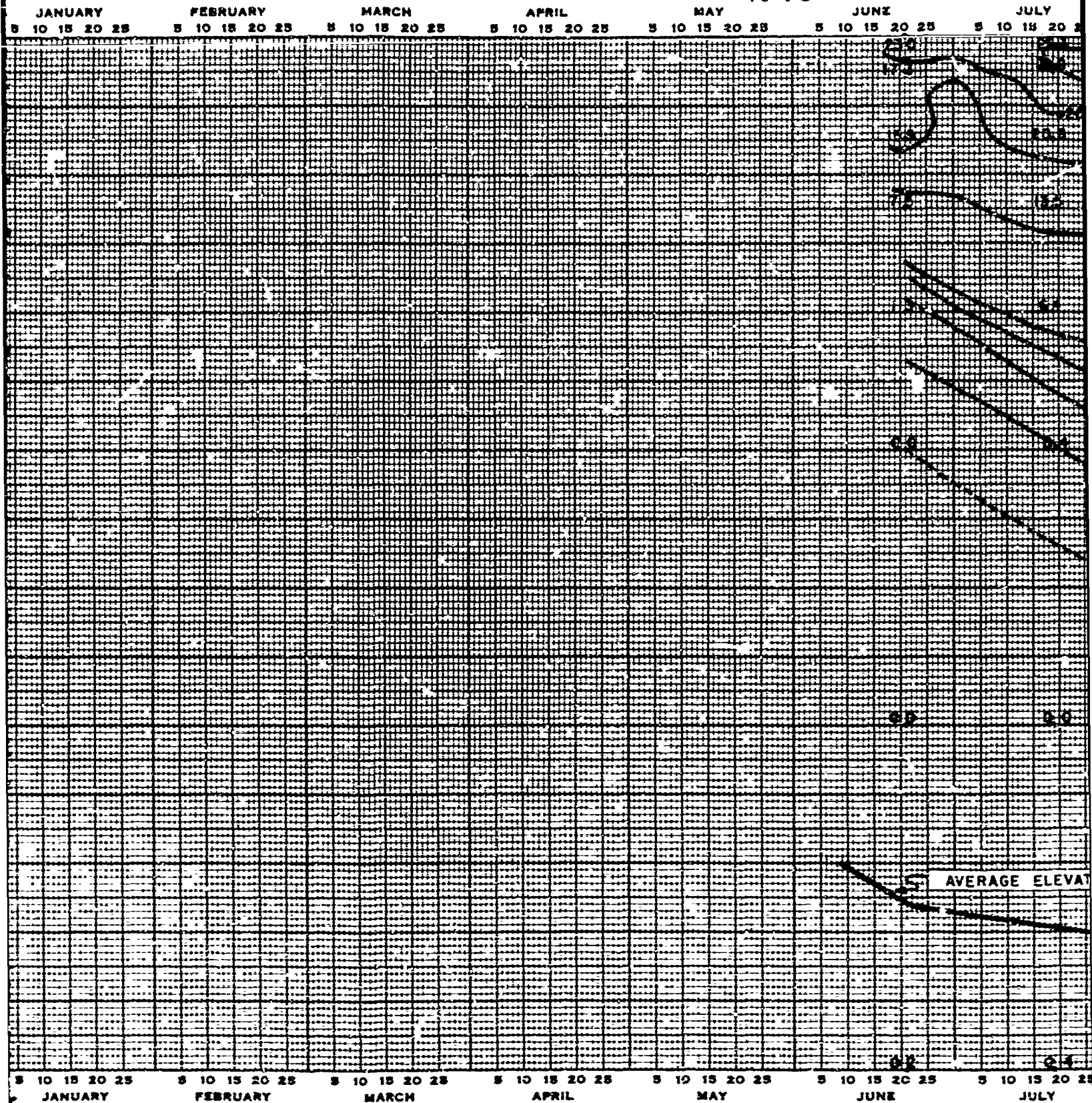


A

LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZER

1948



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO

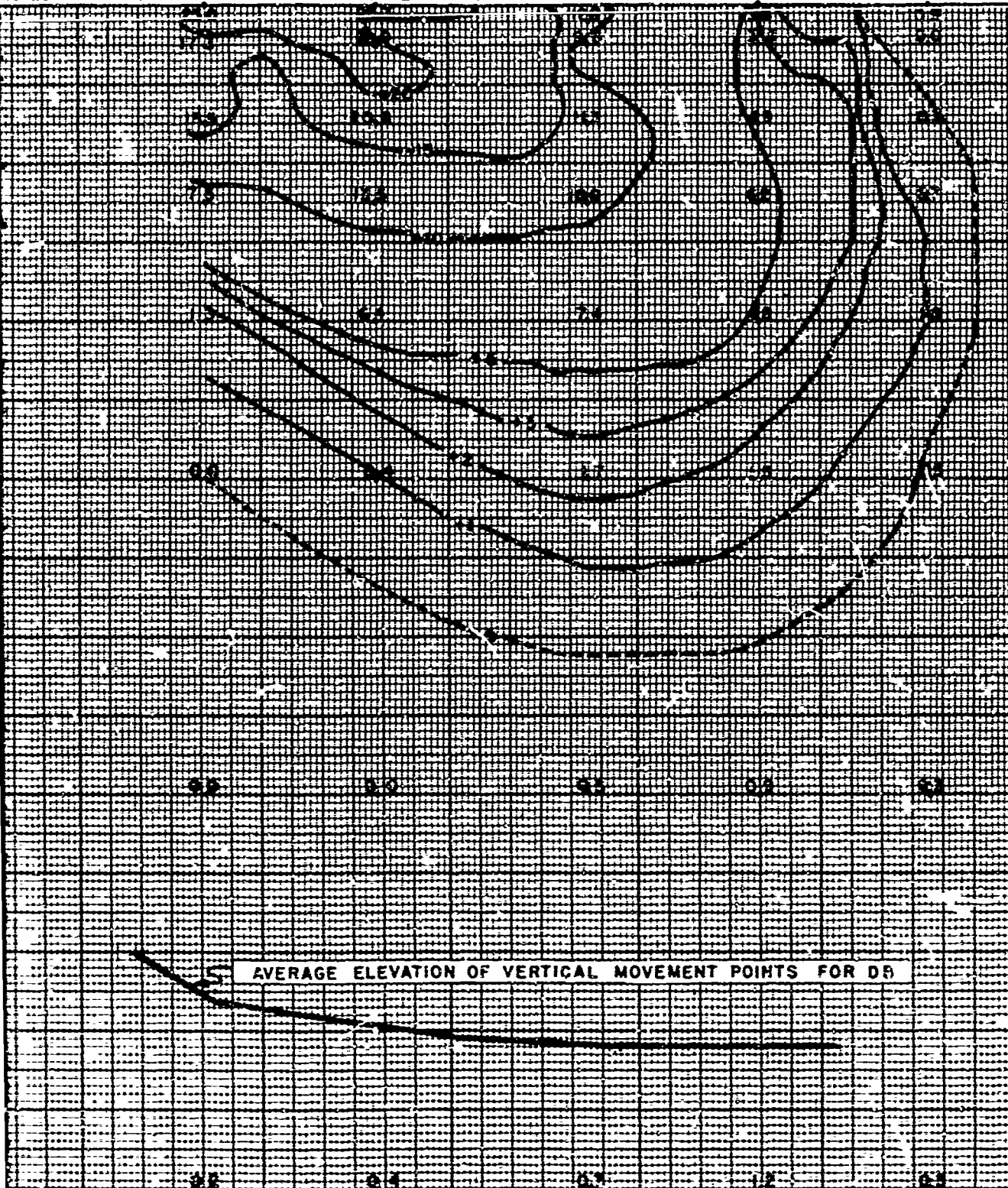
B

GENERAL NOTES

ISOTHERMS SHOWN IN DEGREES

1948

JUNE JULY AUGUST SEPTEMBER OCTOBER

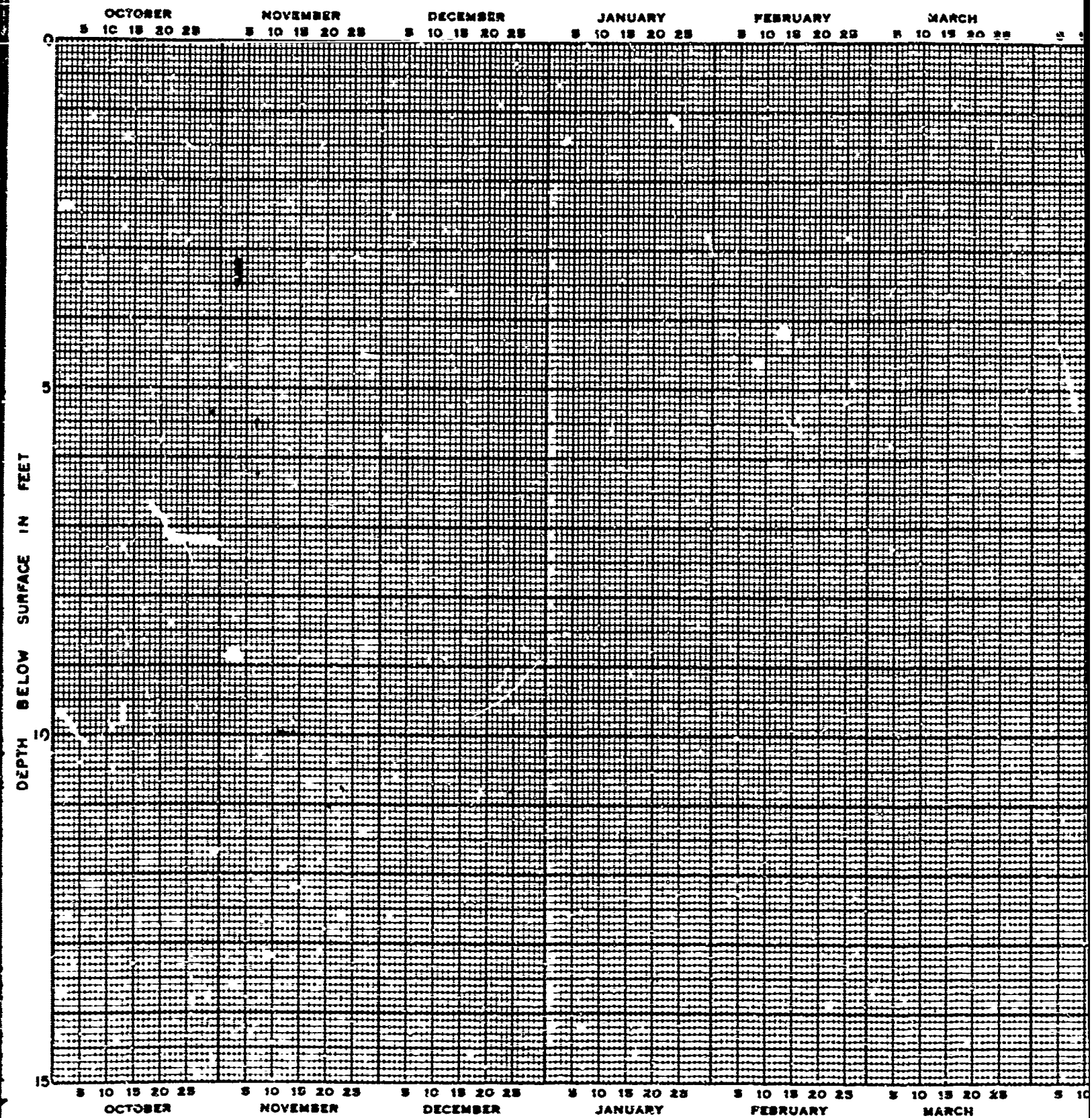


ELEVATION IN FEET

PERMAFROST INVESTIGATION
 FIELD RESEARCH - FAIRBANKS, ALASKA
 AREA NO 1 - HOLE NO. DB
 GROUND ISOTHERMS AND SURFACE ELEVATIONS
 1948

GENERAL NOTES
 ISOTHERMS SHOWN IN DEGREES CENTIGRADE

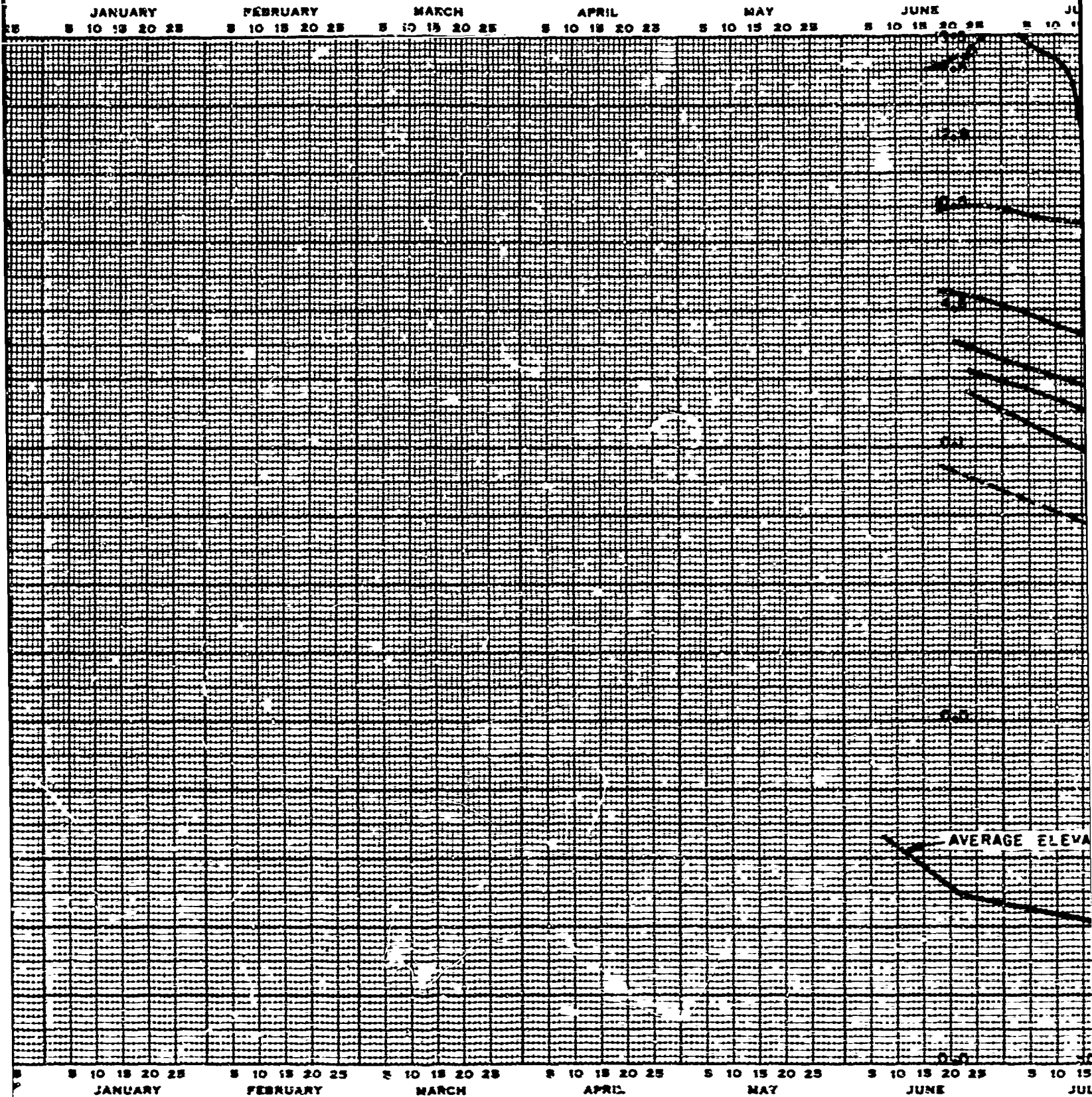
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950



LEGEND

- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

1948



LEGEND

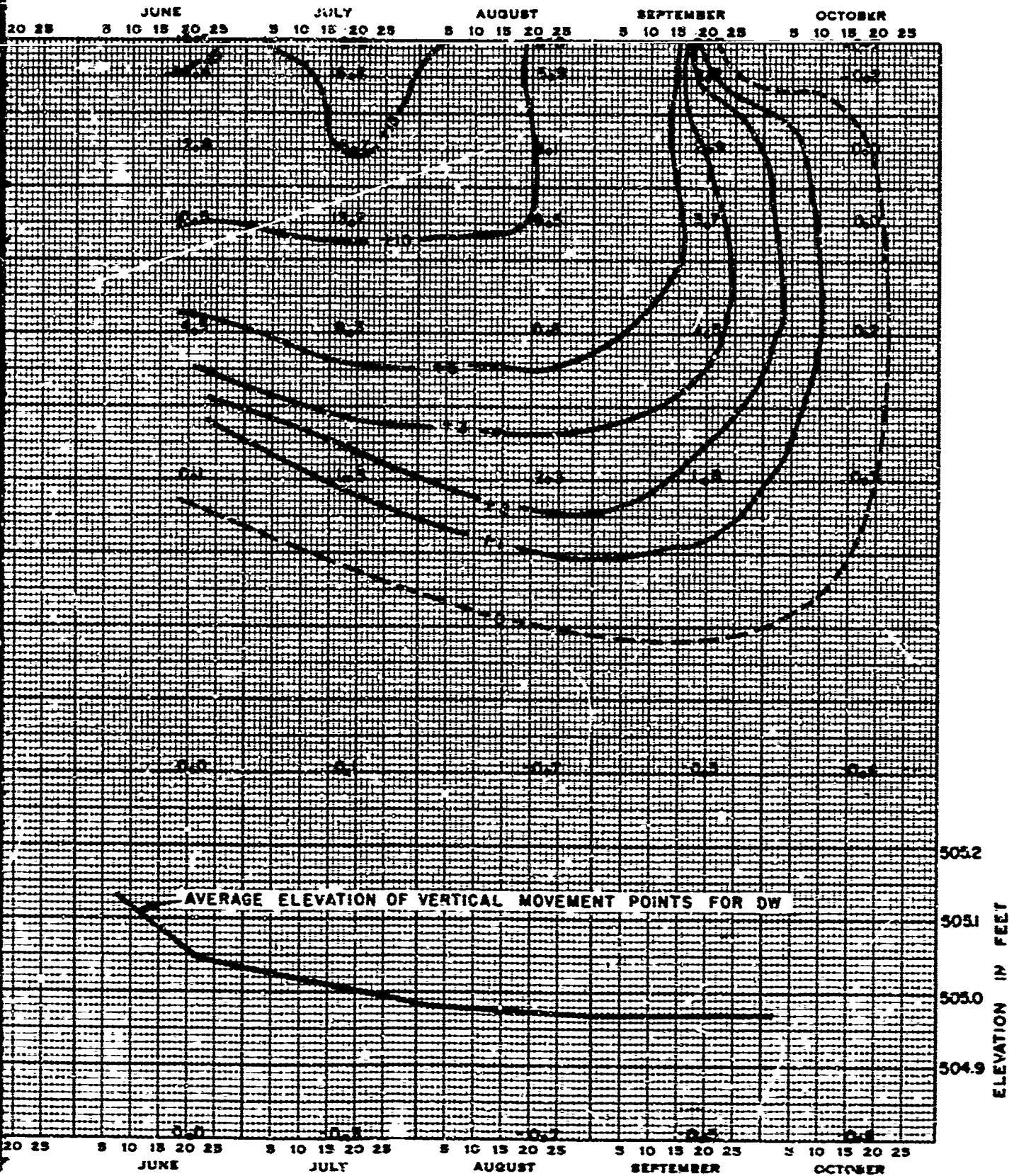
- ZERO ISOTHERM
- ISOTHERMS OTHER THAN ZERO
(ABOVE OR BELOW AS MARKED)

GENERAL NOTES:

ISOTHERMS SHOWN IN DEGREES CENTIGRAD

B

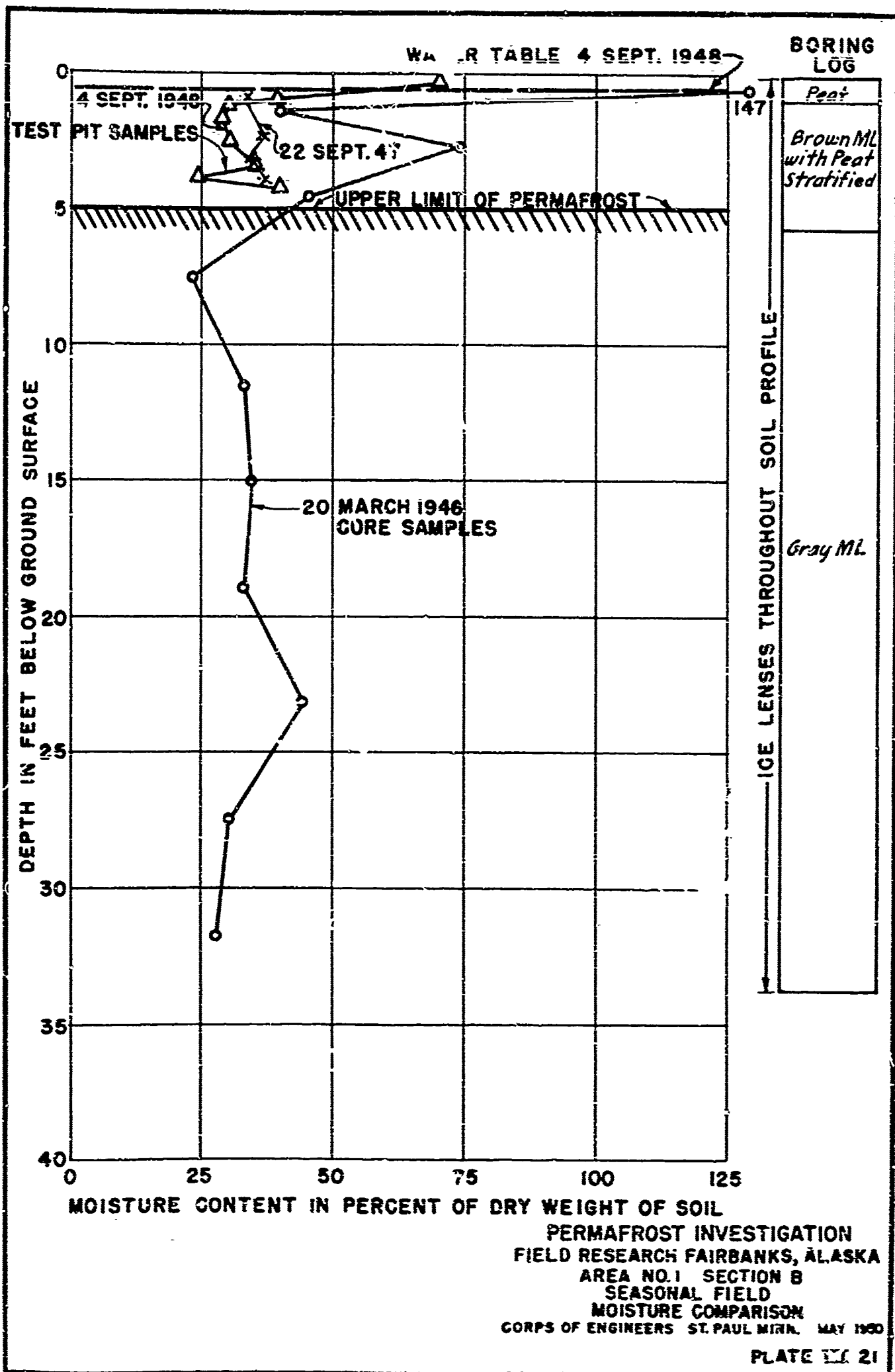
1948



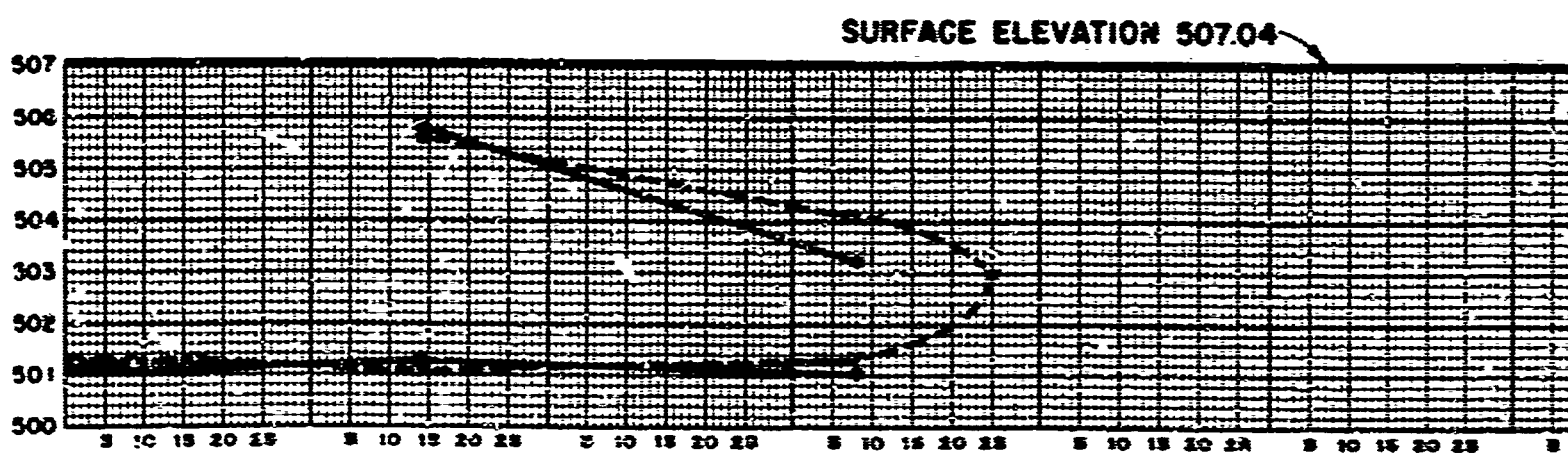
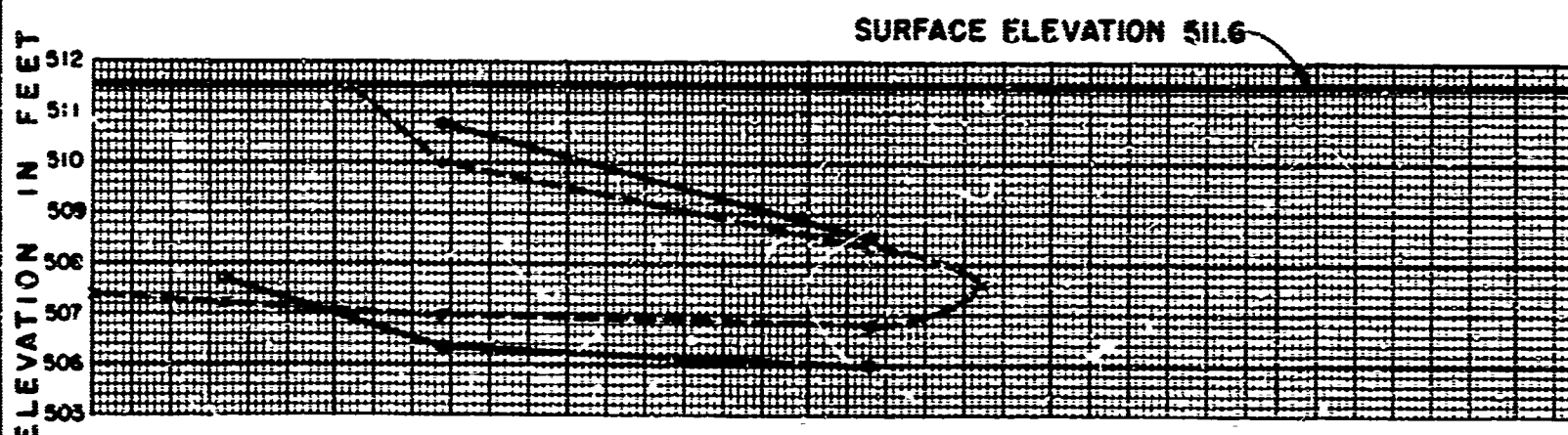
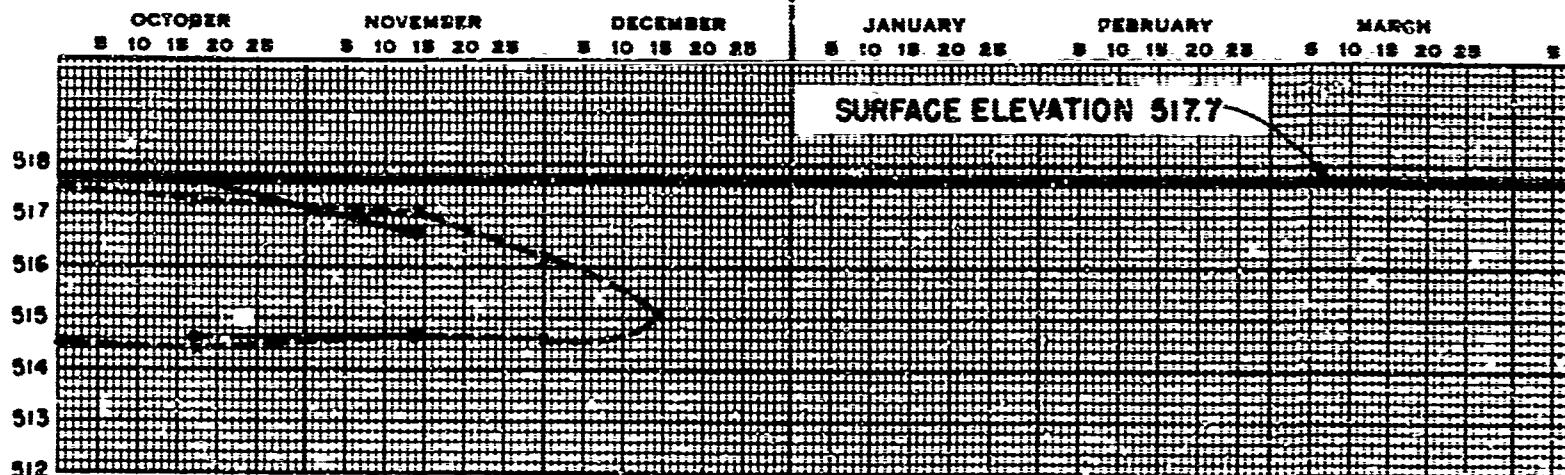
NOTES:
ISOTHERMS SHOWN IN DEGREES CENTIGRADE.

PERMAFROST INVESTIGATION
FIELD RESEARCH-FAIRBANKS, ALASKA
AREA NO. 1-HOLE NO. DW
GROUND ISOTHERMS AND SURFACE ELEVATIONS
1948

CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950



1946



A

1947

JANUARY FEBRUARY MARCH APRIL MAY JUNE JULY

SURFACE ELEVATION 517.7

POINT A

SURFACE ELEVATION 511.6

POINT B

SURFACE ELEVATION 507.04

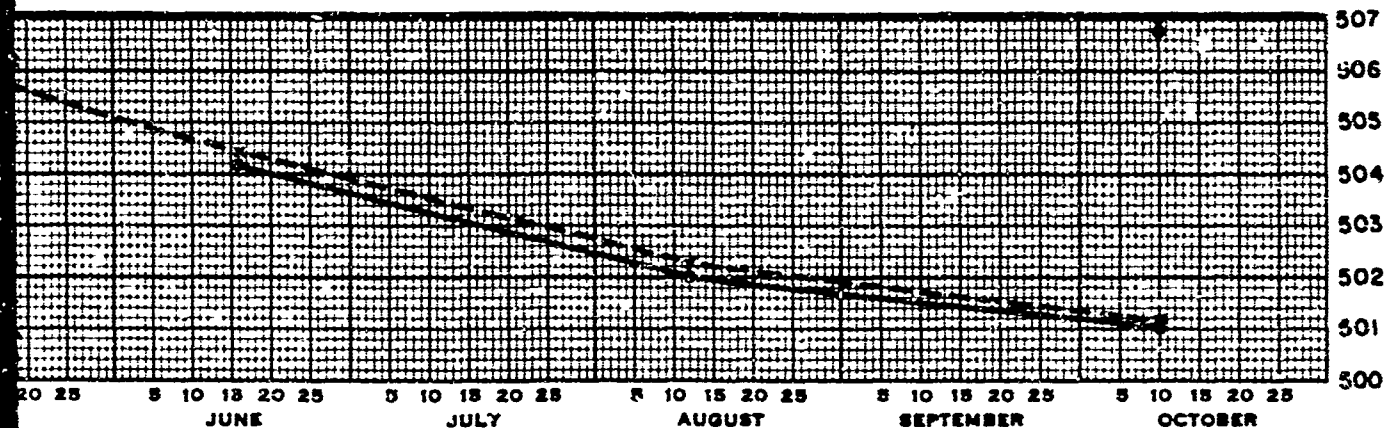
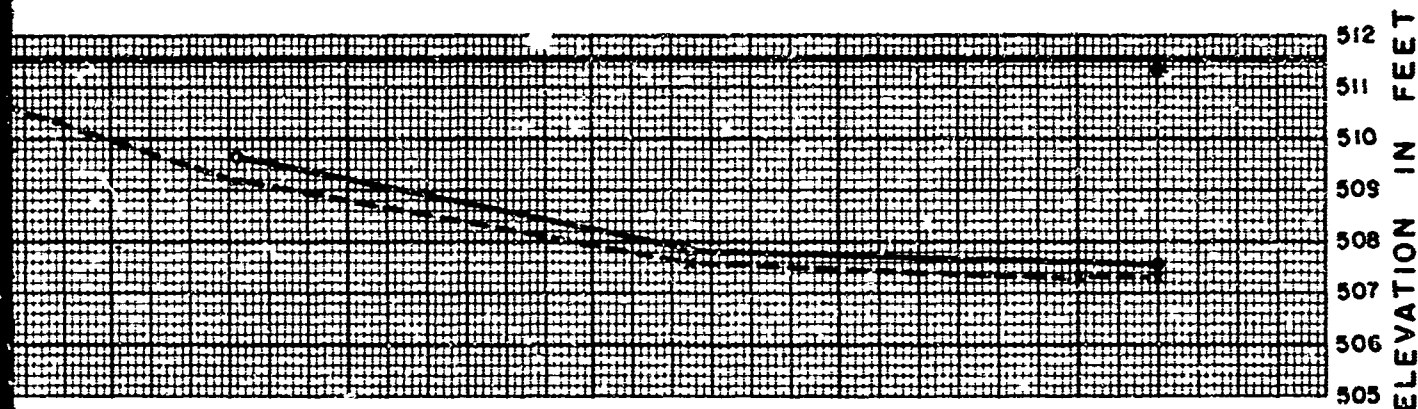
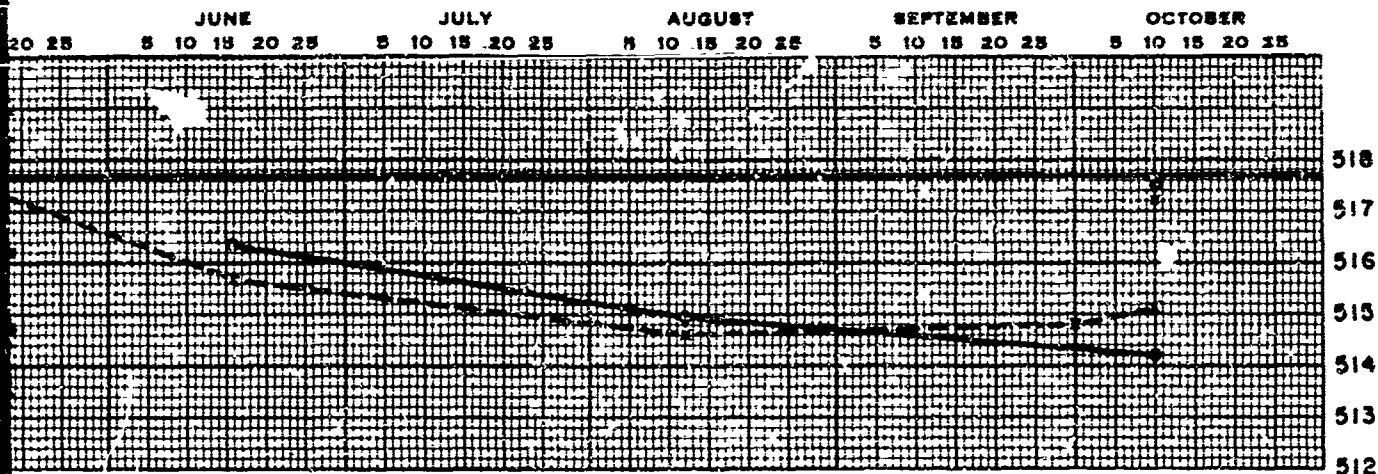
POINT C

LEGEND

- FROST LINE AND PERMAFROST TABLE AS DETERMINED BY PROSING.
- x— ZERO DEGREE CENTIGRADE ISOTHERM AS DETERMINED BY GROUND TEMPERATURE READING.

3

1947



LEGEND

SOLID LINE AND PERMAFROST TABLE
 DETERMINED BY PROBING.
 DASHED LINE AND PERMAFROST TABLE
 DETERMINED BY GROUND TEMPERATURE
 PROBING.

PERMAFROST INVESTIGATION FIELD RESEARCH, FAIRBANKS, ALASKA AREA NO. 1

DEPTH OF THAW OBTAINED BY
 PROBING- GROUND TEMPERATURES

1946 - 47

CORPS OF ENGINEERS, ST. PAUL, MINN. MAY, 1950

1947

OCTOBER

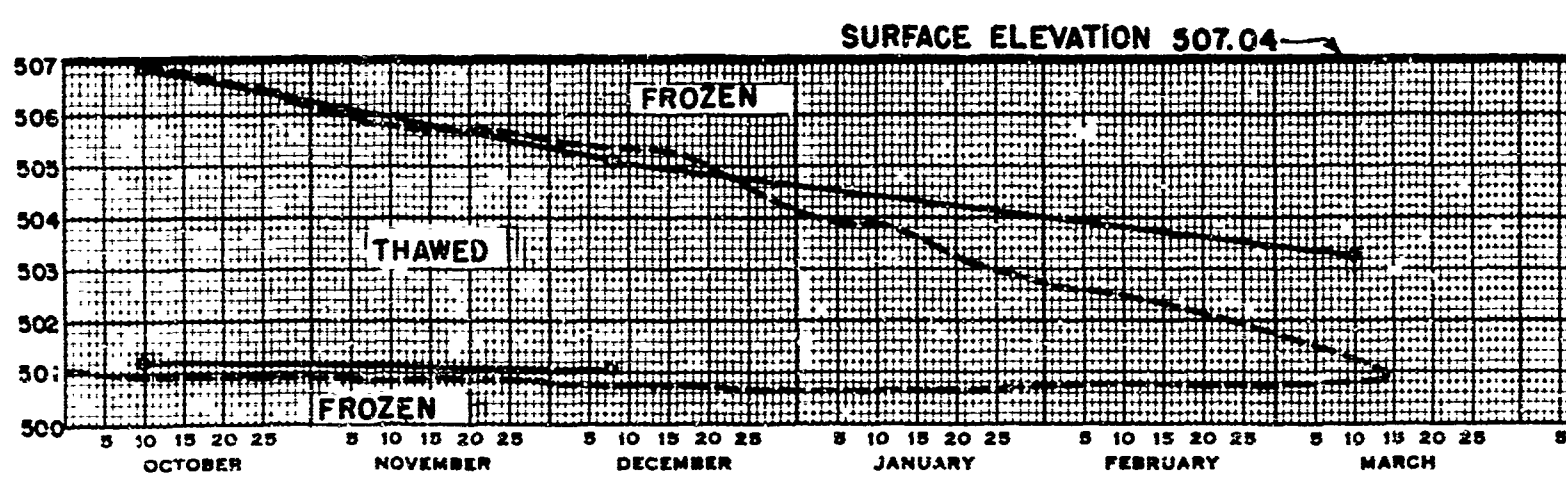
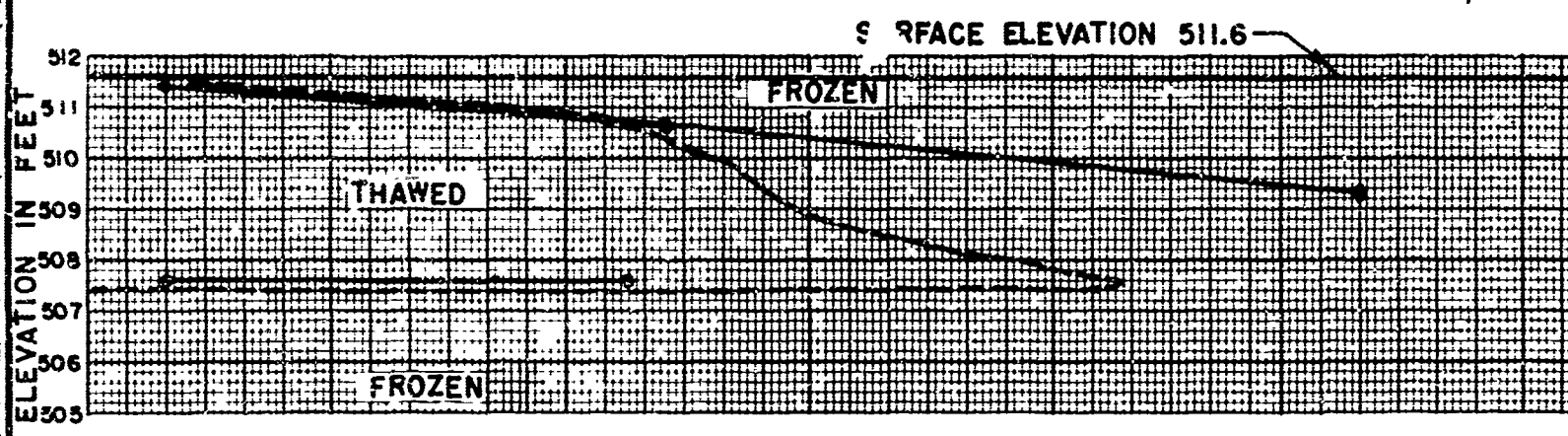
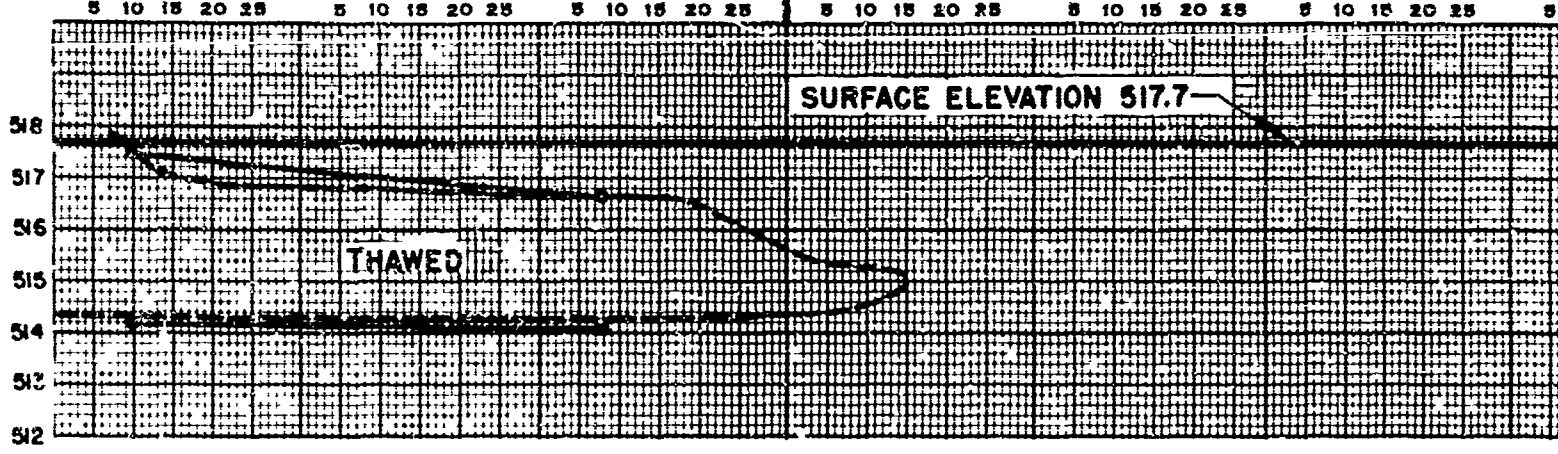
NOVEMBER

DECEMBER

JANUARY

FEBRUARY

MARCH



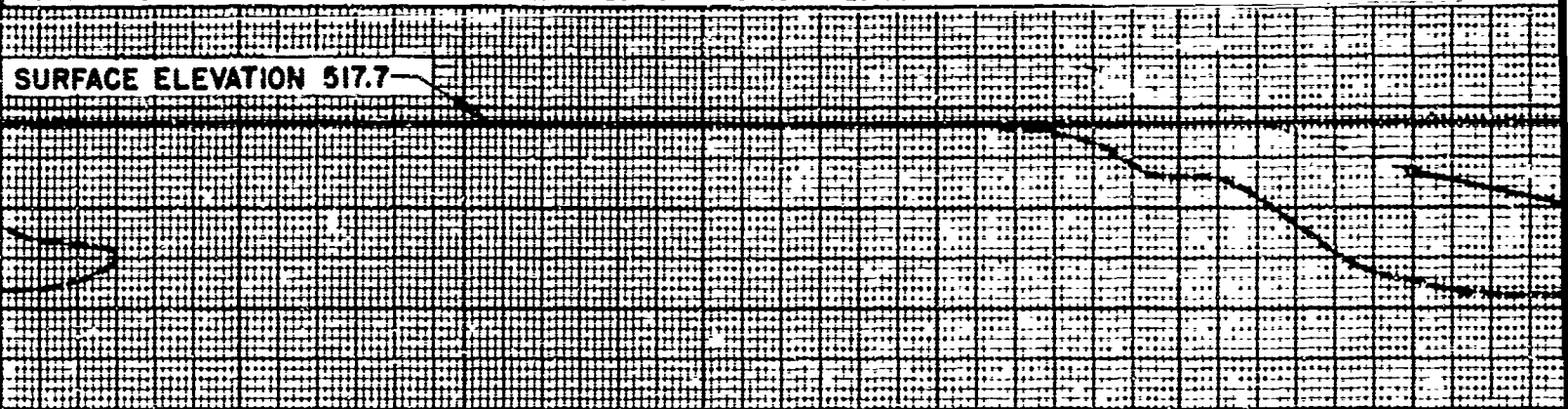
A

LEGEND

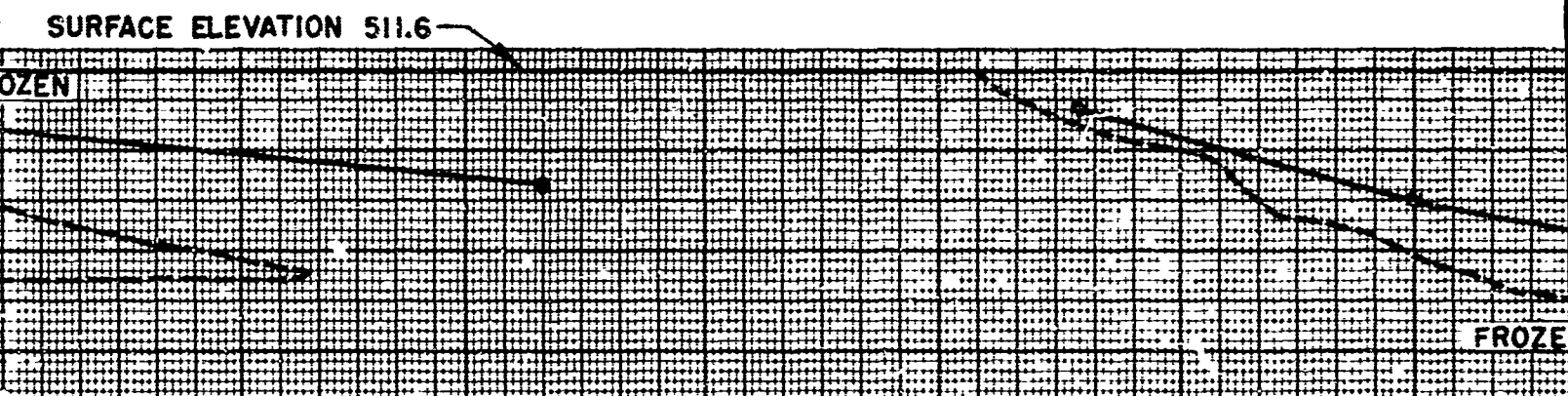
- FROST LINE AND PERMAFROST TABLE AS DETERMINED BY PROBING
- ZERO DEGREE CENTIGRADE ISOTHERMS AS DETERMINED BY GROUND TEMPERATURE READING

194

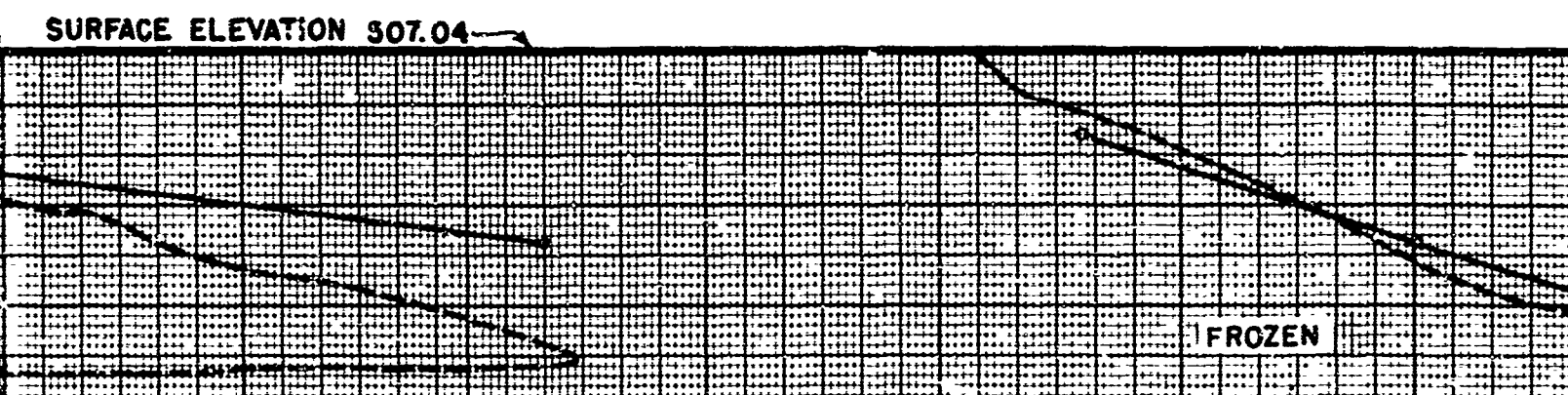
JANUARY FEBRUARY MARCH APRIL MAY JUNE JULY



POINT A



POINT B



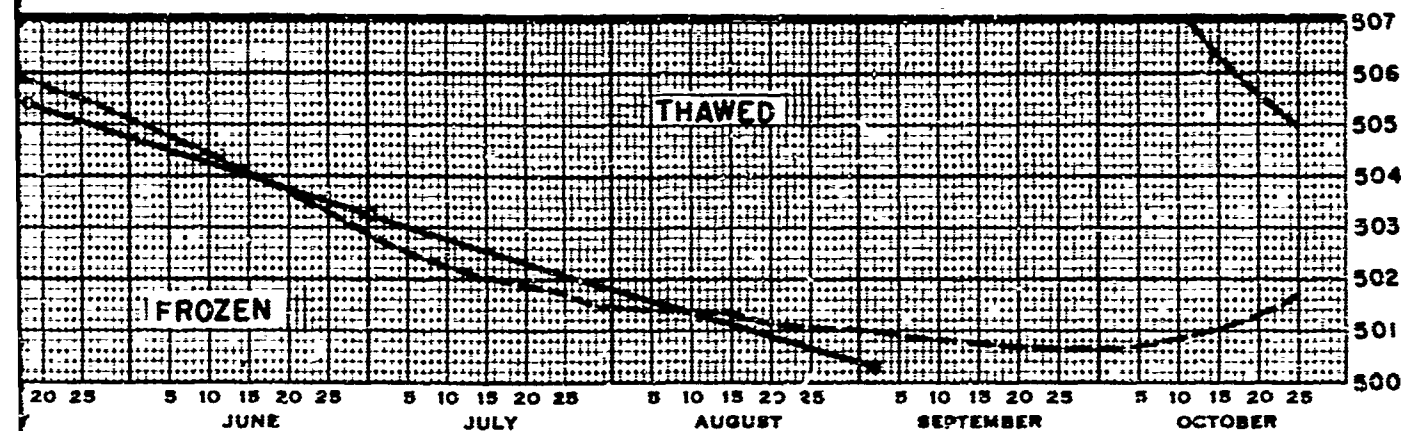
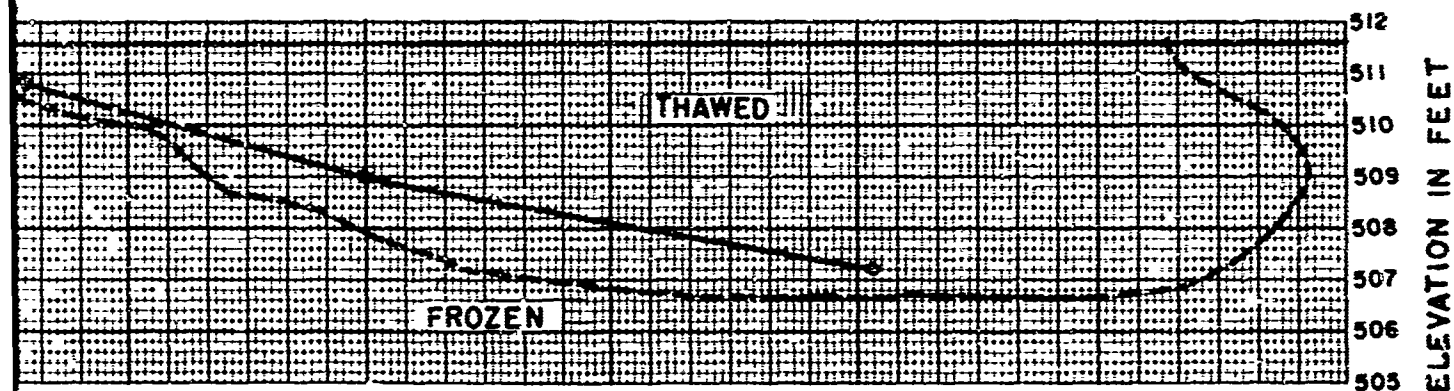
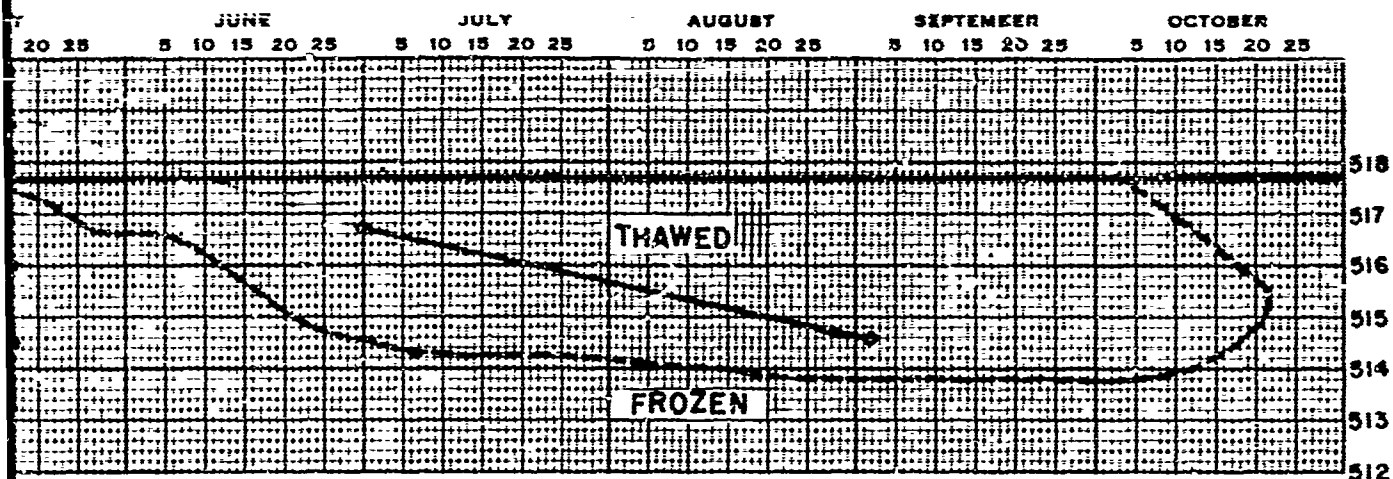
POINT C

LEGEND

- FROST LINE AND PERMAFROST TABLE AS DETERMINED BY PROBING
- - - - - ZERO DEGREE CENTIGRADE ISOTHERMS AS DETERMINED BY GROUND TEMPERATURE READING

B

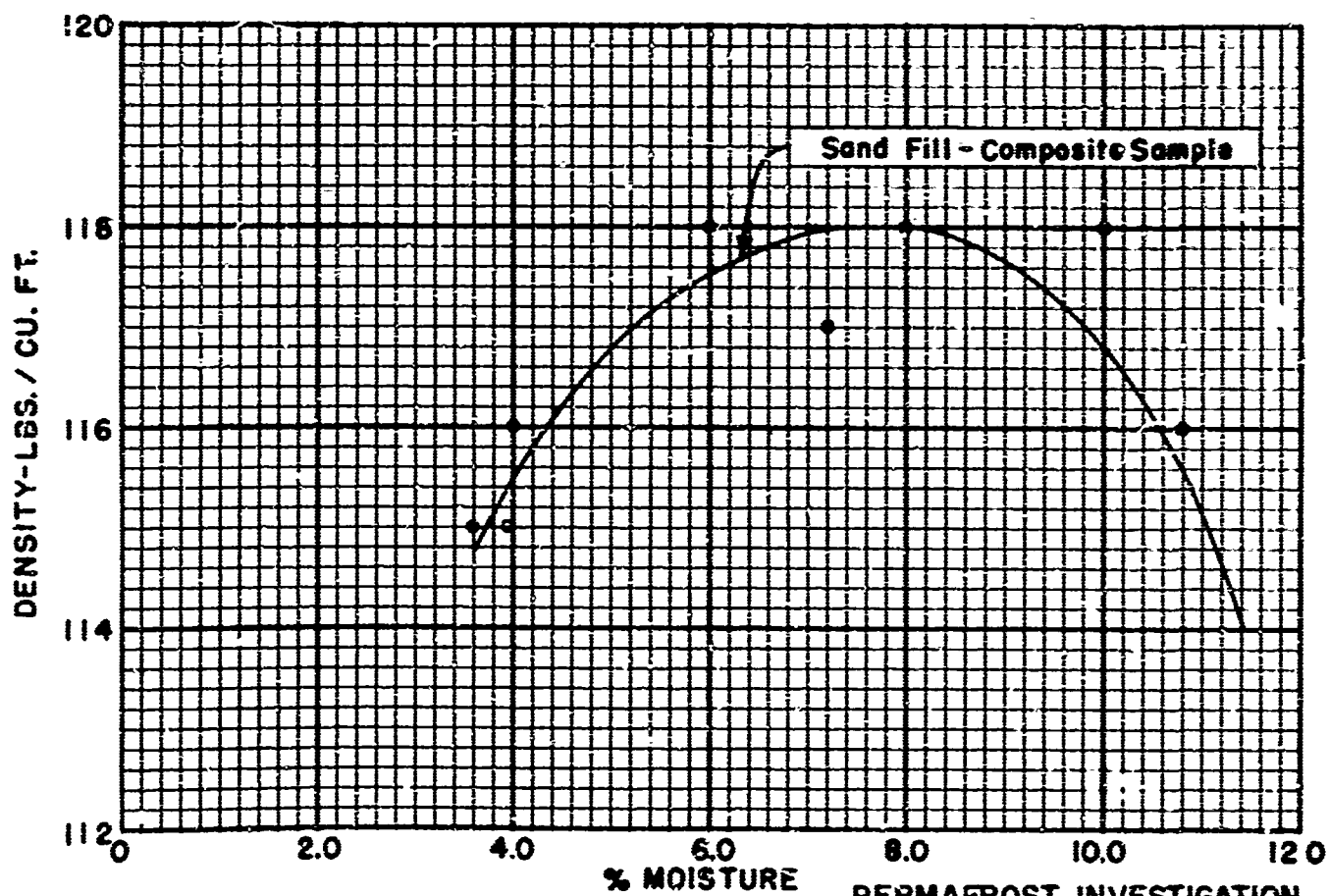
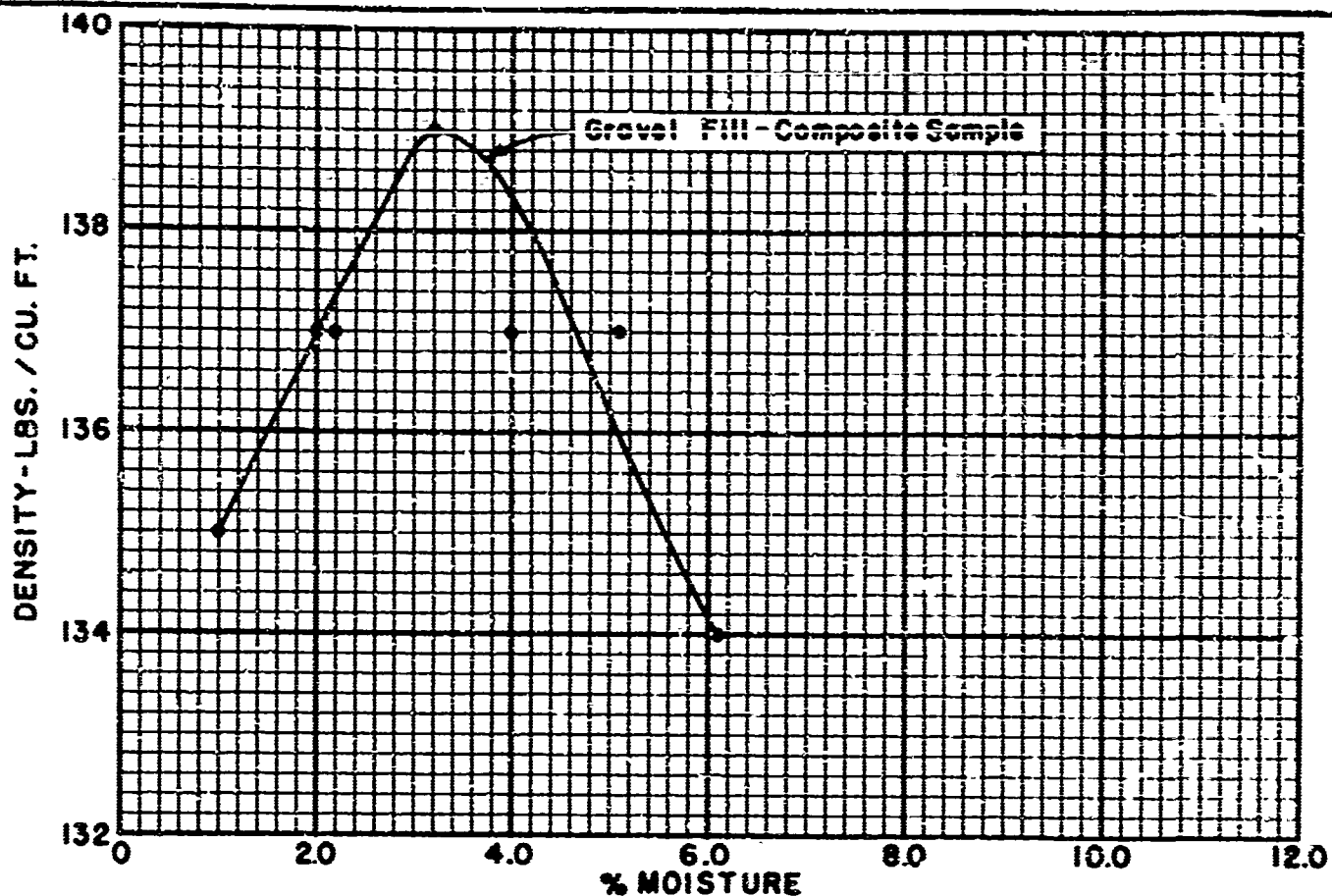
1948



PERMAFROST INVESTIGATION
FIELD RESEARCH-FAIRBANKS ALASKA
AREA NO. 1

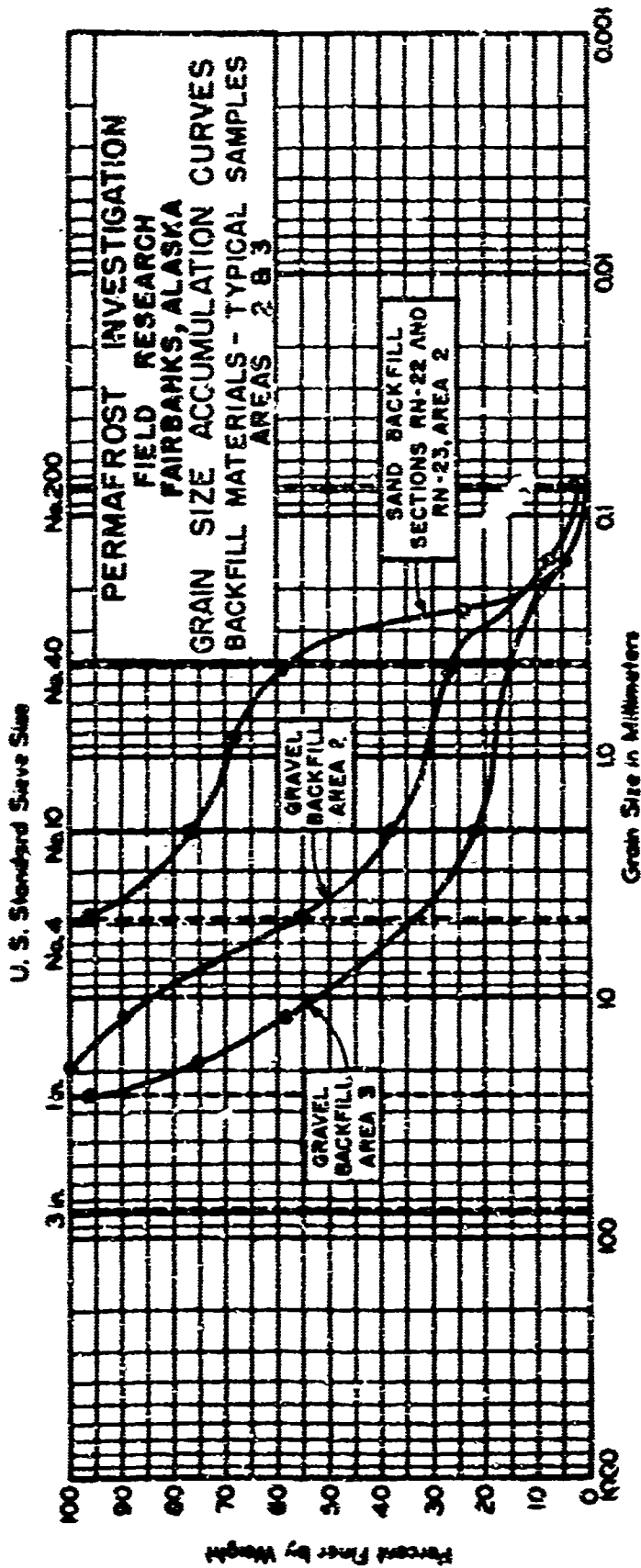
DEPTH OF THAW OBTAINED BY
PROBING - GROUND TEMPERATURES
1947 - 48

CORPS OF ENGINEERS, ST. PAUL, MINN. MAY. 1950



PERMAFROST INVESTIGATION
 FIELD RESEARCH-FAIRBANKS, ALASKA
 BACKFILL MATERIAL - AREA 2 & 3
 MOISTURE-DENSITY CURVES
 CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950

MECHANICAL ANALYSIS



COBBLES	GRAVEL			SAND		SILT or CLAY
	Coarse	Medium	Fine	Coarse	Fine	

Number	Natural Moisture	LL	PL	PI	Classification	Remarks
Area 2	-	-	-	-	Sand	Nonplastic
Area 2	-	-	-	-	Gravel	"
Area 3	-	-	-	-	"	"

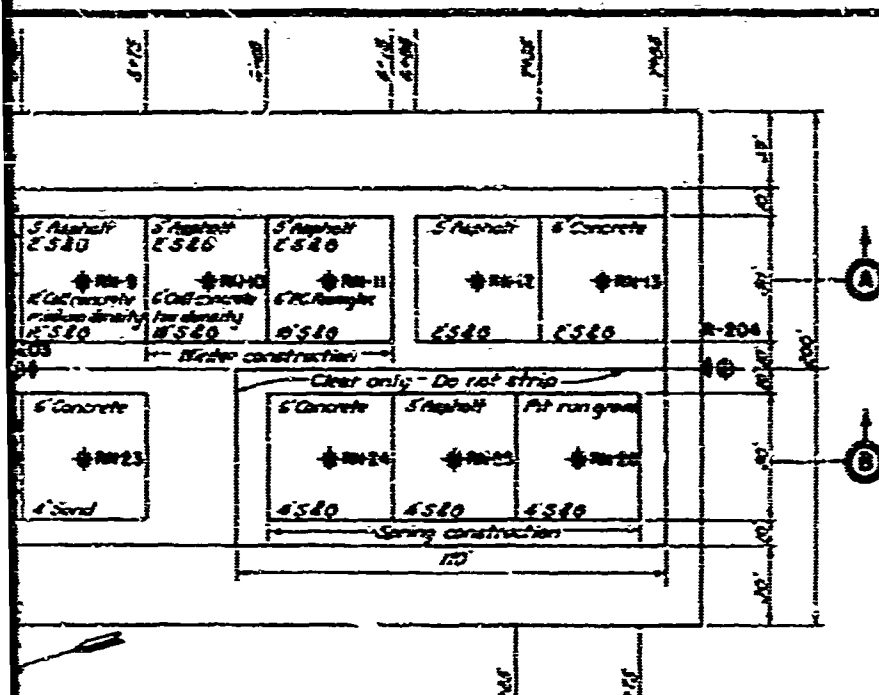
SOILS LABORATORY
PERMAFROST
LADD A.F.B., ALASKA

PROJECT Field Research

AREA 2 & 3 Backfill Material

LOCATION See Curve Mole No. -

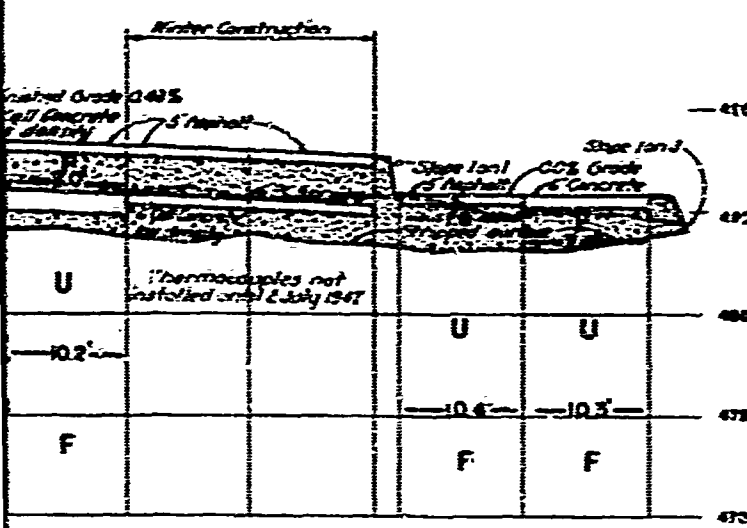
PLOTTED BY - CHECKED BY -



RM-9 RM-10 RM-11 RM-12 RM-13

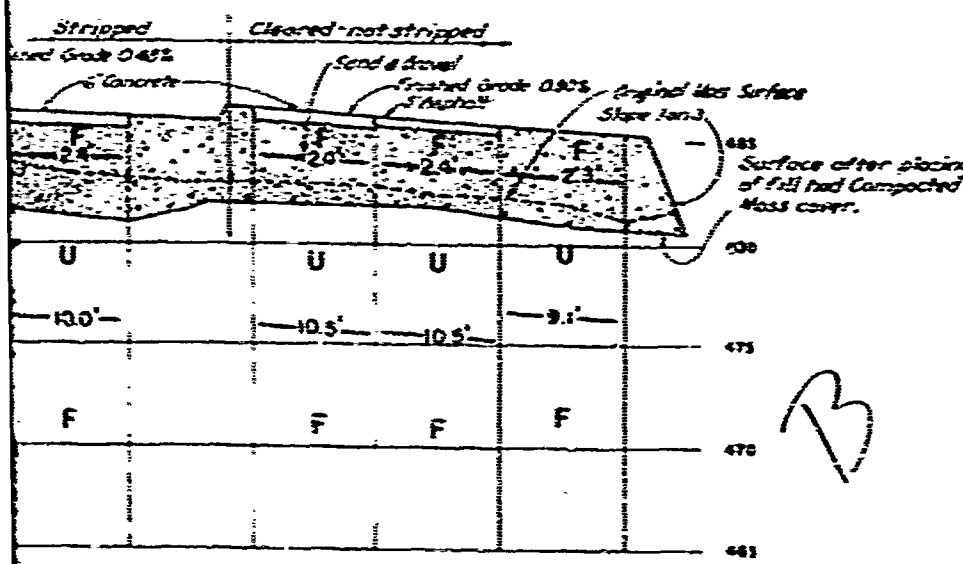
LEGEND

- 30' Core boring (Ground temperature hole)
- 15' Churn drill boring (Ground temperature hole)
- Frost observation point.
- 525 Pit run sand and gravel.
- Limit of frozen and unfrozen zones with depth below surface of section.
- F Frozen Zone
- U Unfrozen Zone



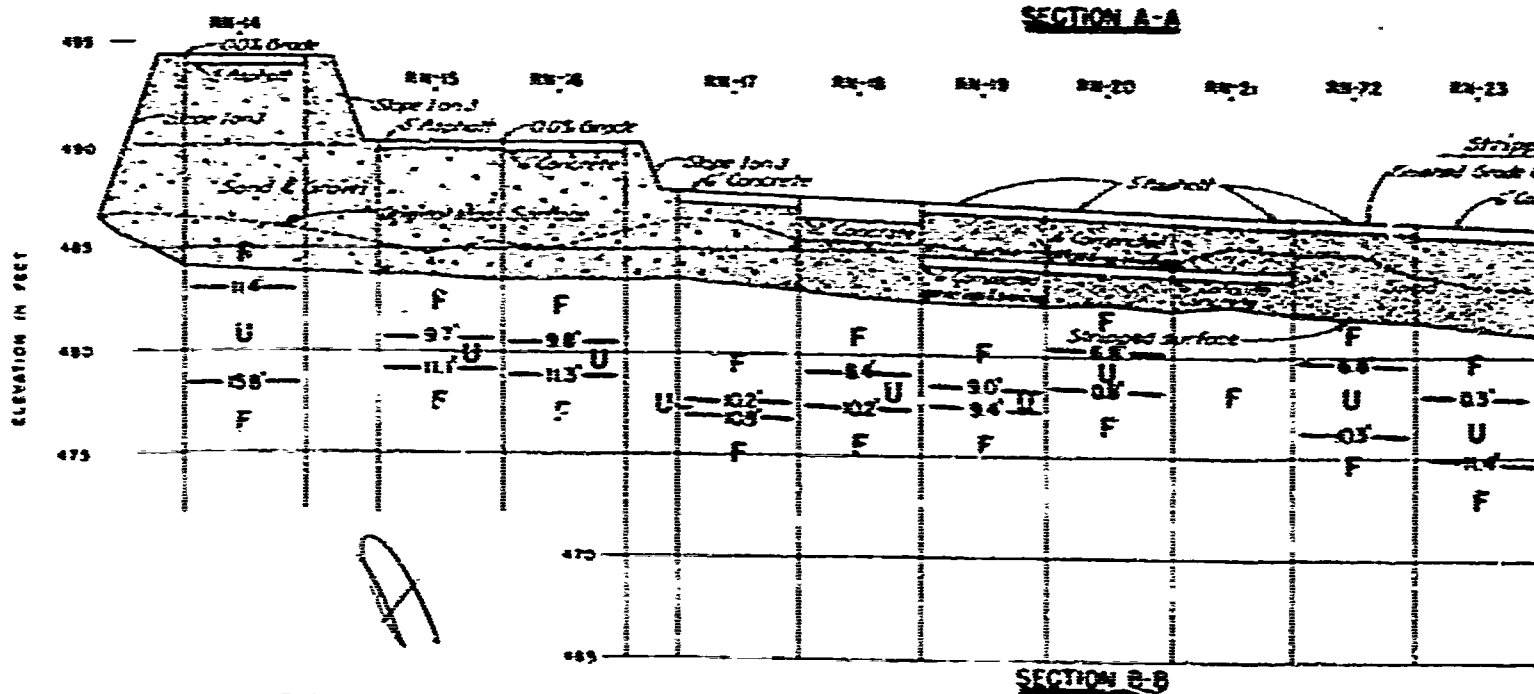
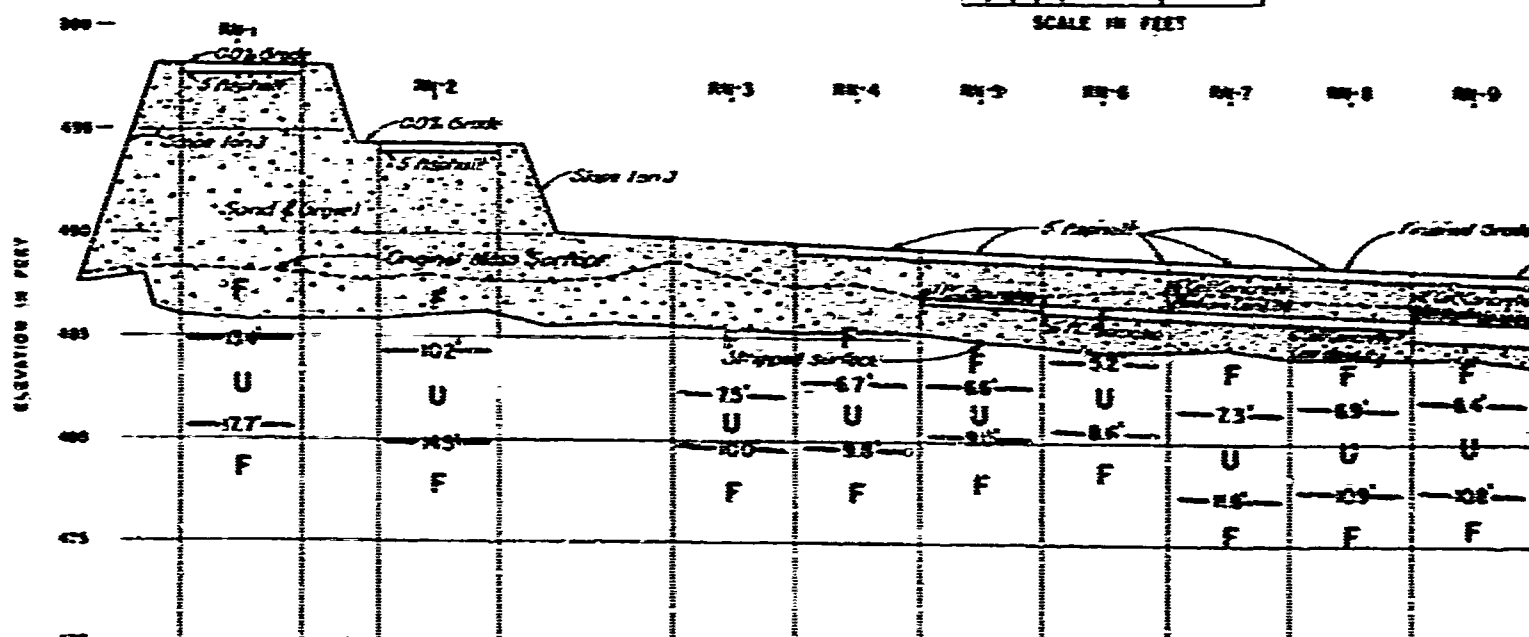
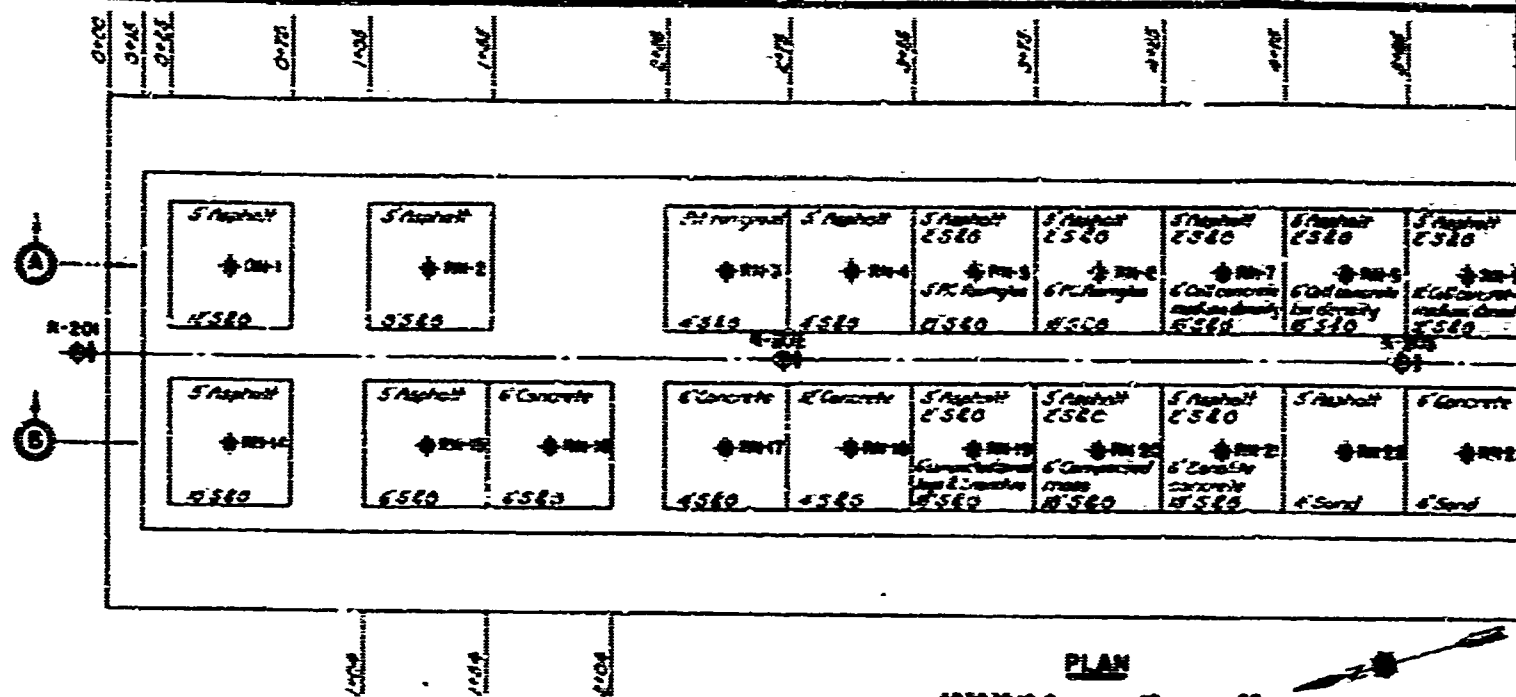
Note:
At ground temperature hole R-221, depth of seasonal frost = 64". Depth to permafrost = 4.6".
At ground temperature hole R-224, no seasonal frost recorded. Depth to permafrost = 5.3".

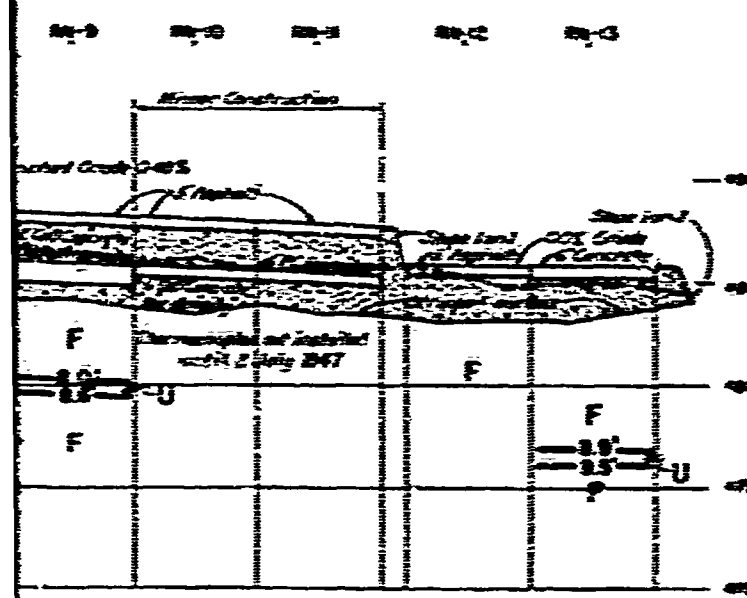
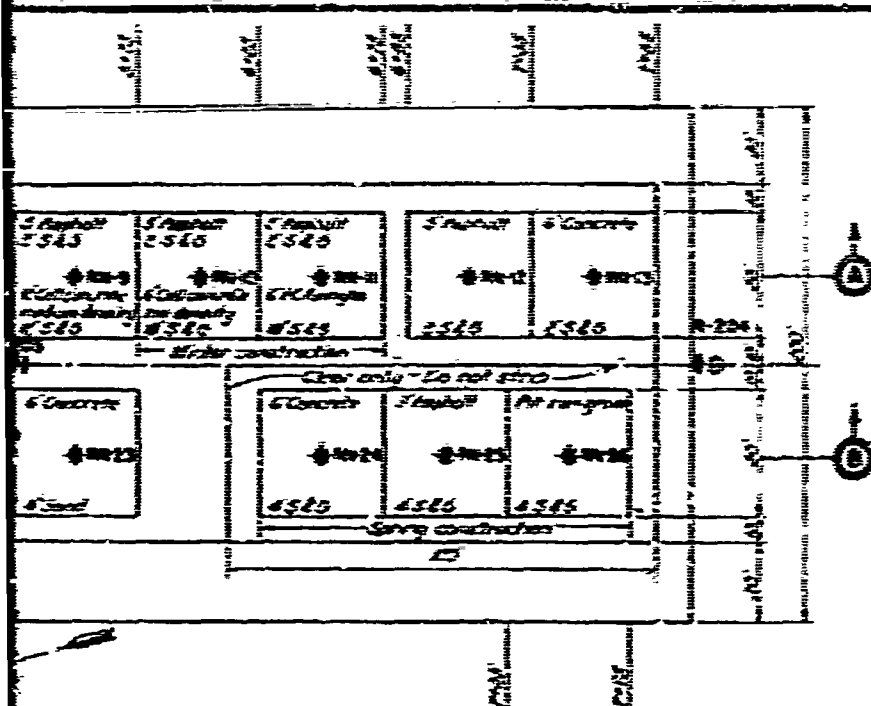
RM-23 RM-24 RM-25 RM-26



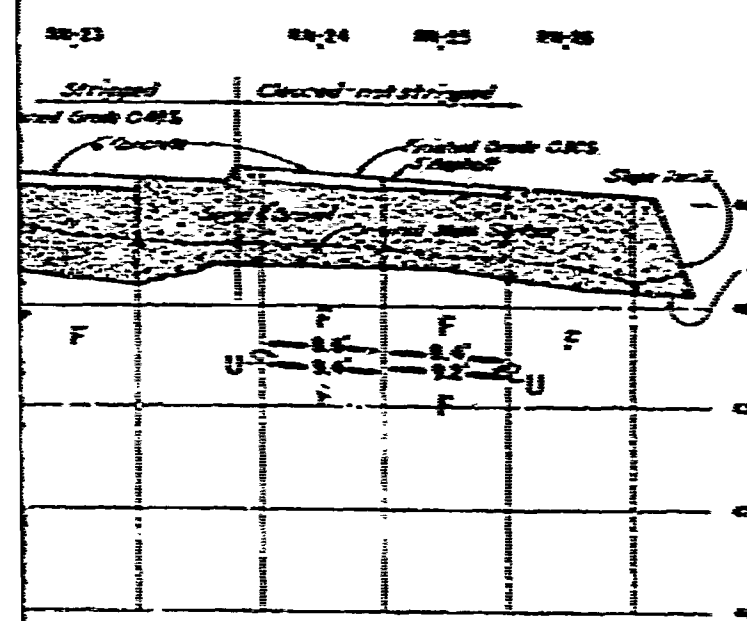
PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 2
RUNWAY FOUNDATION STUDIES
FROZEN AND UNFROZEN ZONES
1 NOV. 1946
SCALE AS SHOWN

CORPS OF ENGINEERS, ST. PAUL, MINN.
DRAWN BY E.C.T. TRACED BY D.F.J. CHECKED BY





- LEGEND**
- ✦ 30 Core boring (Ground temperature hole)
 - ✪ 15 Core drill boring (Ground temperature hole)
 - ✦ Frost observation point.
 - SL5 Pit run sand and gravel.
 - Limit of frozen and unfrozen zones with depth below surface of section.
 - F Frozen Zone
 - U Unfrozen Zone

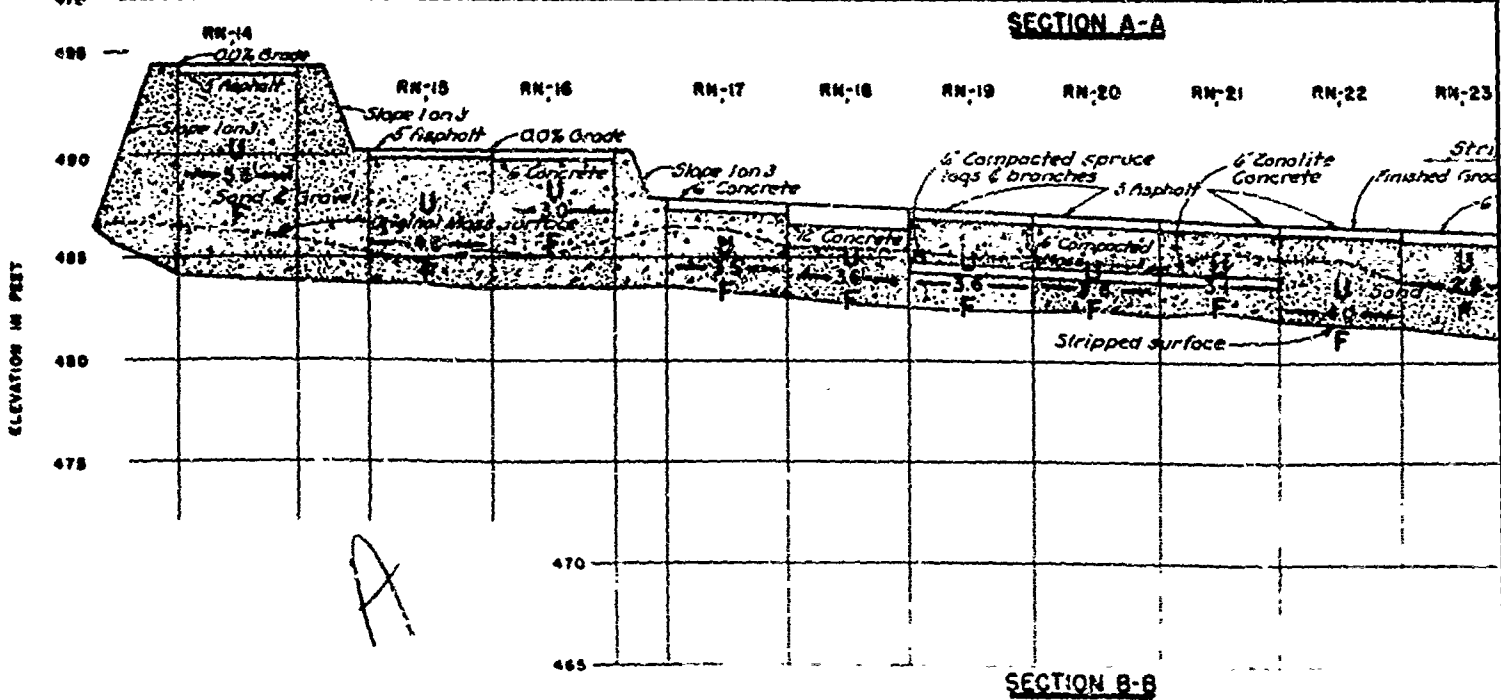
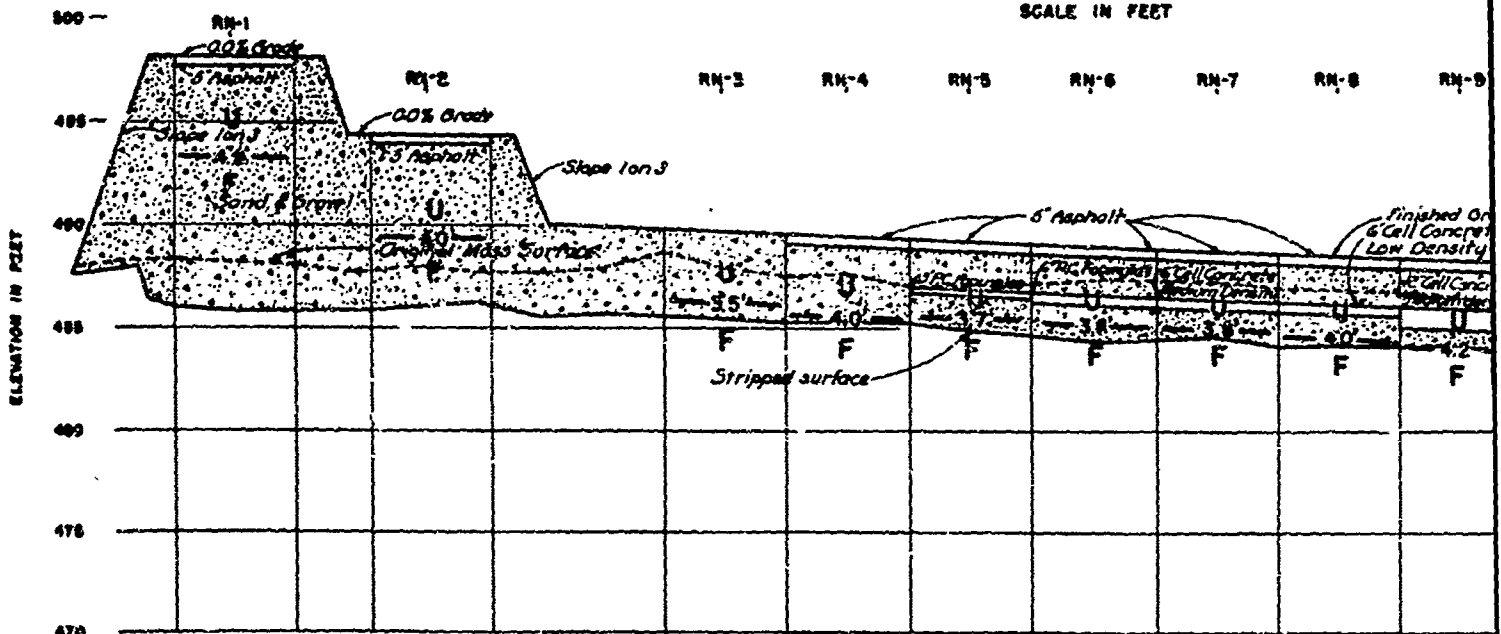
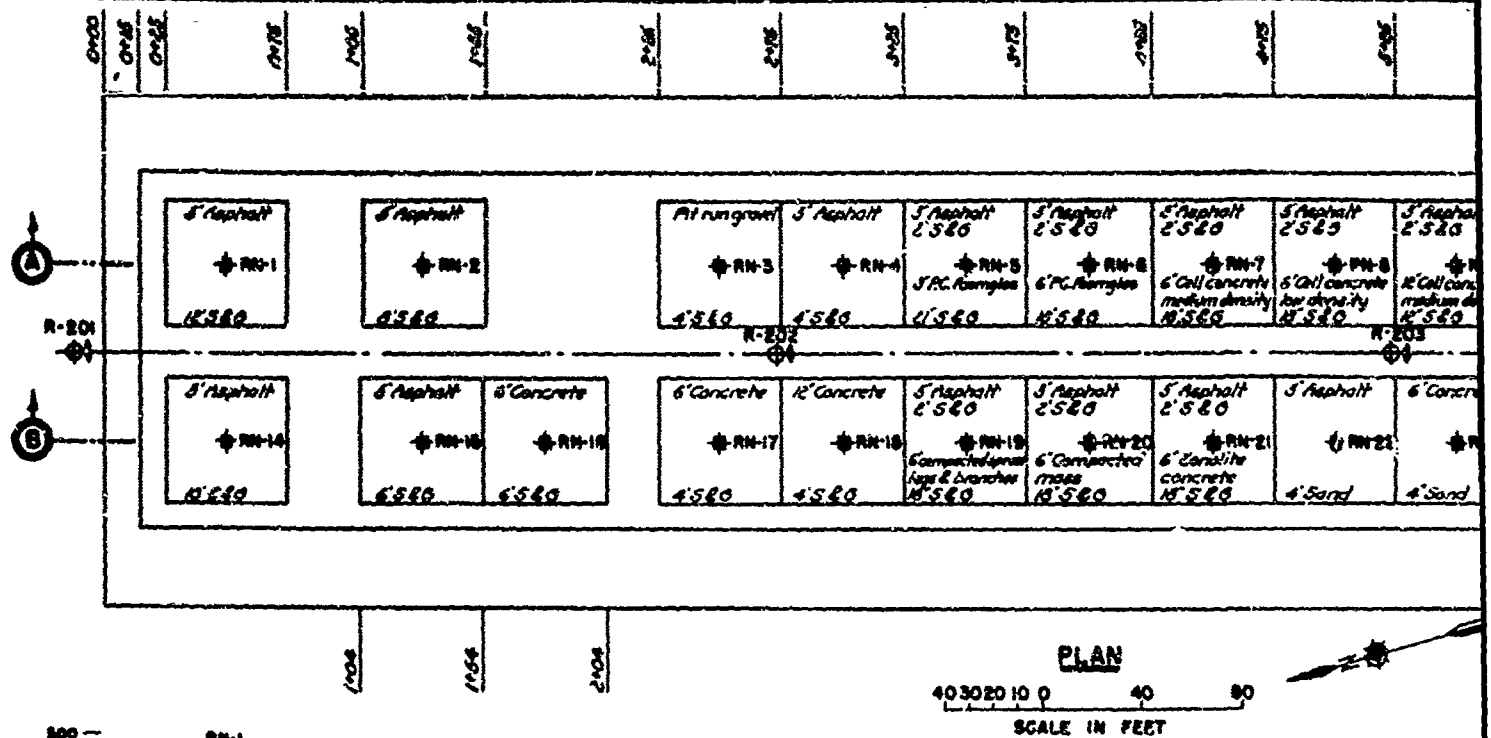


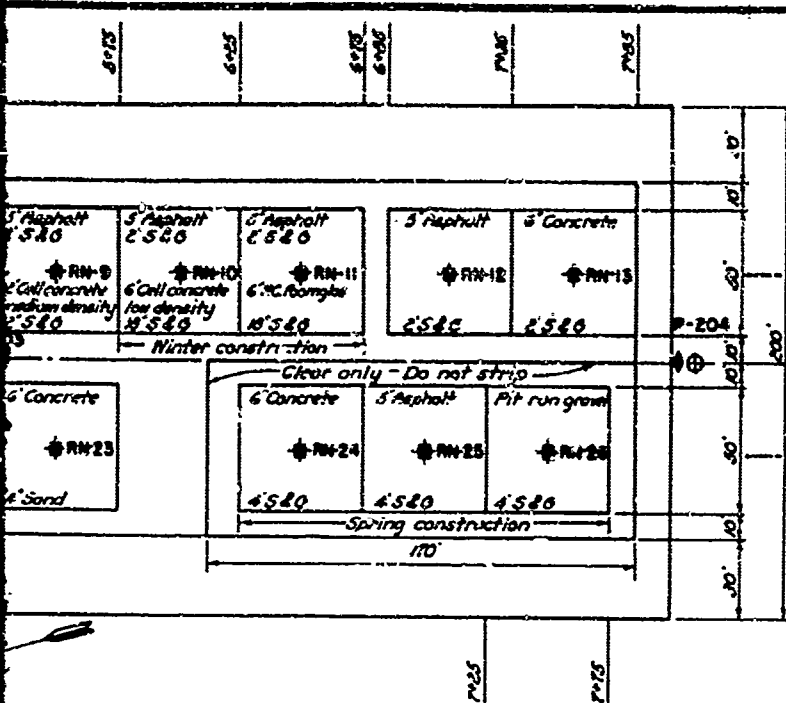
Note:
At ground temperature holes 2-201 and 2-204
seasonal frost and permafrost have merged.

PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 2
RUNWAY FOUNDATION STUDIES
FROZEN AND UNFROZEN ZONES
1 APRIL 1947
SCALE AS SHOWN

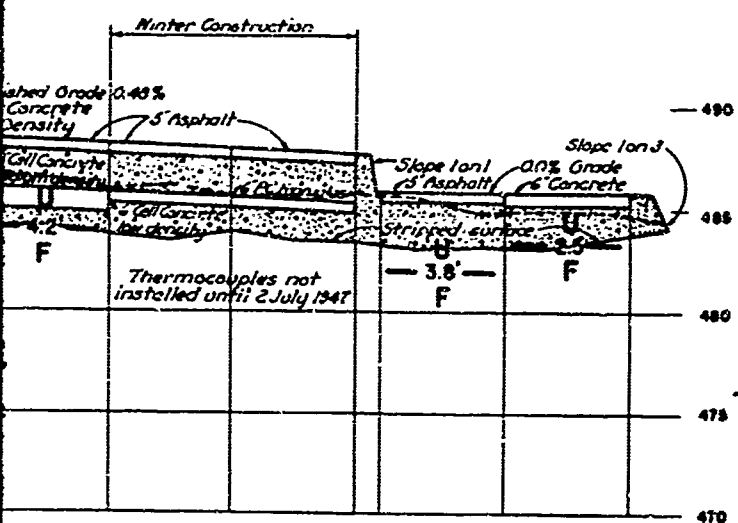
B

COPIES OF ENGINEERS, ST. PAUL, MINN.
DRAWN BY E.C.C. CHECKED BY E.C.C. DESIGNED BY E.C.C.





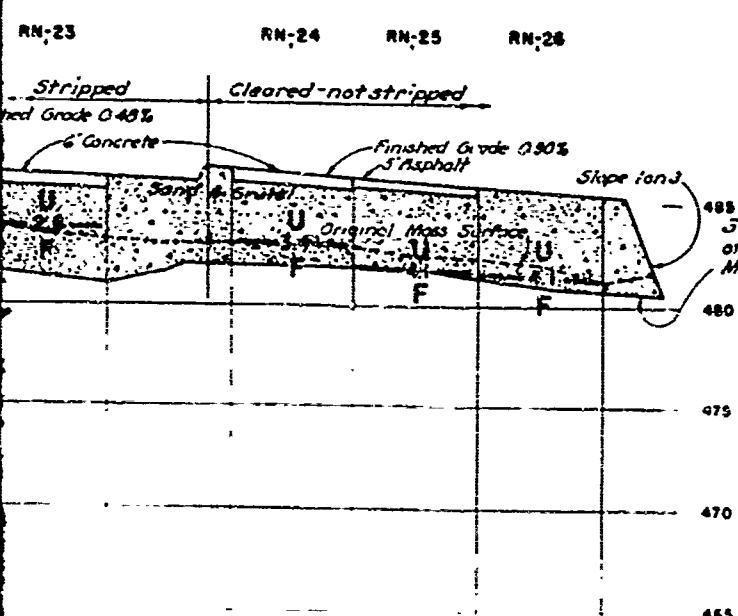
RN-9 RN-10 RN-11 RN-12 RN-13



LEGEND

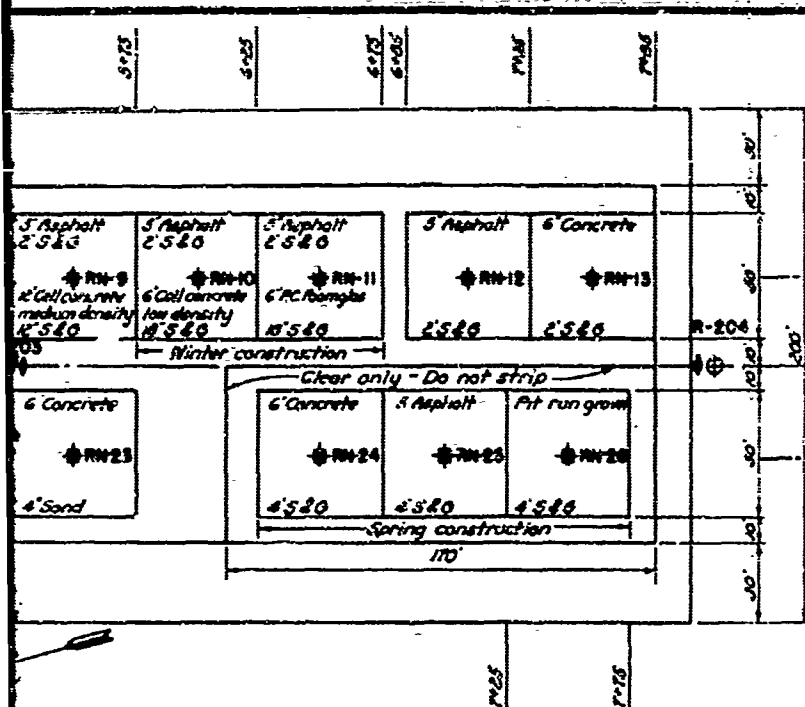
- 30' Core boring (Ground temperature hole)
- 15' Churn drill boring (Ground temperature hole)
- Frost observation point.
- 5&6 Pit run sand and gravel.
- 3.8' Limit of frozen and unfrozen zones with depth below surface of section.
- F Frozen Zone
- U Unfrozen Zone

Note:
At ground temperature hole R 27, no observations possible due to flooding of junction box.
At ground temperature hole R 204 temperature at surface was 0°C, no penetration of thaw recorded.

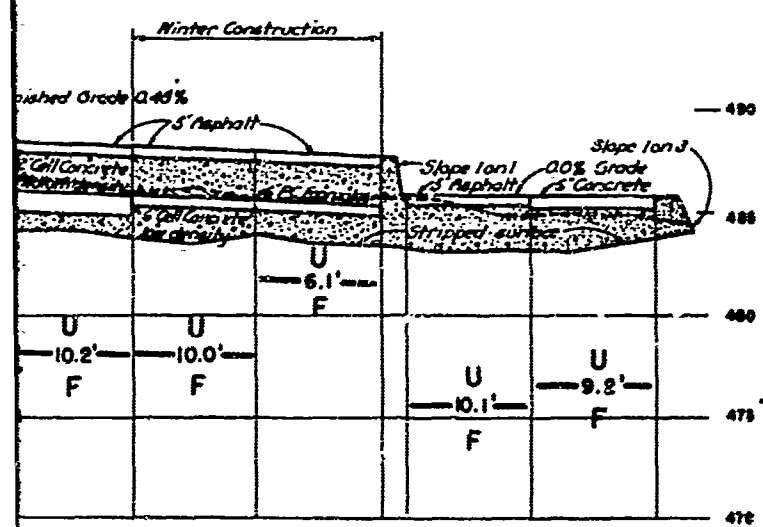


PERMAFROST INVESTIGATION FIELD RESEARCH FAIRBANKS, ALASKA AREA NO. 2 RUNWAY FOUNDATION STUDIES FROZEN AND UNFROZEN ZONES 1 MAY 1947 SCALE AS SHOWN

CORPS OF ENGINEERS, ST PAUL, MINN
DRAWN BY E C T
MAY 1950
CHECKED BY



RN-9 RN-10 RN-11 RN-12 RN-13



LEGEND

- 30' Core boring (Ground temperature hole)
- 15' Churn drill boring (Ground temperature hole)
- Frost observation point.
- 526 Pit run sand and gravel.
- Limit of frozen and unfrozen zones with depth below surface of section.
- F Frozen Zone
- U Unfrozen Zone

Note:

At ground temperature hole R-201, depth to permafrost = 4.6'.
At ground temperature hole R-204, depth to permafrost = 6.0'.

PERMAFROST INVESTIGATION

FIELD RESEARCH
FAIRBANKS, ALASKA

AREA NO. 2

RUNWAY FOUNDATION STUDIES
FROZEN AND UNFROZEN ZONES

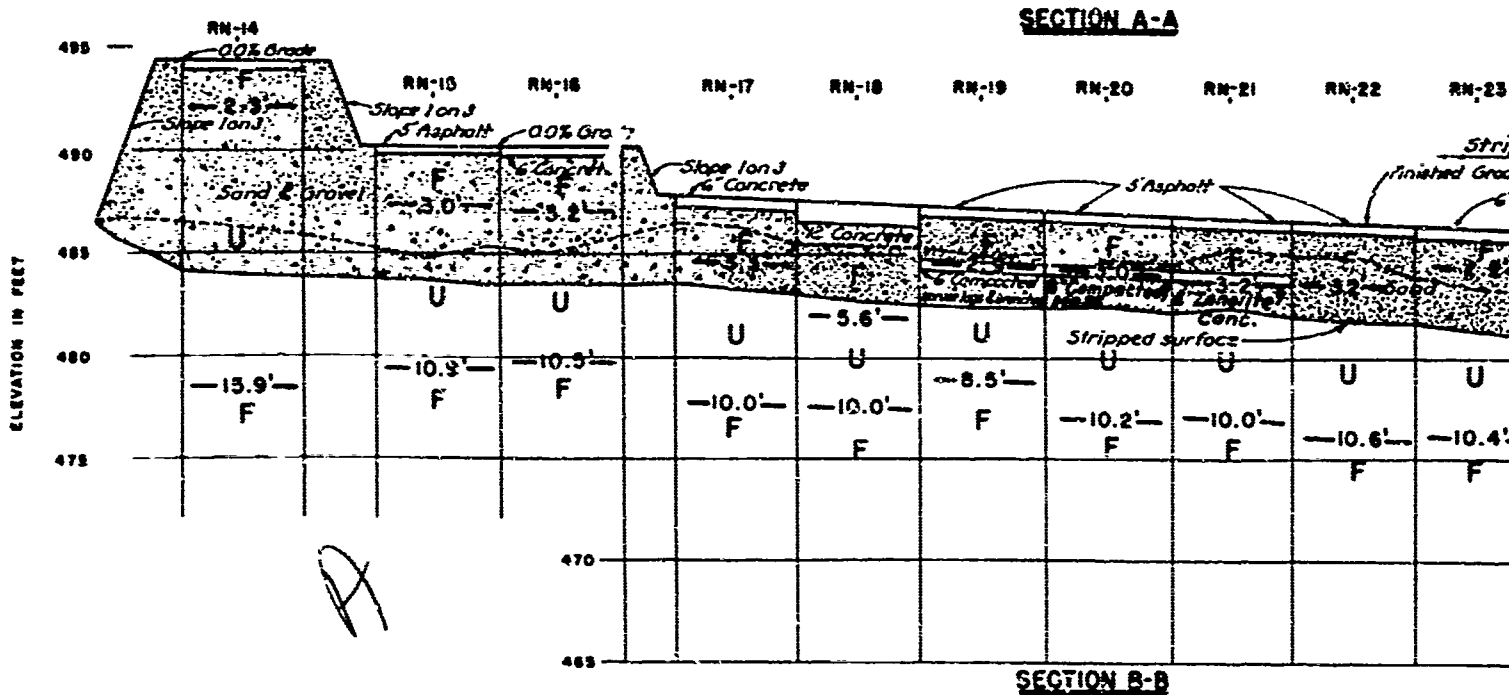
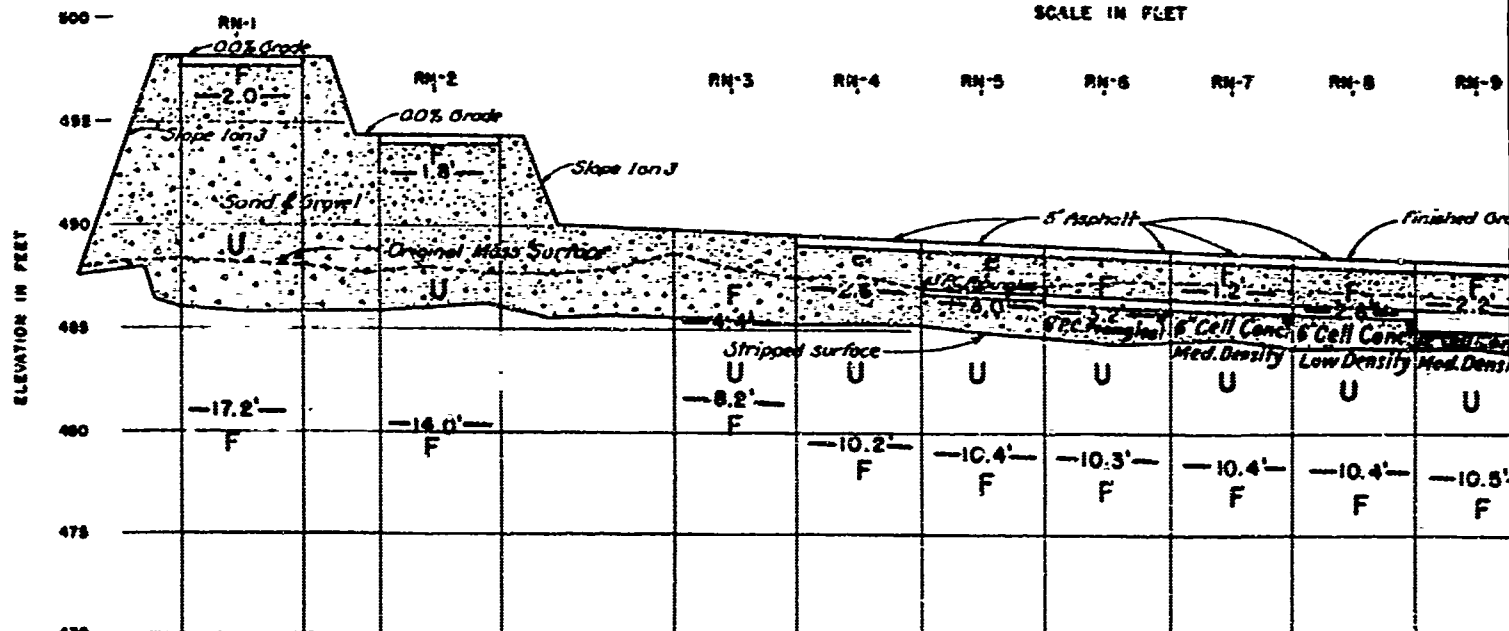
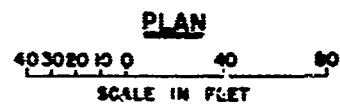
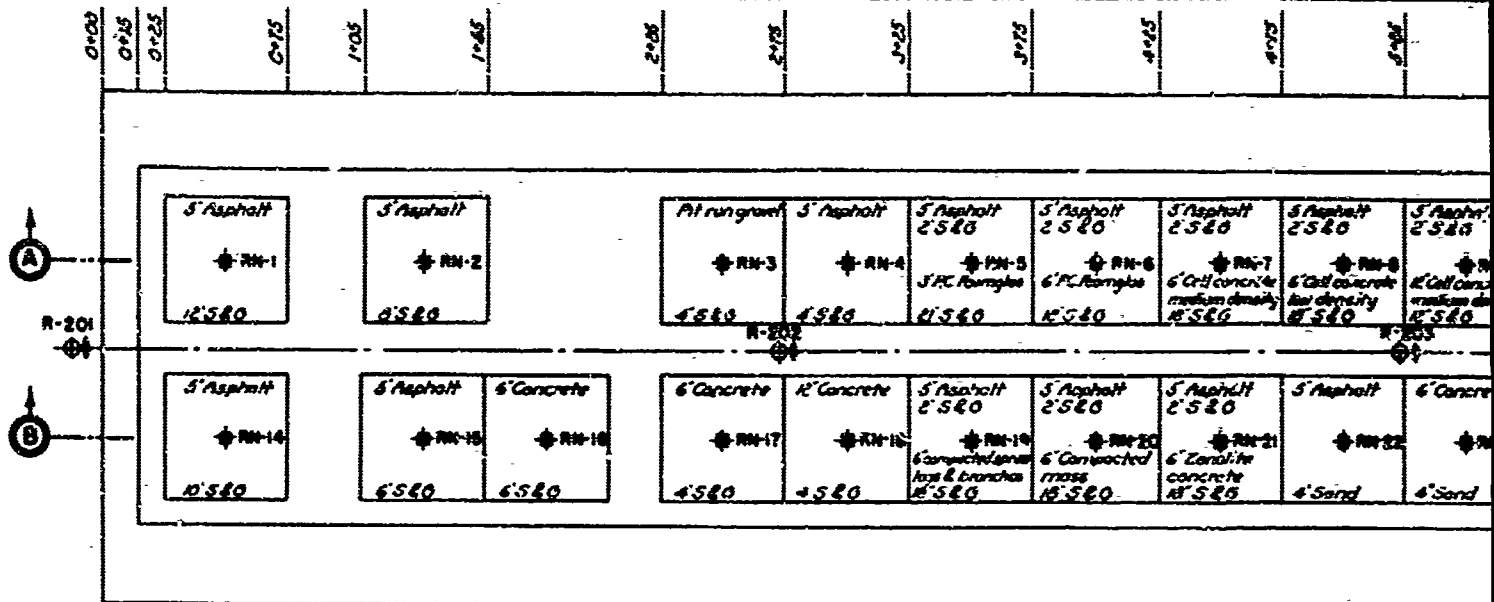
1 OCT. 1947

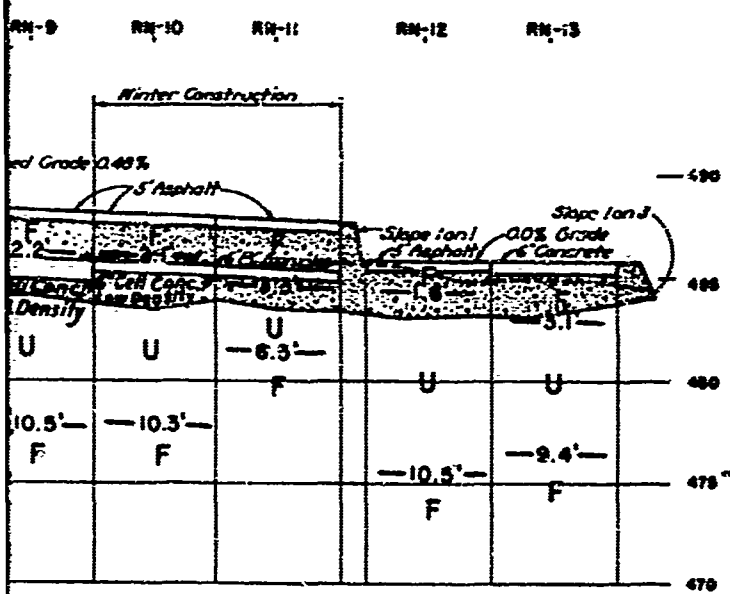
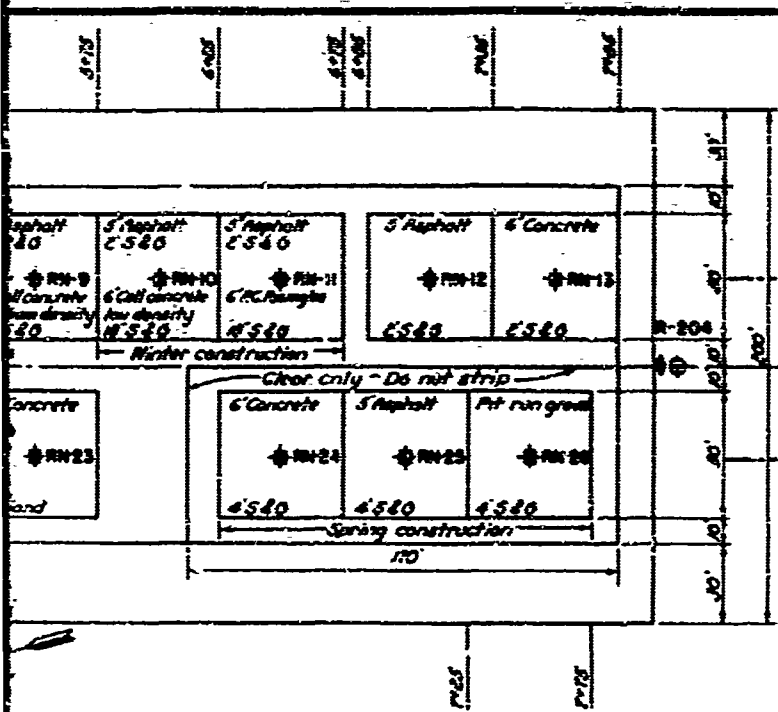
SCALE AS SHOWN

CORPS OF ENGINEERS, ST. PAUL, MINN.
DRAWN BY: E.C.T. TRACED BY: M.J.B.

MAY 1990
CHECKED BY:

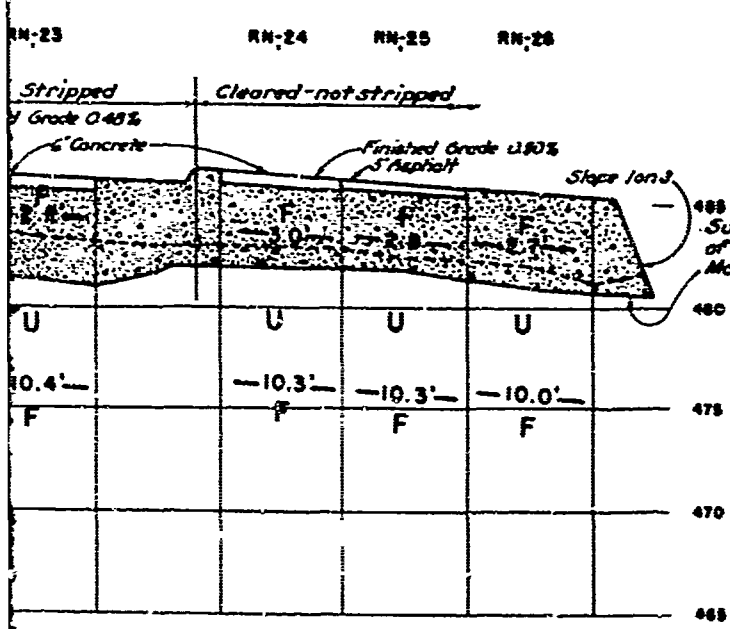
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LEGEND

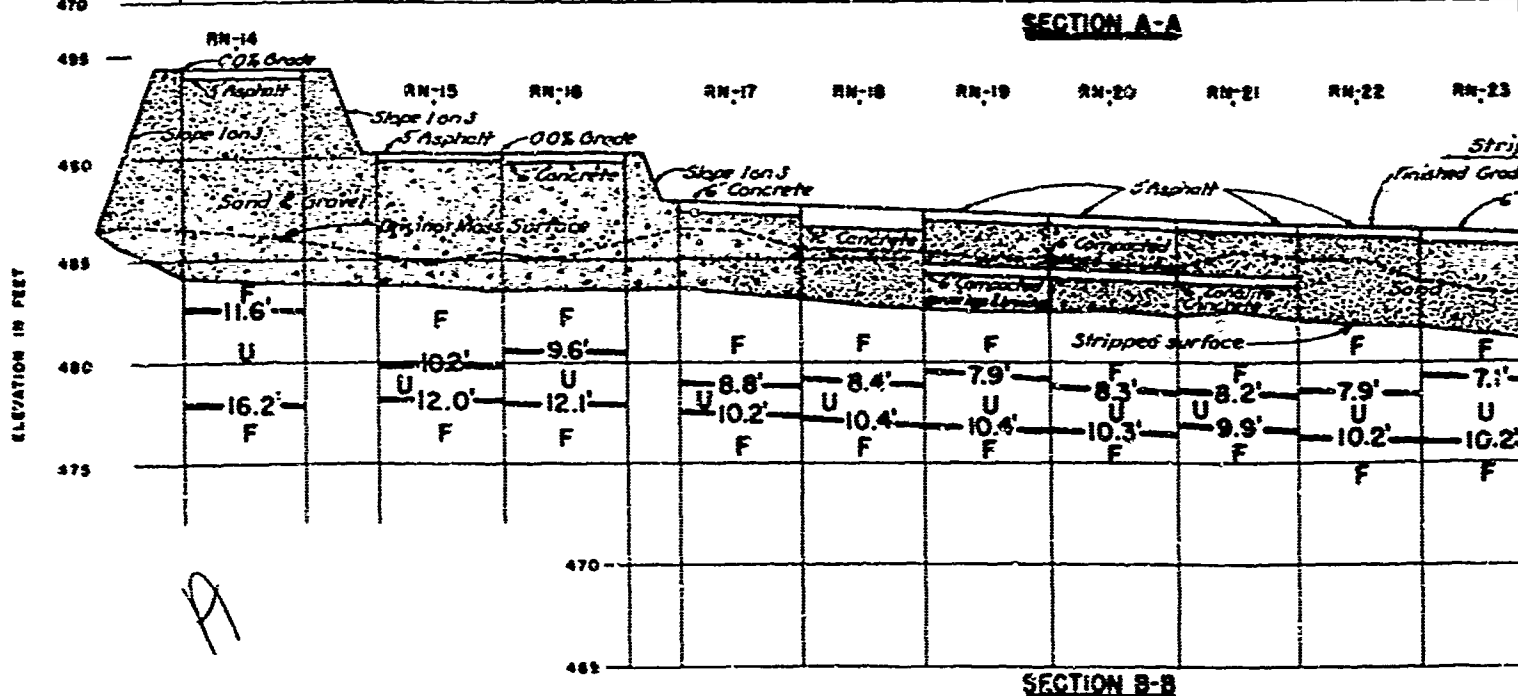
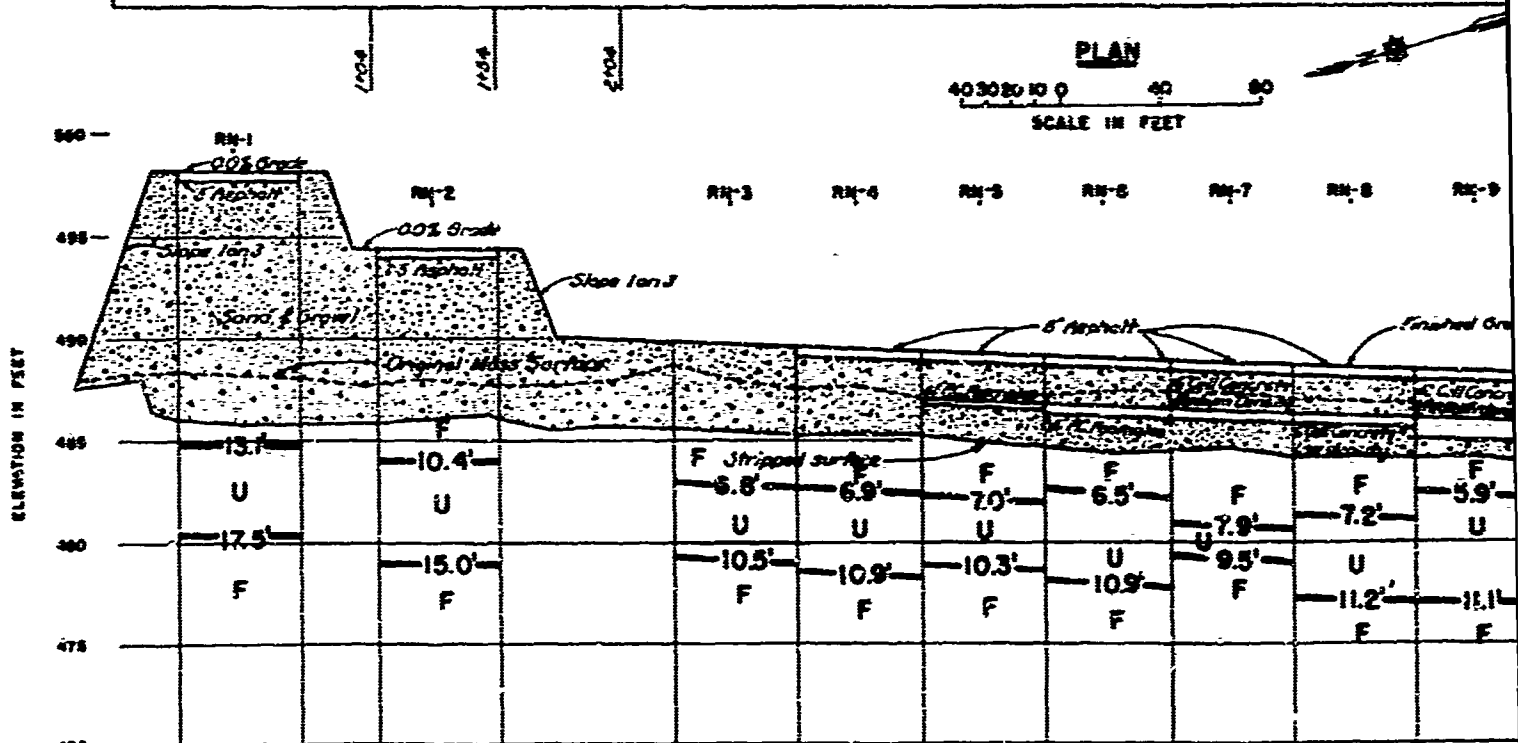
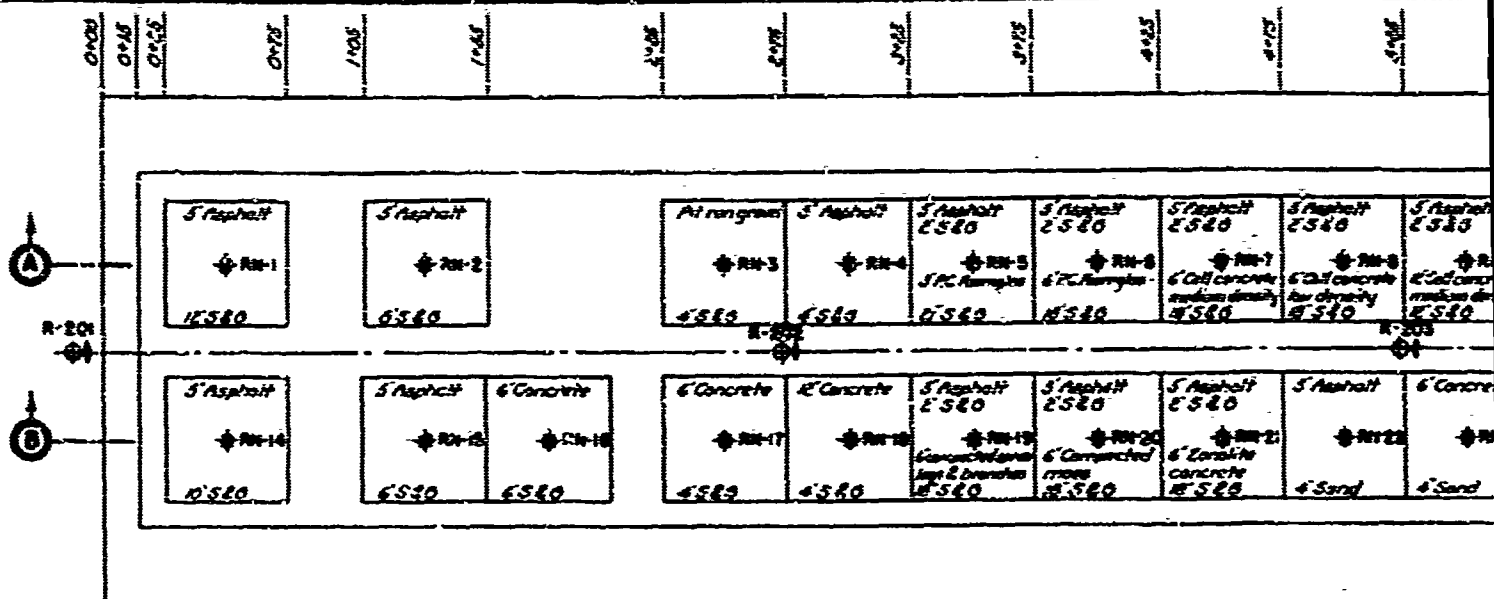
- 30' Core boring (Ground temperature hole)
- 15' Churn drill boring (Ground temperature hole)
- Frost observation point.
- 586 Pit run sand and gravel.
- Limit of frozen and unfrozen zones with depth below surface of section.
- F Frozen Zone
- U Unfrozen Zone

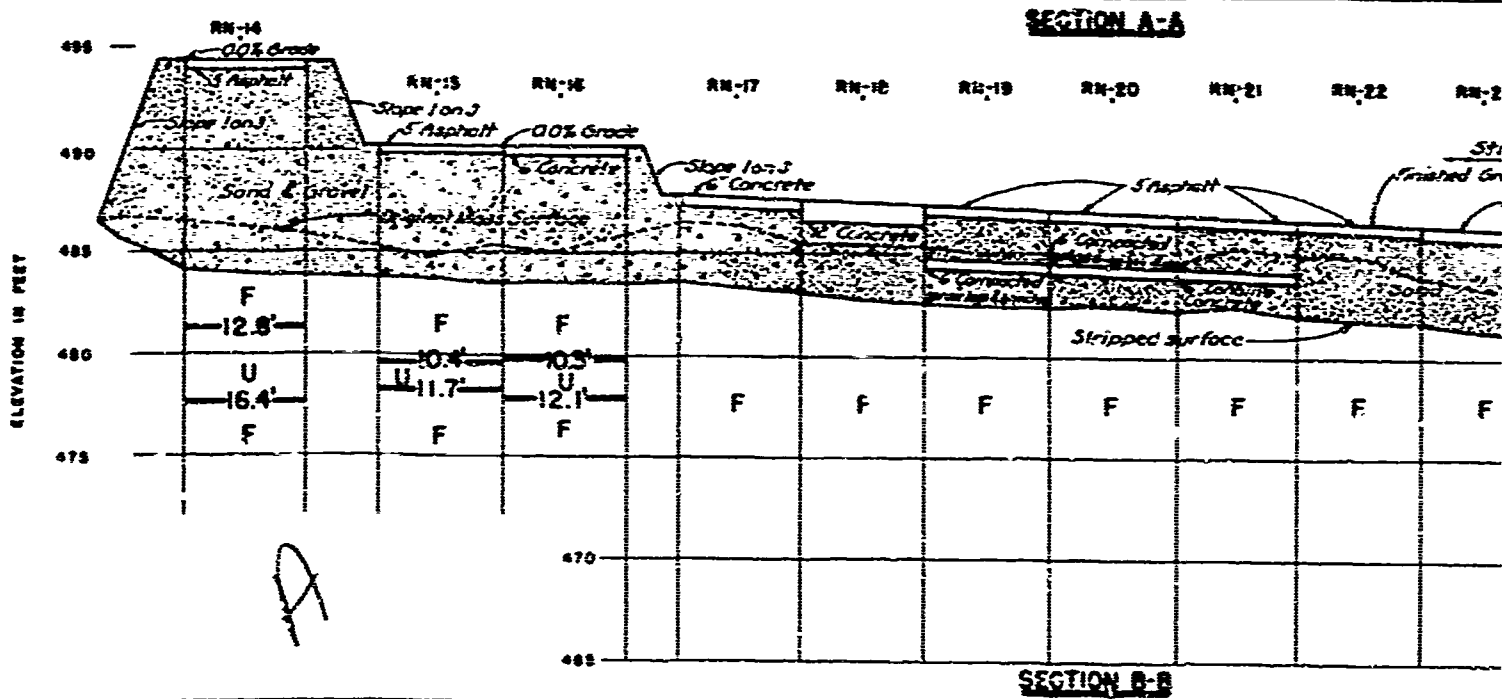
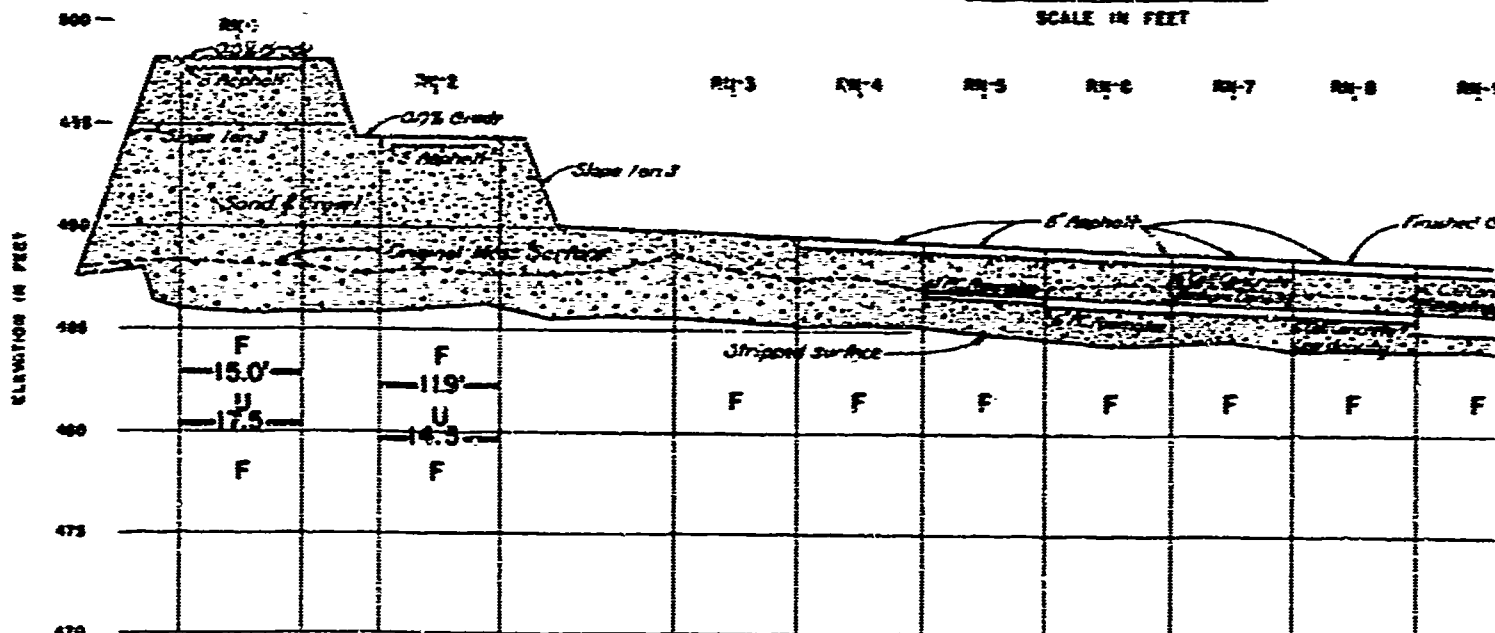
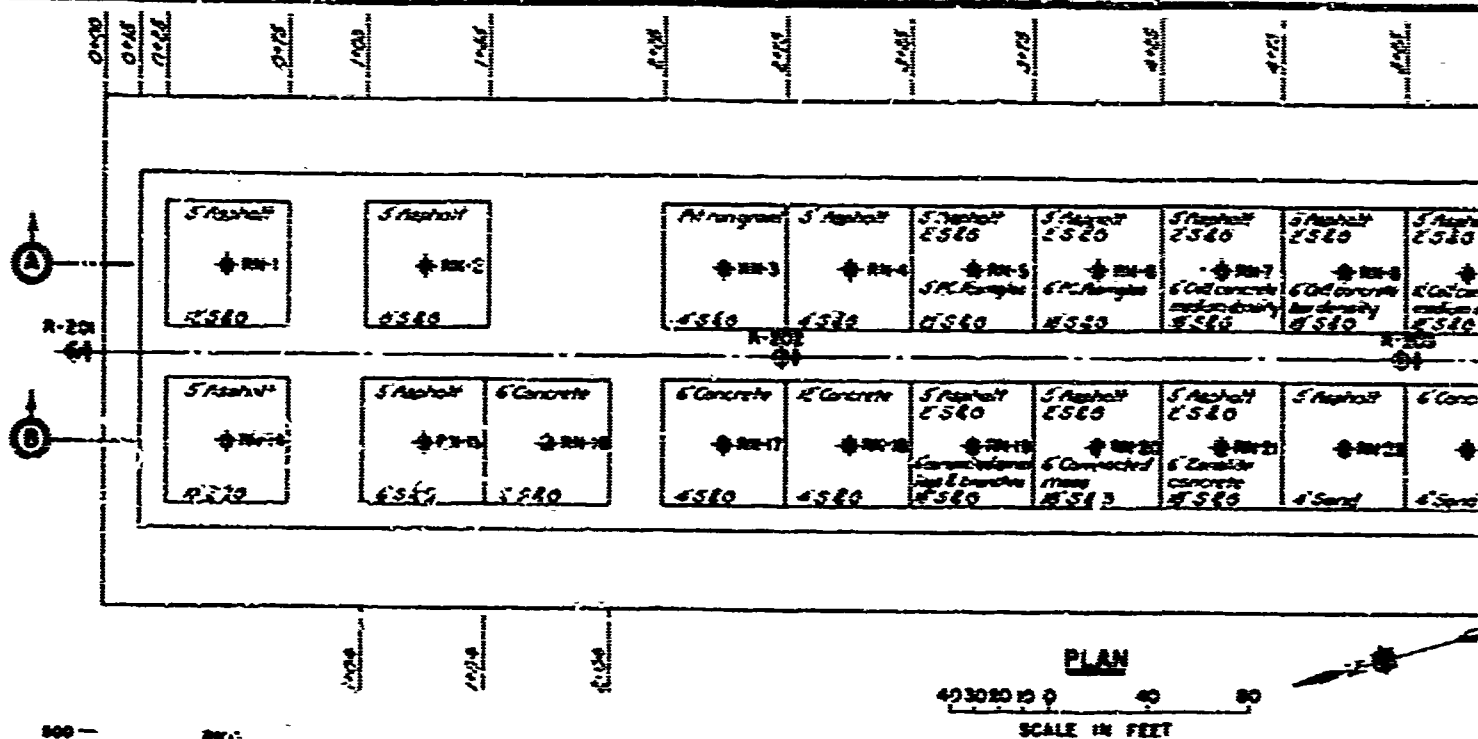


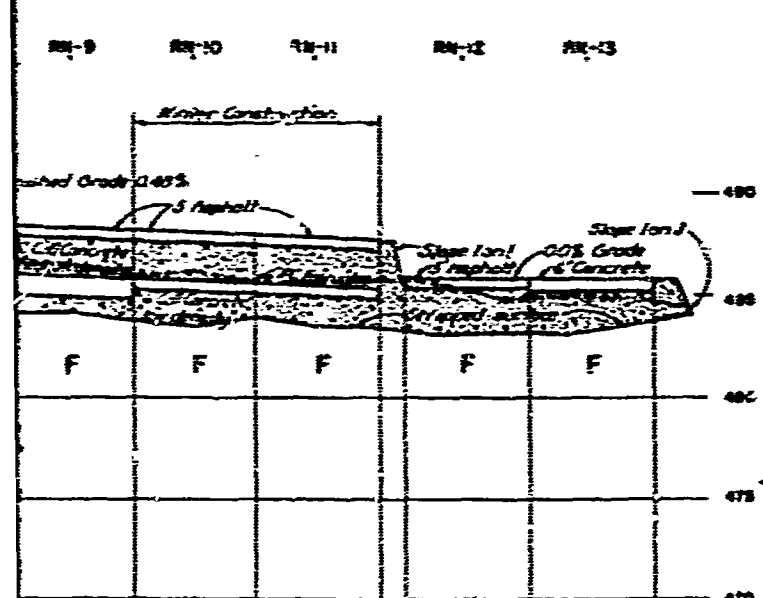
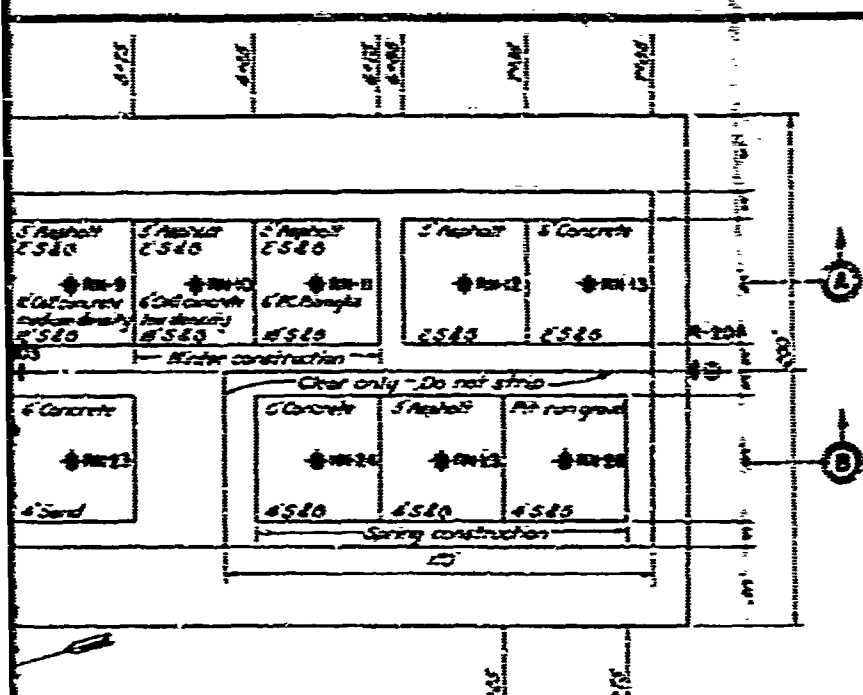
Note:
 1. At ground temperature hole R-201 seasonal frost and permafrost have merged.
 2. At ground temperature hole R-204 depth of seasonal frost = 11'. Depth to permafrost = 5.0'.

PERMAFROST INVESTIGATION FIELD RESEARCH FAIRBANKS, ALASKA AREA NO. 2 RUNWAY FOUNDATION STUDIES FROZEN AND UNFROZEN ZONES 31 OCT 1947 SCALE AS SHOWN

CORPS OF ENGINEERS, ST. PAUL, MINN.
 DRAWN BY E.C.T. TRACED BY W.O.S. MAY 1990
 CHECKED BY:

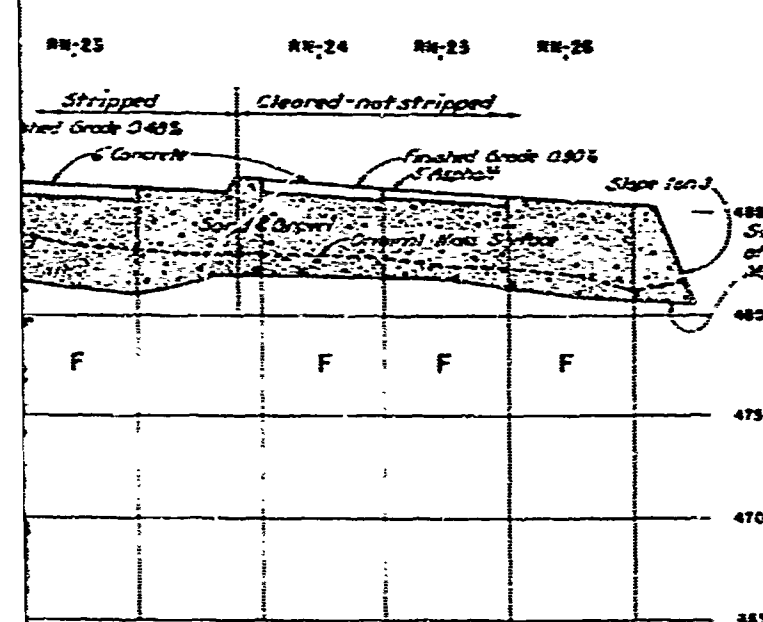






LEGEND

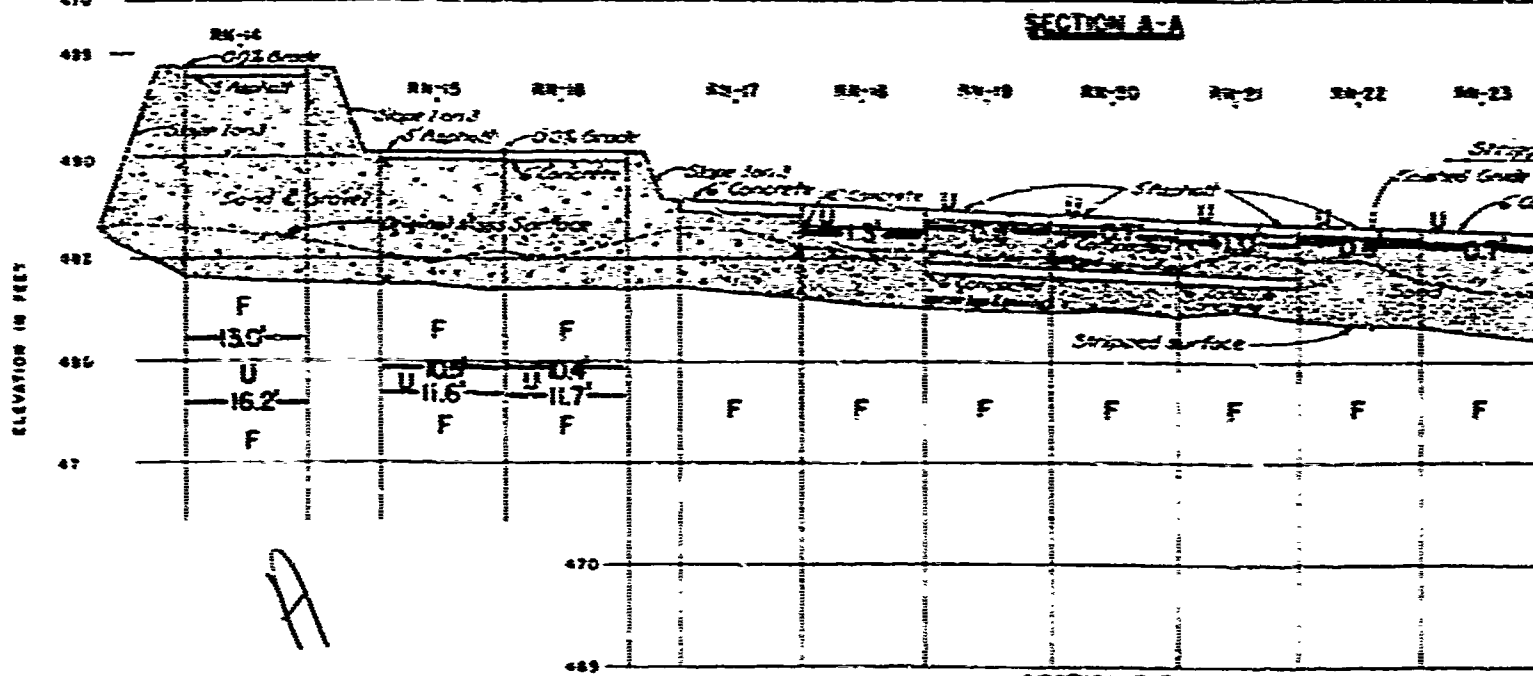
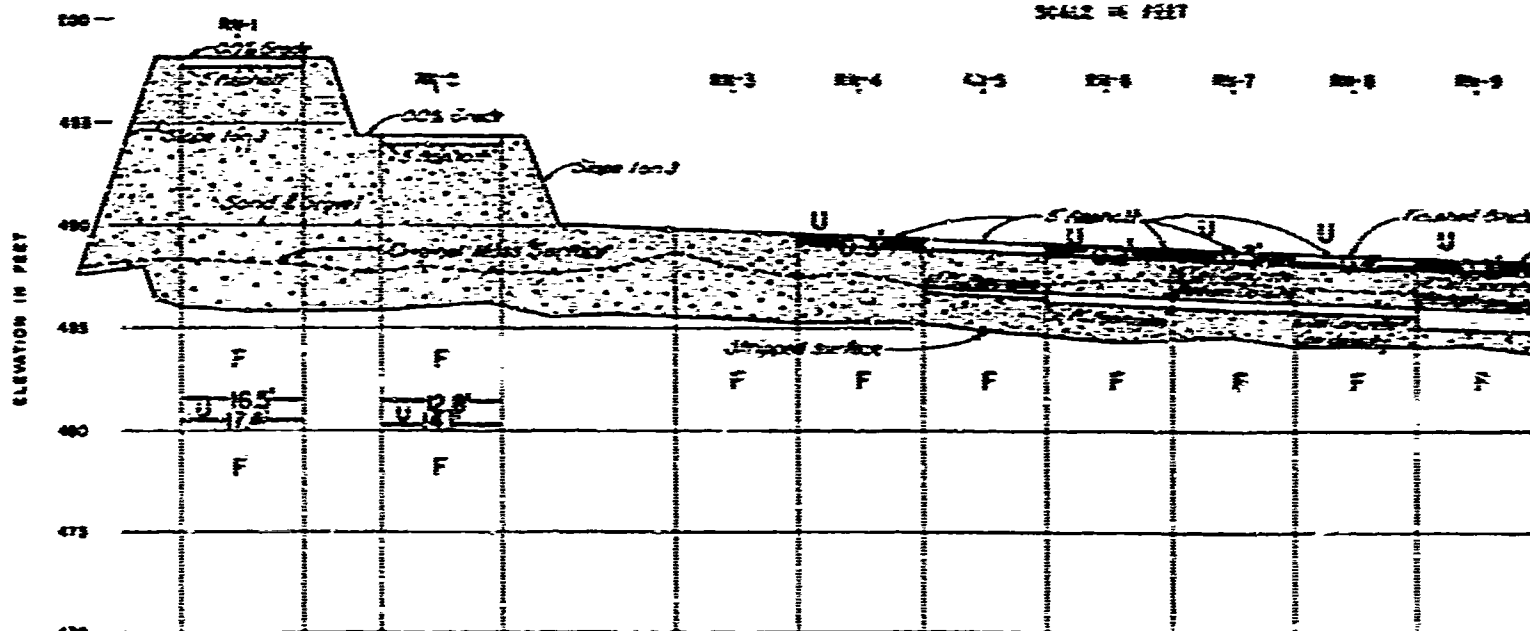
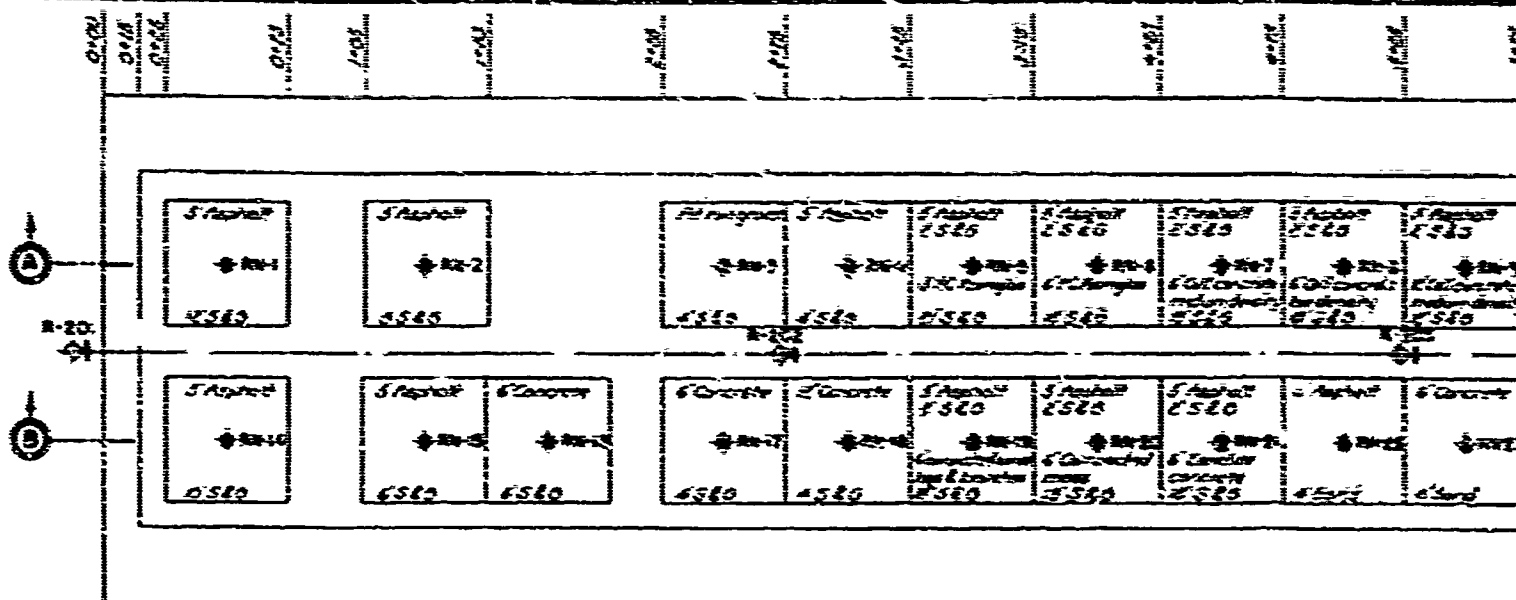
- 30' Core boring (Ground temperature hole)
- 15' Churn drill boring (Ground temperature hole)
- Frost observation point.
- 586 Pit run sand and gravel.
- Limit of frozen and unfrozen zones with depth below surface of section.
- F Frozen zone.
- U Unfrozen zone.

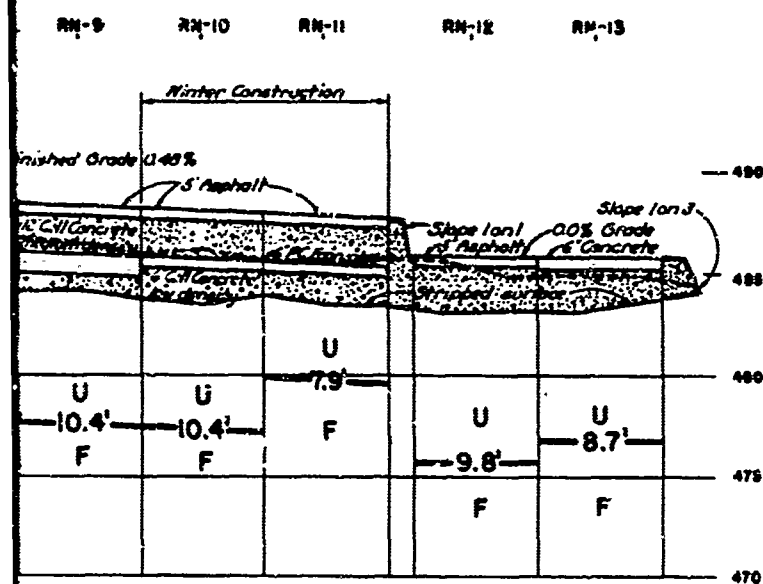
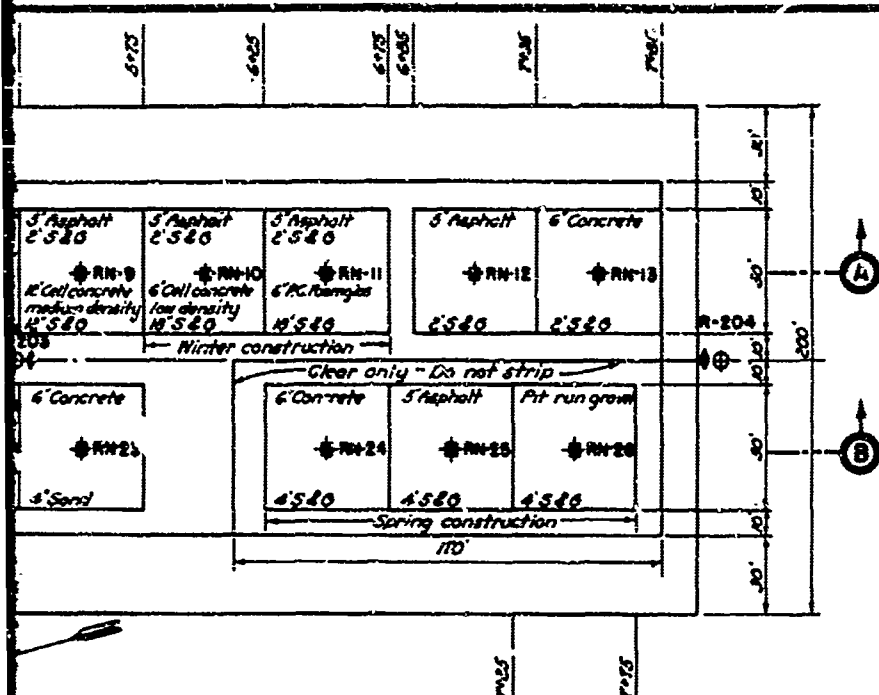


Note:
 1. ground temperature hole R-201, depth to permafrost-4.6'.
 2. ground temperature hole R-204, depth to permafrost-6.0'.

PERMAFROST INVESTIGATION FIELD RESEARCH FAIRBANKS, ALASKA AREA NO. 2 RUNWAY FOUNDATION STUDIES FROZEN AND UNFROZEN ZONES 1 APRIL 1948 SCALE AS SHOWN

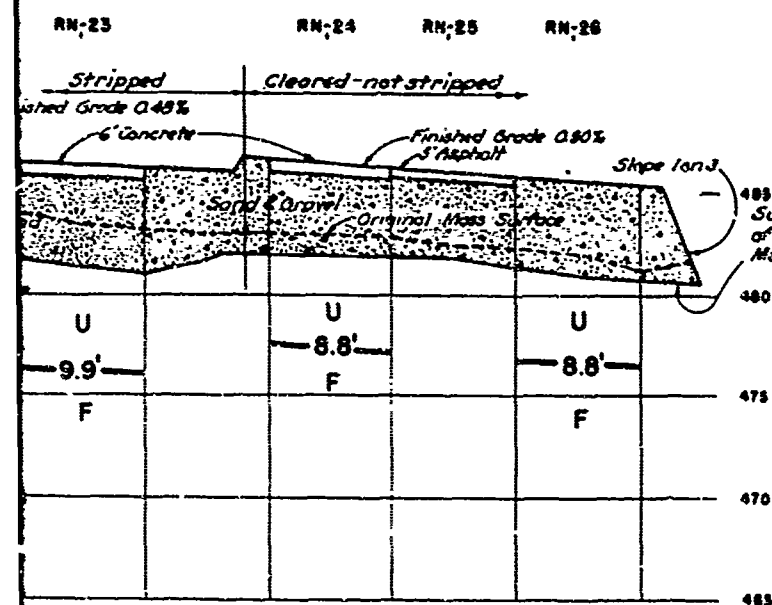
CORPS OF ENGINEERS, ST. PAUL, MINN.
 DRAWN BY E.C.T. TRACED BY W.J.E. MAY 1950
 CHECKED BY





LEGEND

- ⊕ 30' Core boring (Ground temperature hole)
- ⊕ 15' Churn drill boring (Ground temperature hole)
- ◆— Frost observation point.
- 5&6 Pit run sand and gravel.
- 9.2- Limit of frozen and unfrozen zones with depth below surface of section.
- F Frozen zone.
- U Unfrozen zone.



Note:

At ground temperature hole R-201, south to permafrost 4.6'.
At ground temperature hole R-204, depth to permafrost 6.0'.

PERMAFROST INVESTIGATION FIELD RESEARCH FAIRBANKS, ALASKA

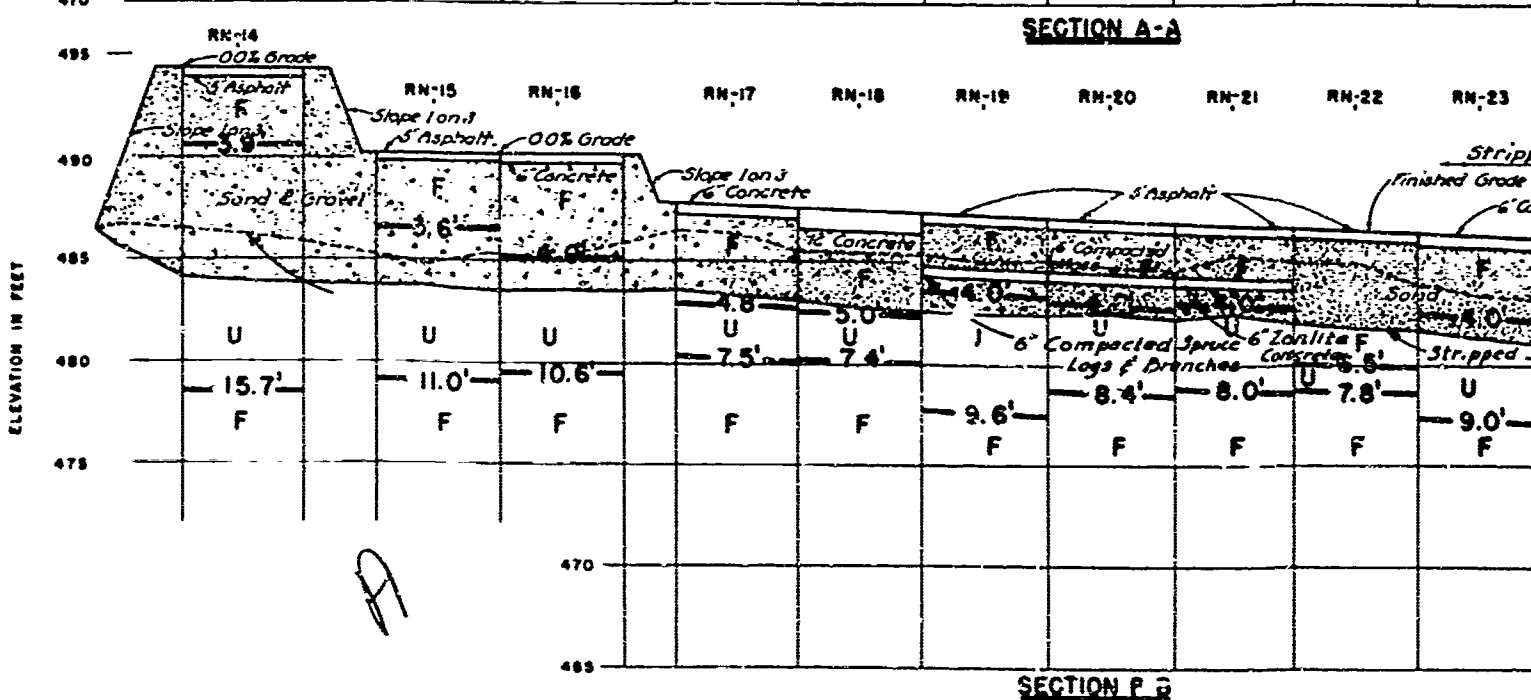
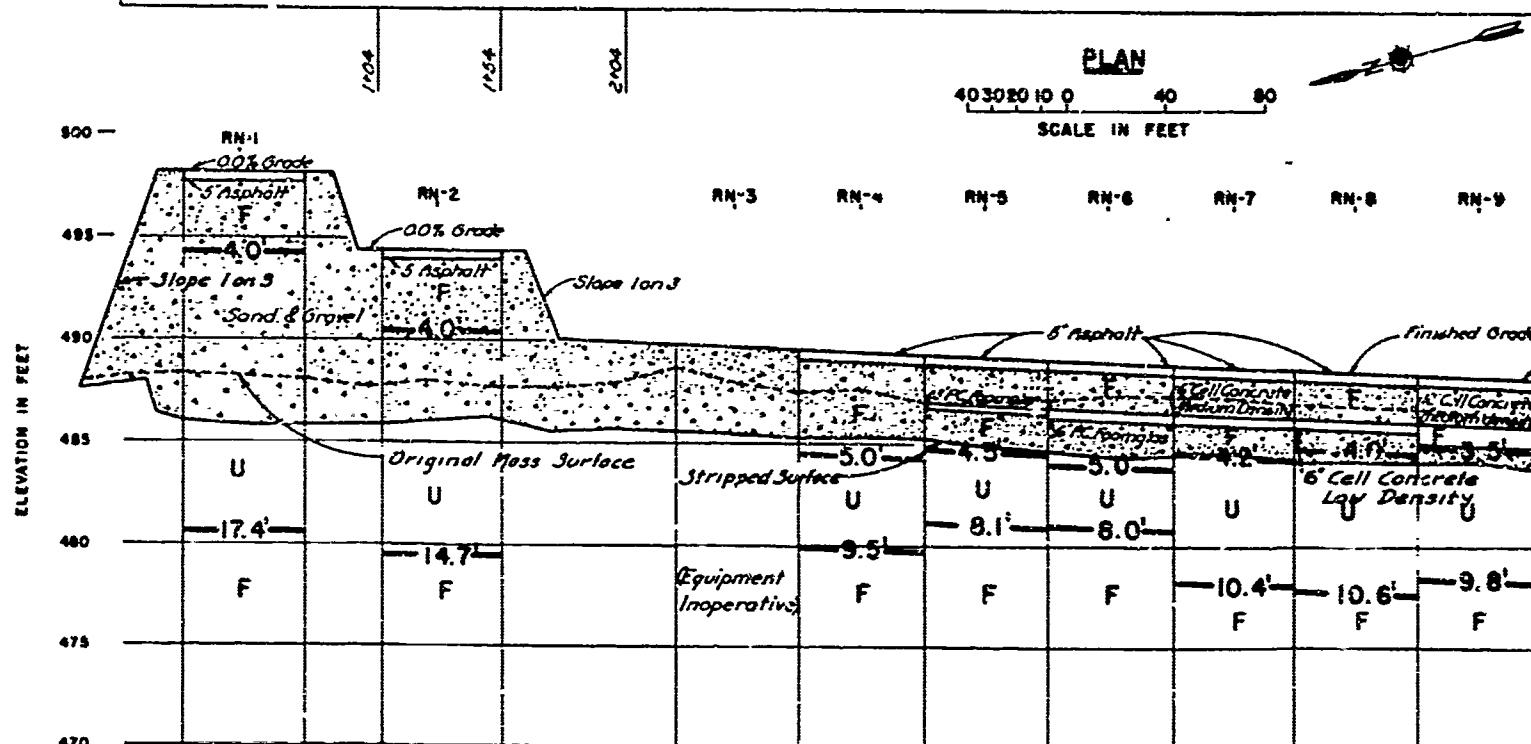
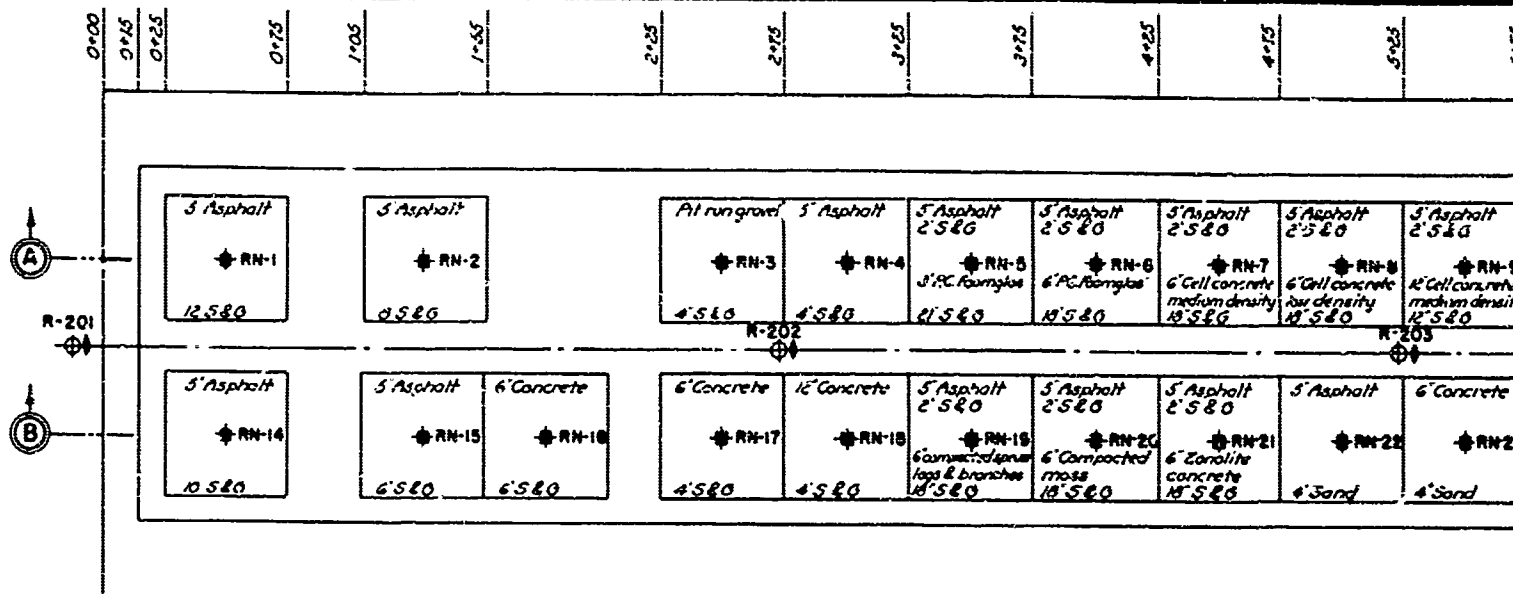
AREA NO. 2

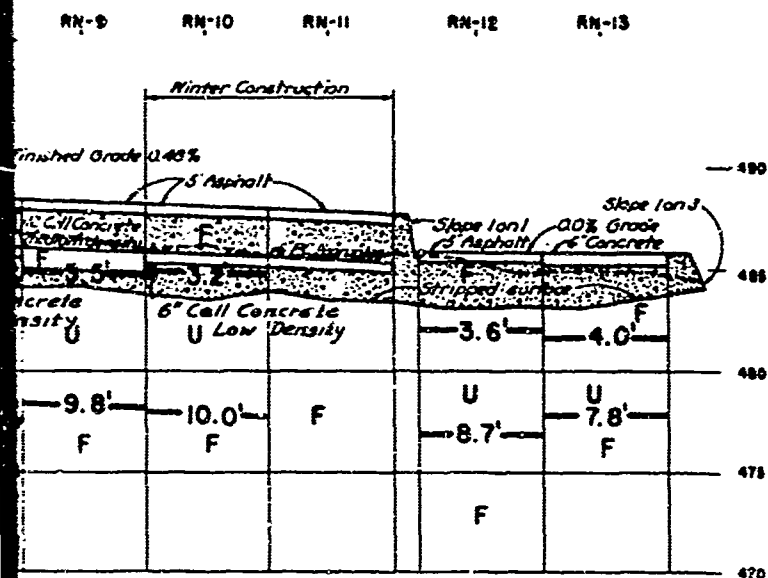
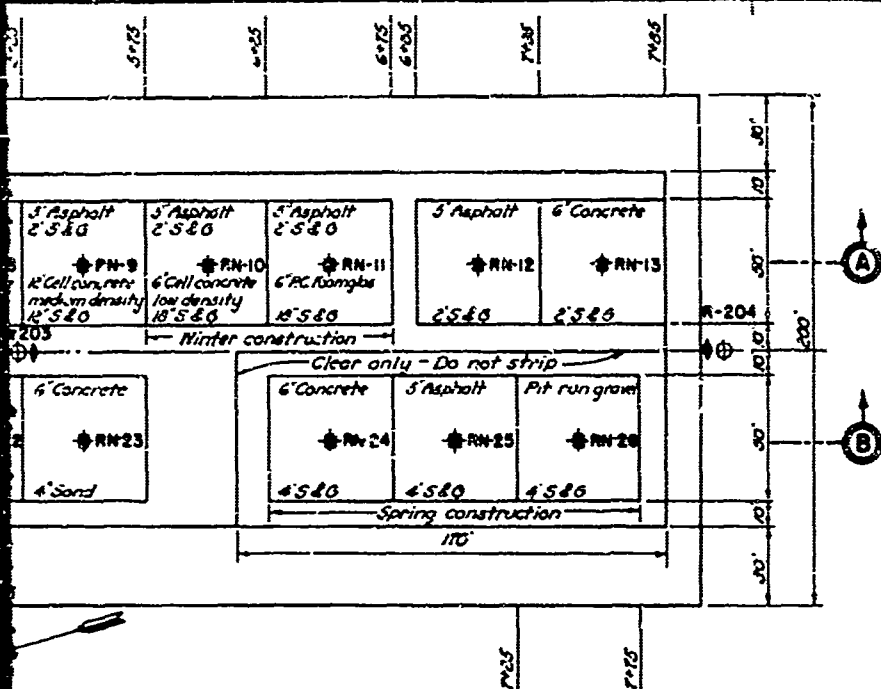
RUNWAY FOUNDATION STUDIES
FROZEN AND UNFROZEN ZONES
1 OCTOBER 1948

SCALE AS SHOWN

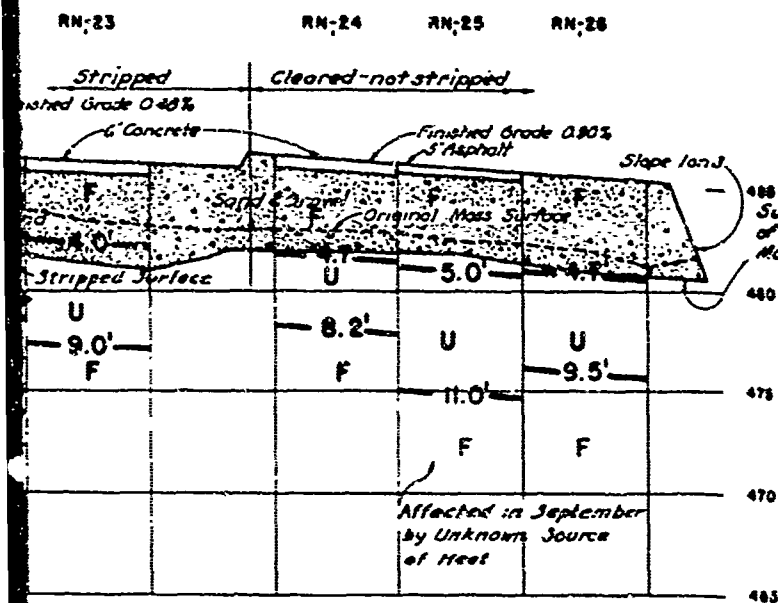
CORPS OF ENGINEERS, ST. PAUL, MINN.
DRAWN BY: E. C. T. TRACED BY: M. J. S.

MAY 1950
CHECKED BY:





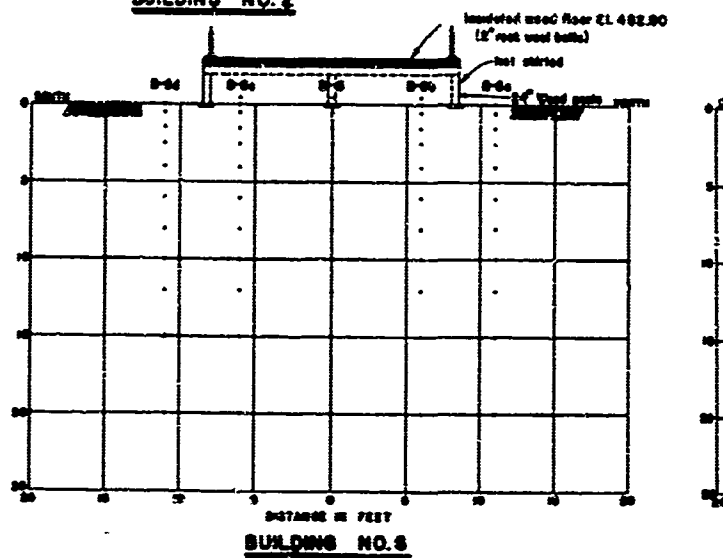
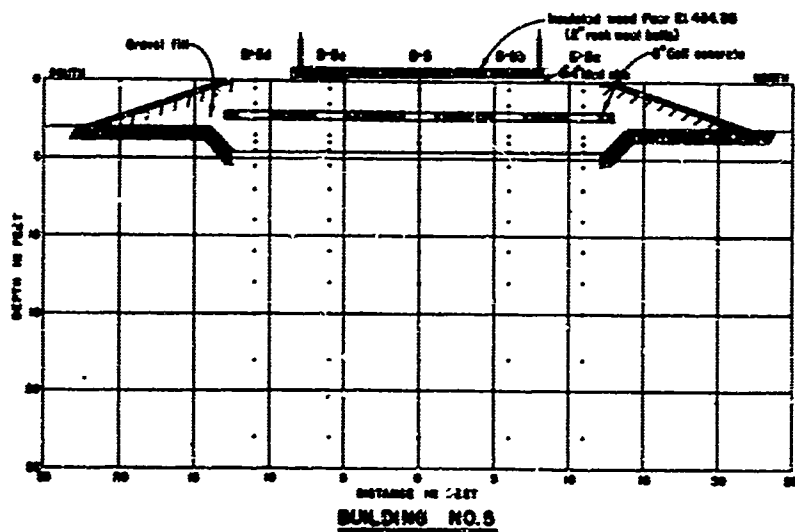
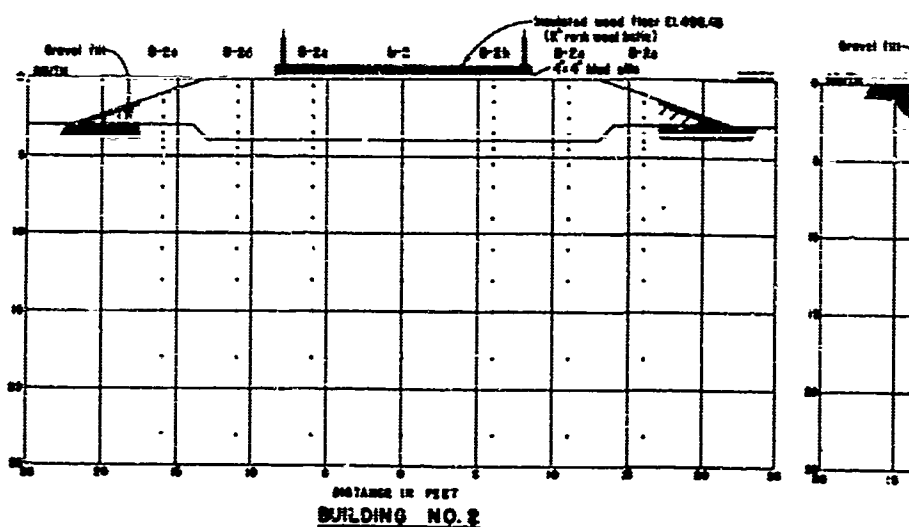
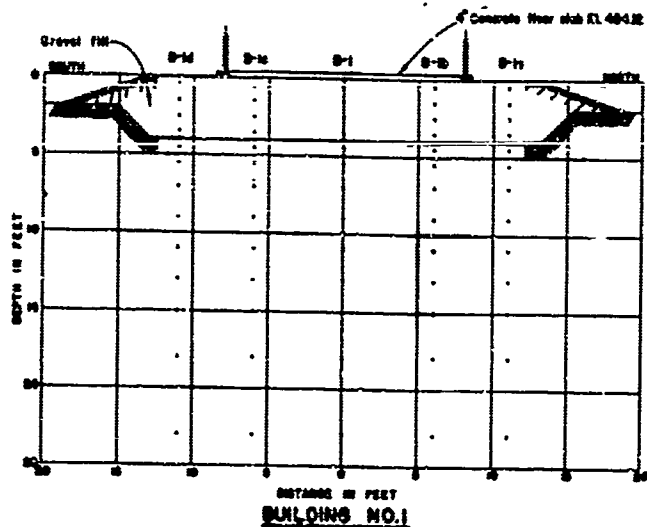
- LEGEND**
- ◆ 30' Core boring (Ground temperature hole)
 - ◆ 15' Churn drill boring (Ground temperature hole)
 - ◆ Frost observation point.
 - 5&G Pit run sand and gravel.
 - 92- Limit of frozen and unfrozen zones with depth below surface of section.
 - F Frozen zone
 - U Unfrozen zone

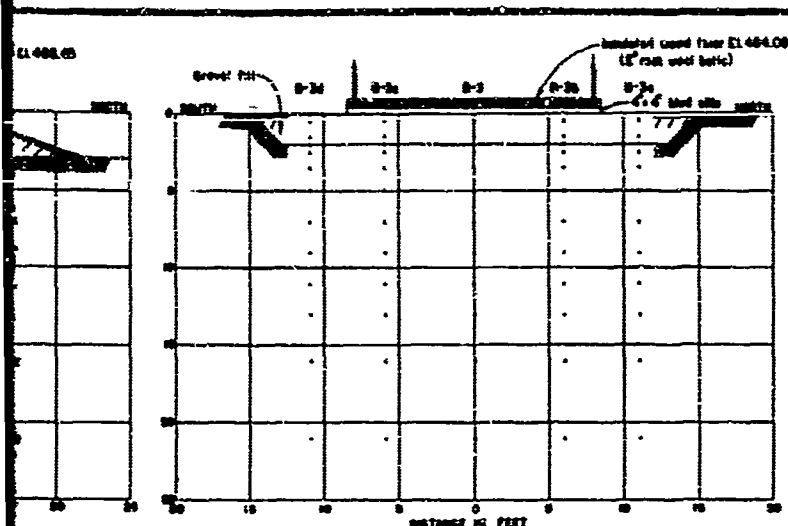


Note:
 At ground temperature hole R-201, depth to permafrost = 4.6'.
 At ground temperature hole R-204, depth to permafrost = 6.0'.

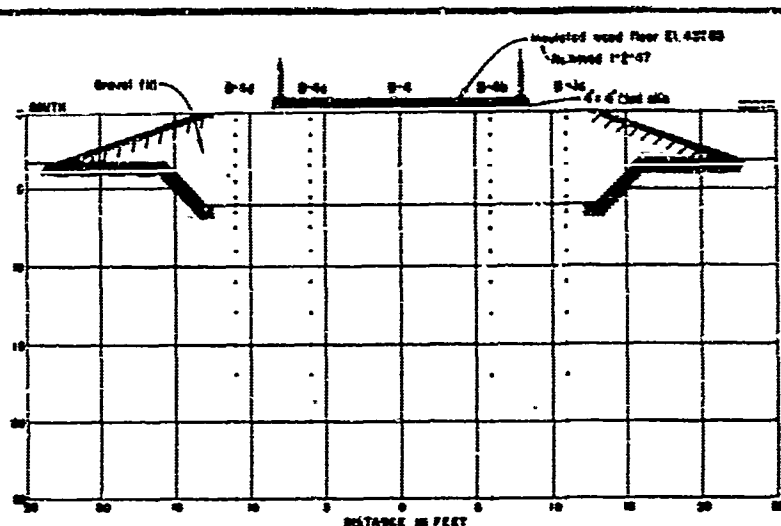
PERMAFROST INVESTIGATION FIELD RESEARCH FAIRBANKS, ALASKA AREA NO. 2 RUNWAY FOUNDATION STUDIES FROZEN AND UNFROZEN ZONES 31 OCTOBER 1948 SCALE AS SHOWN

CORPS OF ENGINEERS, ST. PAUL, MINN.
 DRAWN BY: E.C.T. TRACED BY: M.J.S. MAY 1950
 CHECKED BY:

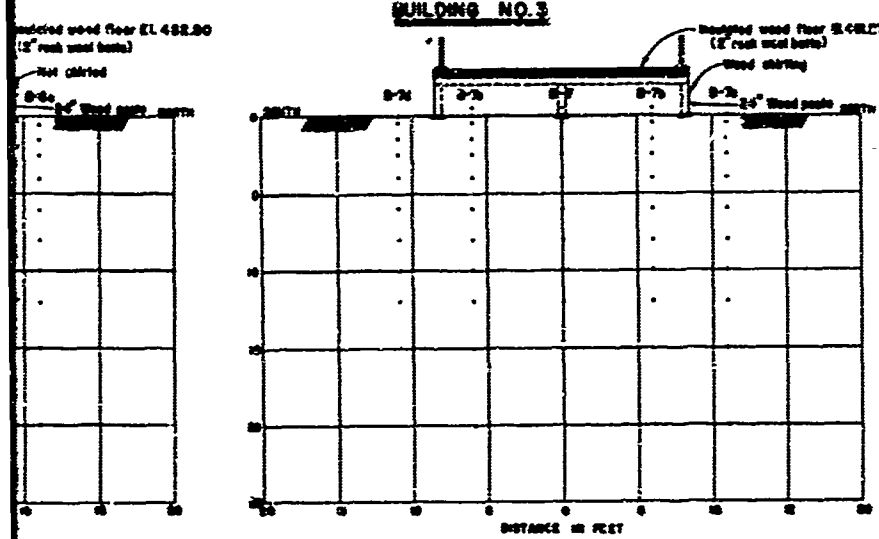




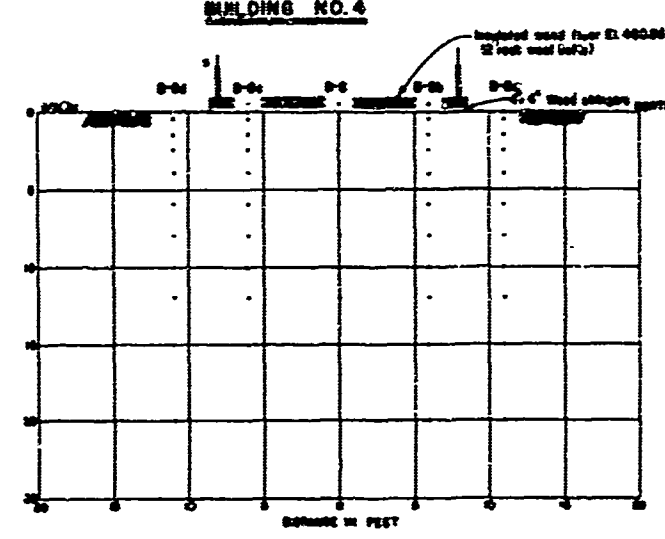
BUILDING NO. 3



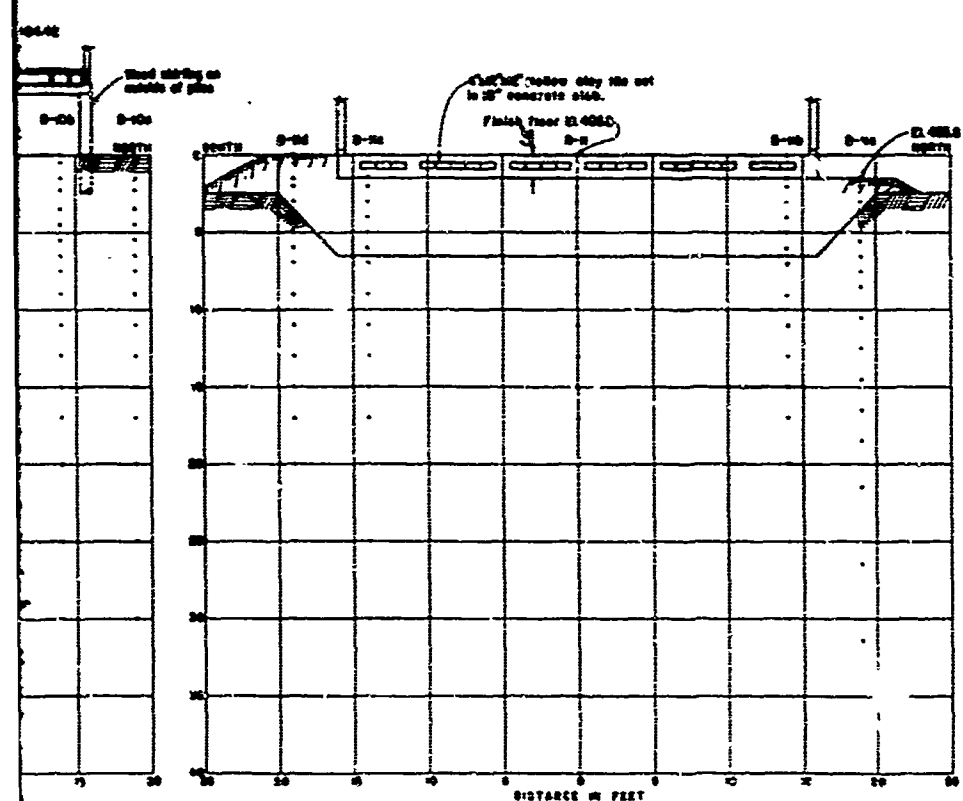
BUILDING NO. 4



BUILDING NO. 7



BUILDING NO. 8



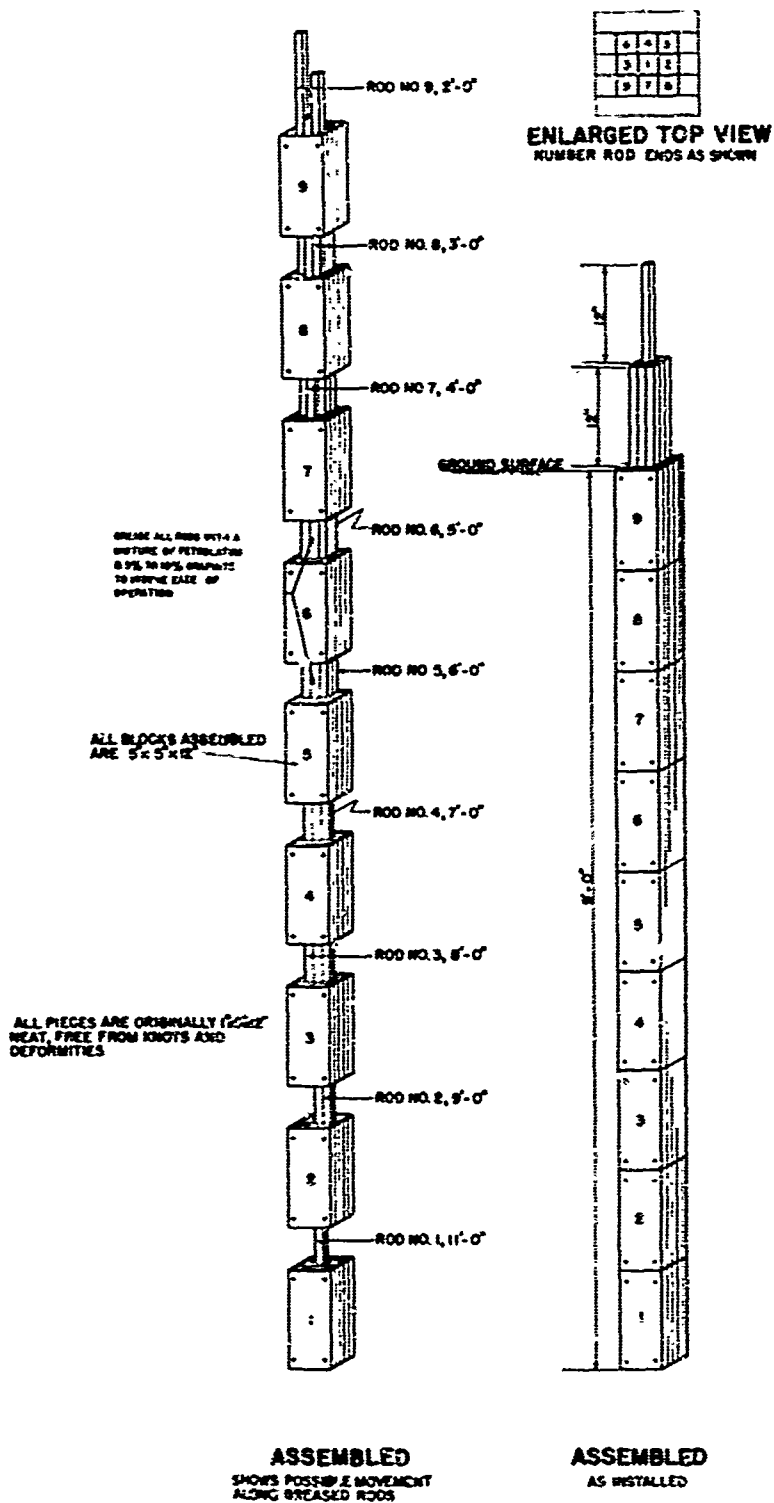
BUILDING NO. 11

NOTE:
1. LEATHCO THE STRAPPING USES BUILDINGS AT LOW POINT OF SLOPE TO PROVIDE DRAINAGE.
2. IN ORDER TO TEST THE EFFECT OF AN INSULATED WOOD FLOOR, THE INSULATION WAS REMOVED FROM THE FLOOR OF BUILDING NO. 4 ON 2 JANUARY 1947.

**PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
BUILDING FOUNDATIONS**

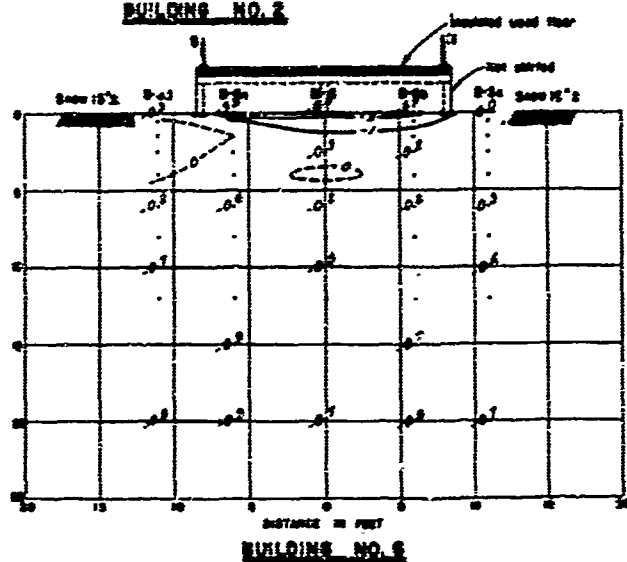
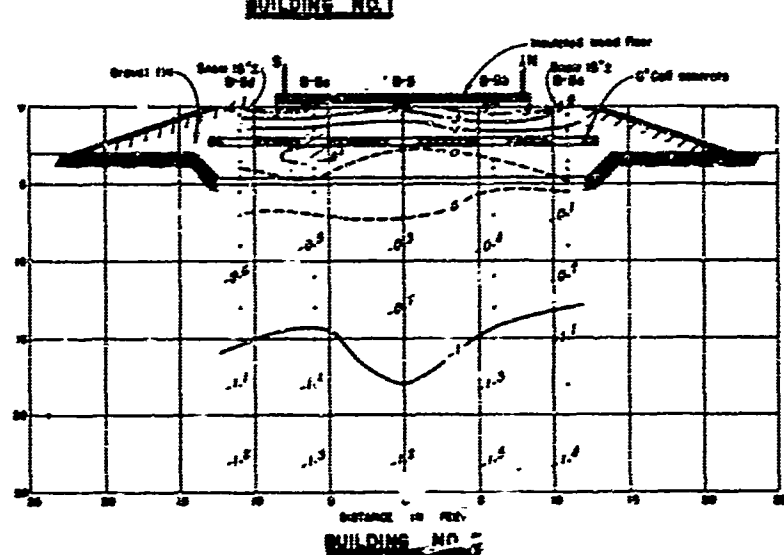
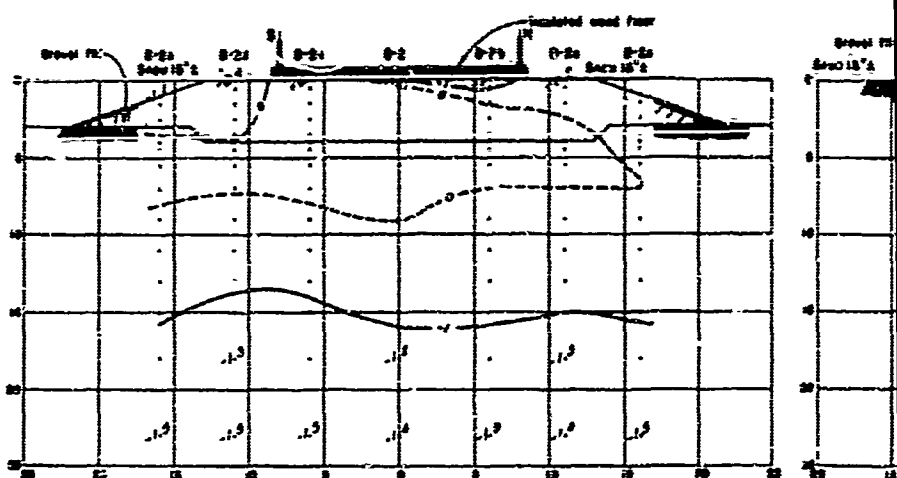
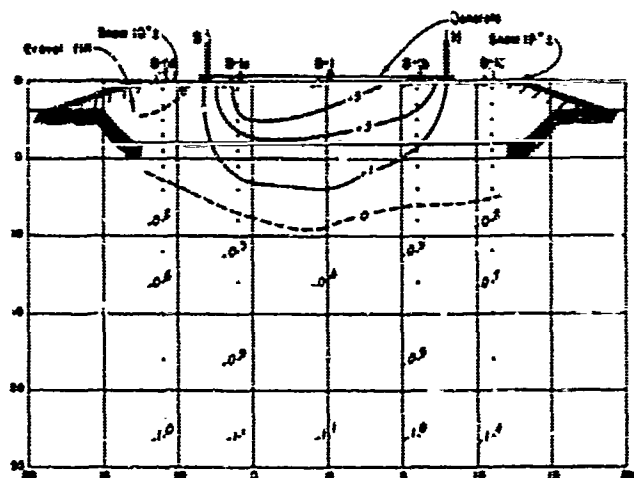
SCALE AS SHOWN
COMPS OF ENGINEERS ST. PAUL, MINN. MAY 1950
DRAWN BY P.P.B. TRACED BY P.P.B. CHECKED BY

B

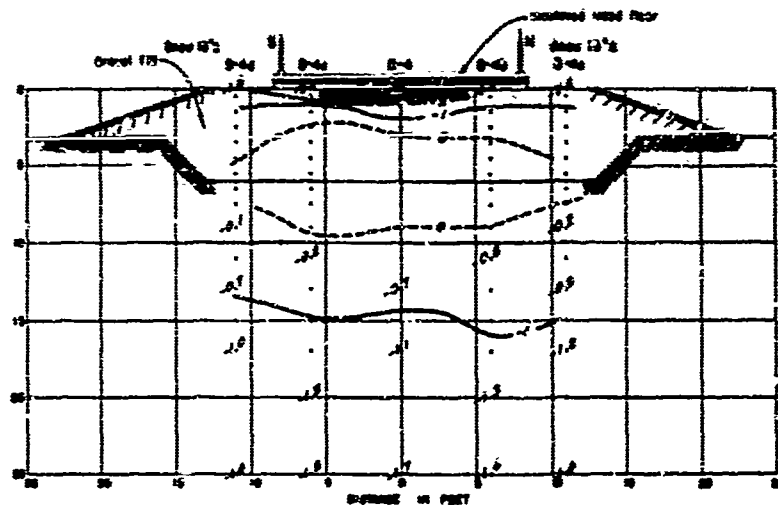


PERMAFROST INVESTIGATION
FIELD RESEARCH-FAIRBANKS, ALASKA
AREA NO. 3
DESIGN OF SWELLOMETER

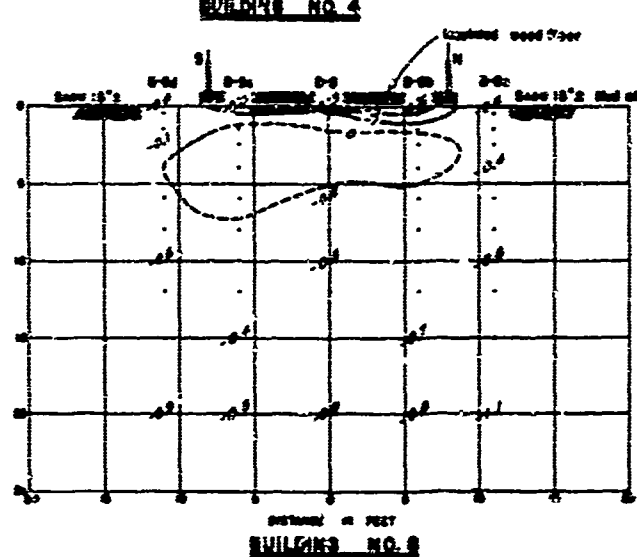
CORPS OF ENGINEERS, ST. PAUL, MINN. MAY 1950



A



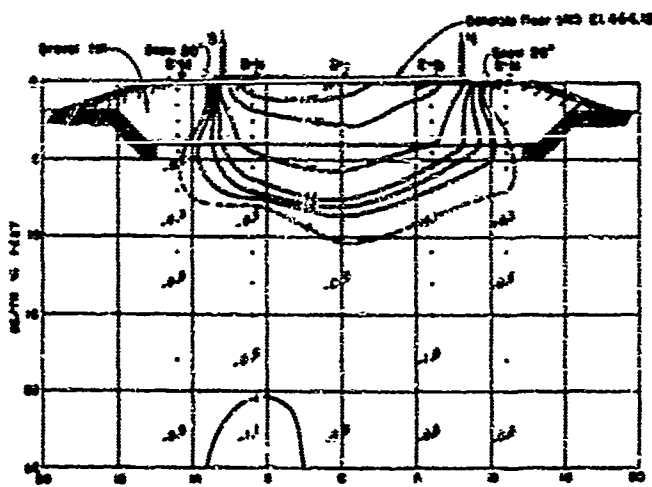
BUILDING NO. 4



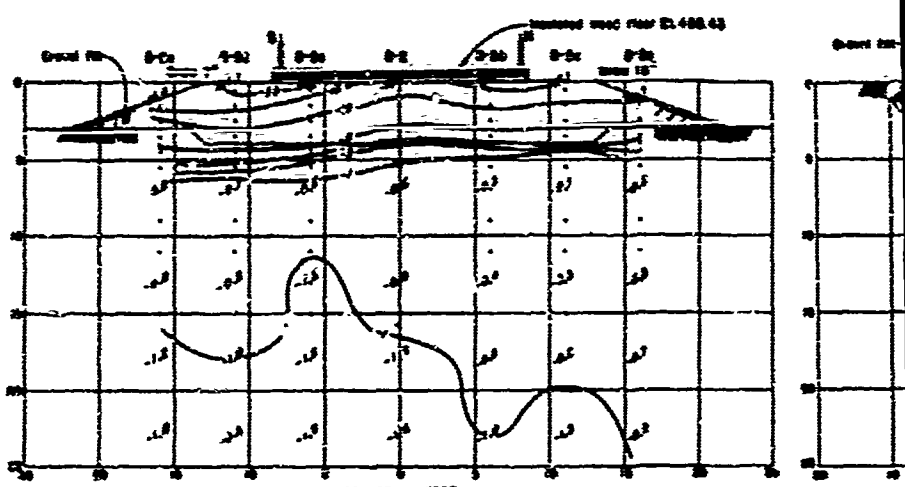
SCALE AS SHOWN

CORPS OF ENGINEERS ST PAUL, MINN MAY. 1900

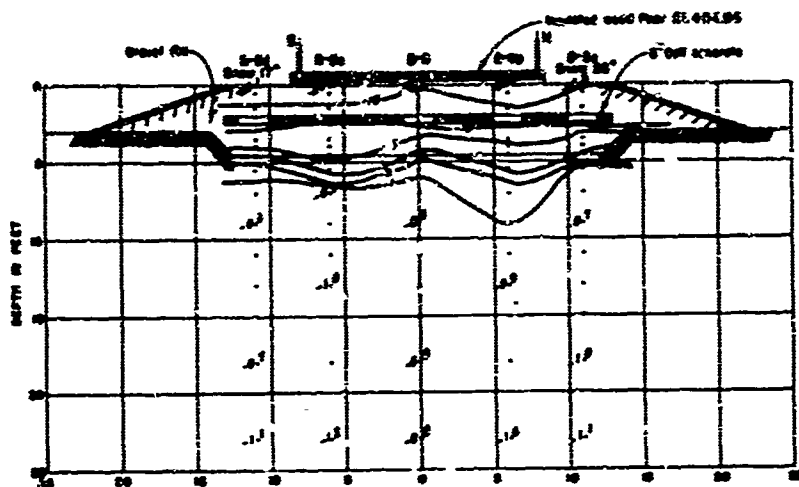
DRAWN BY C. S. TRASS BY R. F. CHECKED BY



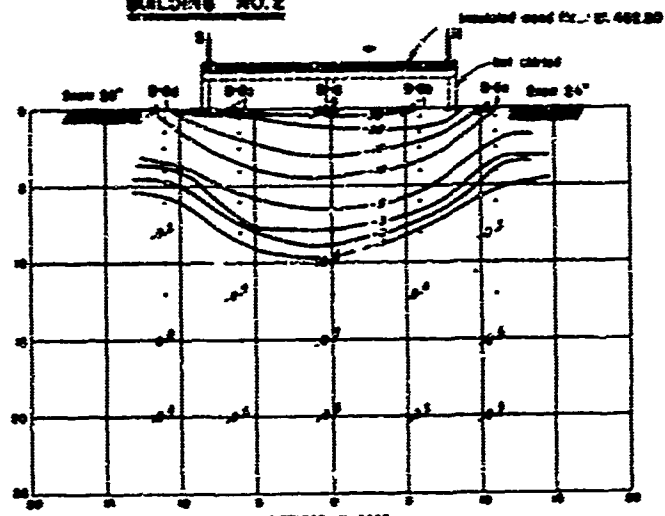
BUILDING NO. 1



BUILDING NO. 2

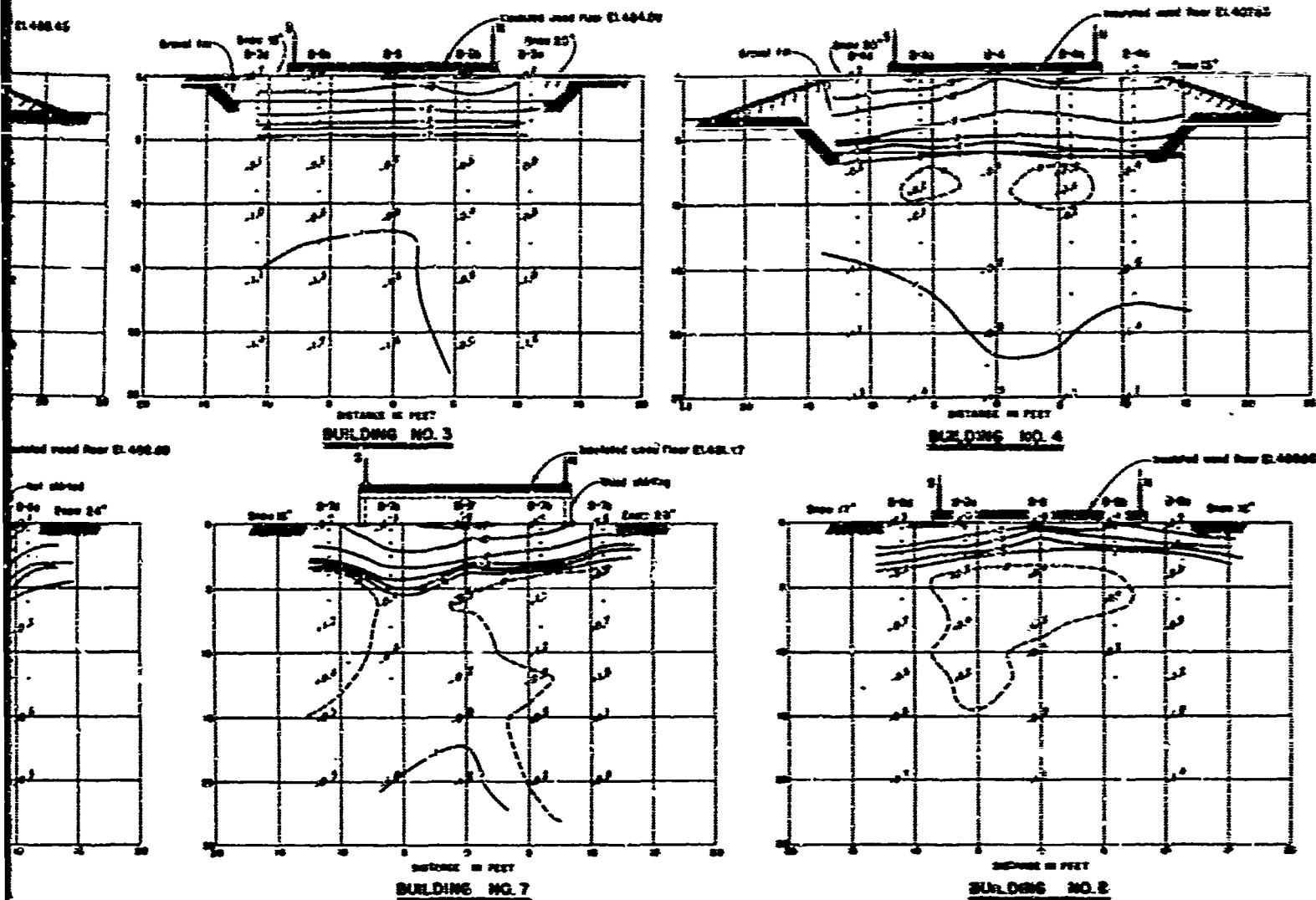


BUILDING NO. 3



BUILDING NO. 4

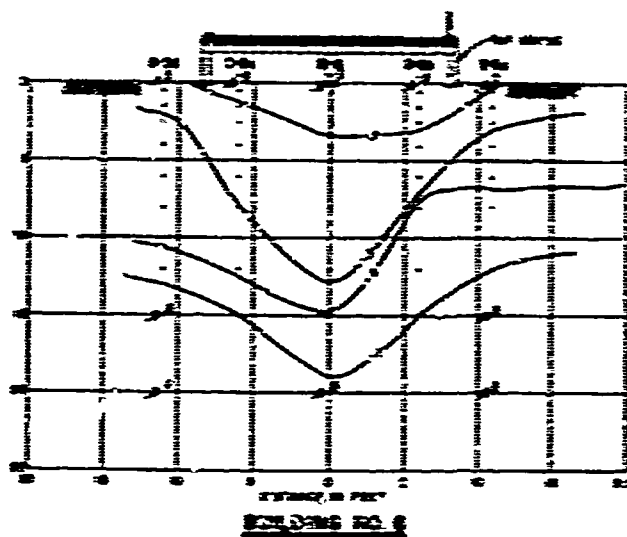
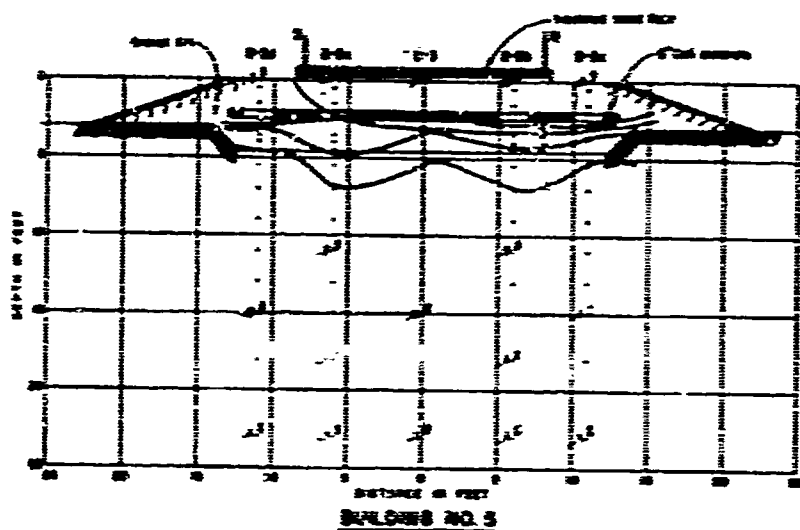
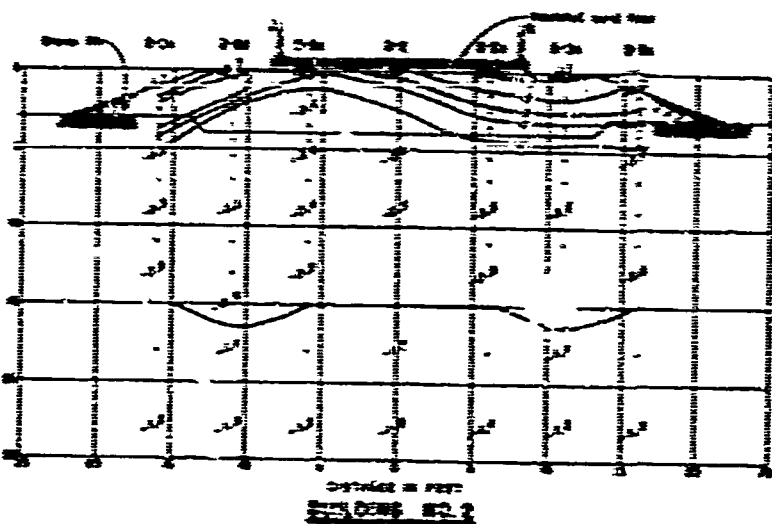
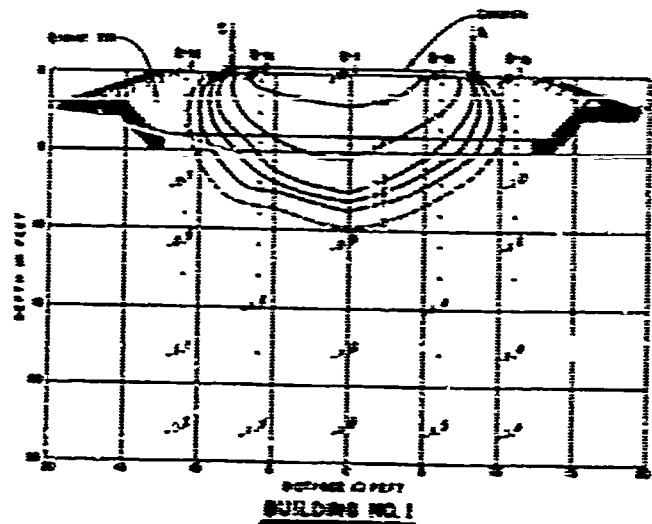
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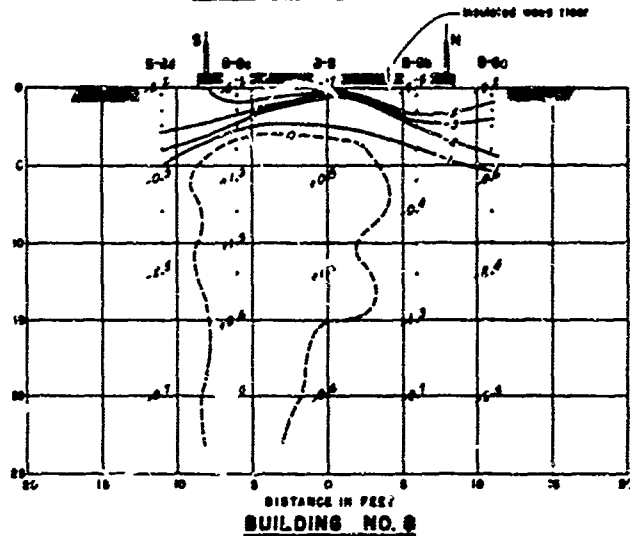
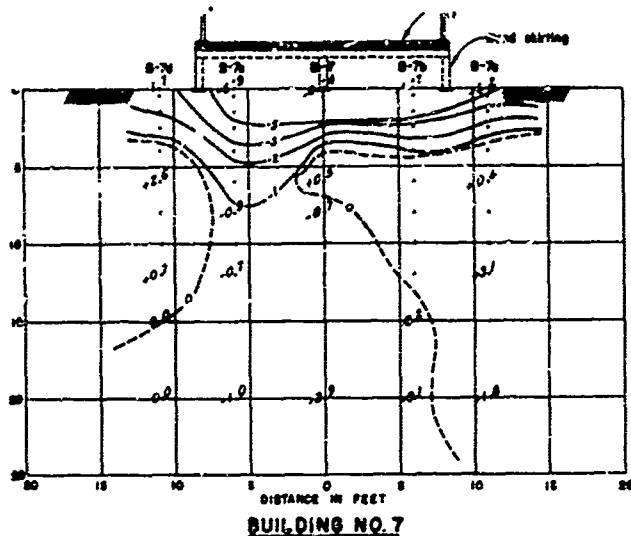
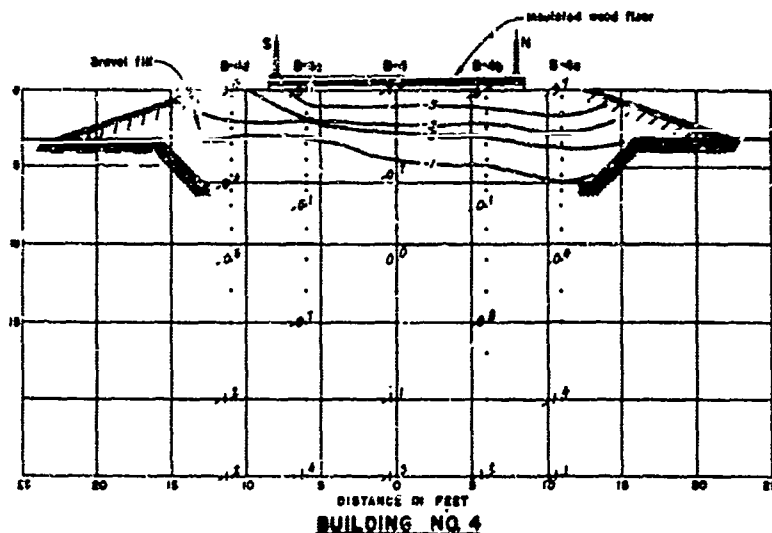
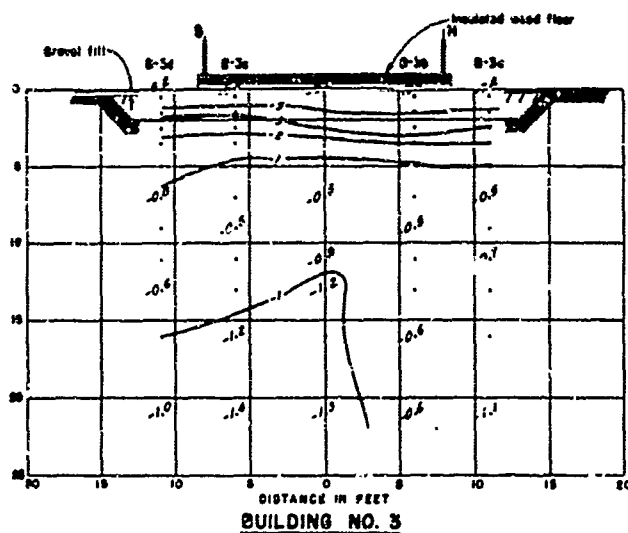
NOTE:
 Isoterm lines shown on plate 3E T
 Temperature observations shown
 in degrees Fahrenheit
 — Zero isotherm
 — Isotherms other than zero

PERMAFROST INVESTIGATION
 FIELD RESEARCH
 FAIRBANKS, ALASKA
 AREA NO. 3
 GROUND TEMPERATURE OBSERVATIONS
 1 FEBRUARY 1947

SCALE AS SHOWN
 CORPS OF ENGINEERS 11 PAUL, 1947 11" 1947
 DRAWN BY P. S. THOMAS BY P. S. THOMAS



R

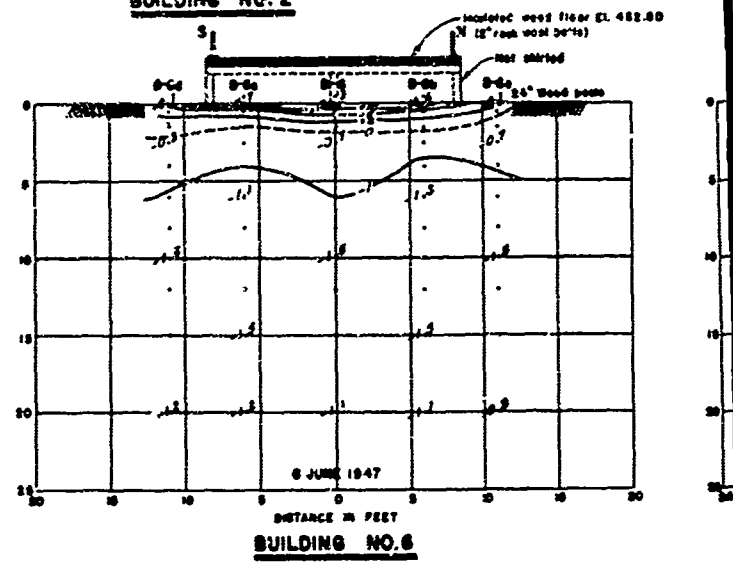
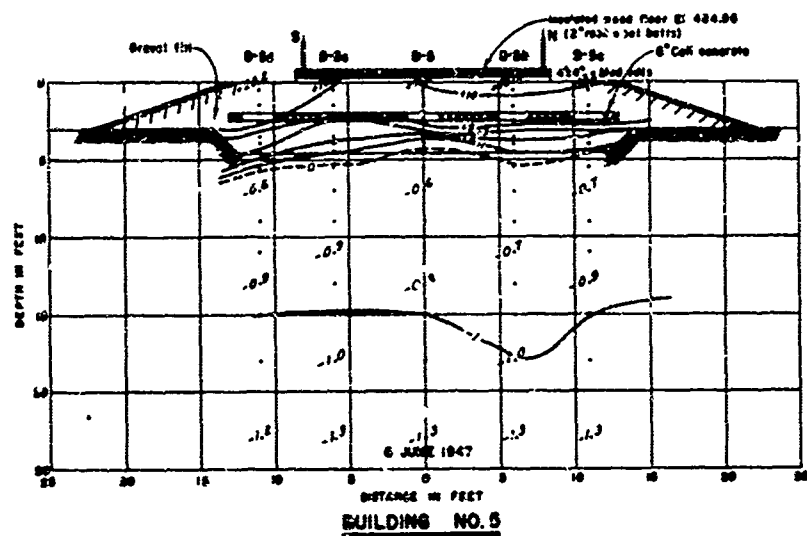
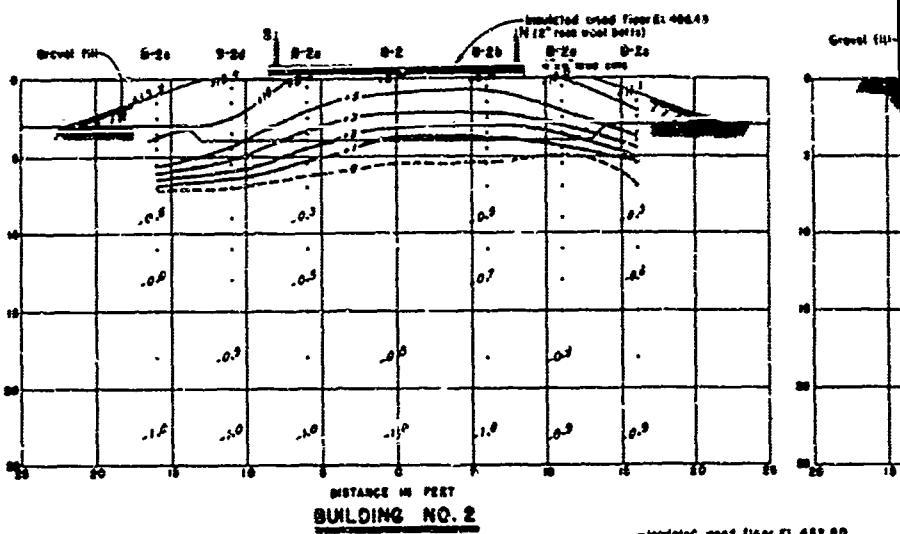
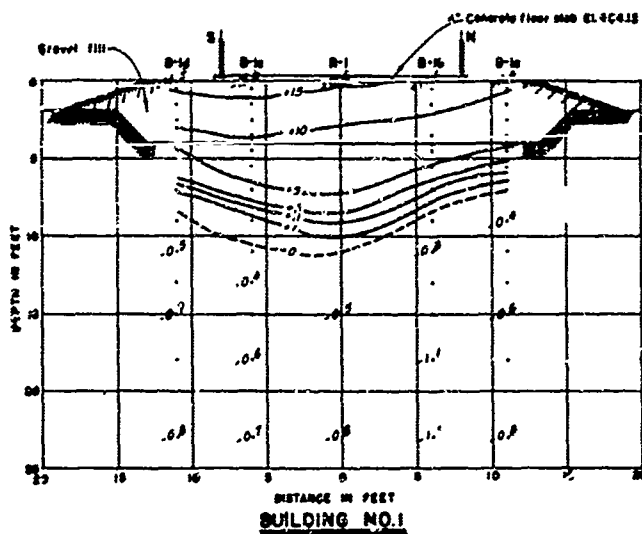


B

Note:
Building locations shown on plate III-7
Temperature observations shown
in degrees centigrade
----- Zero isotherm
----- Isotherms other than zero

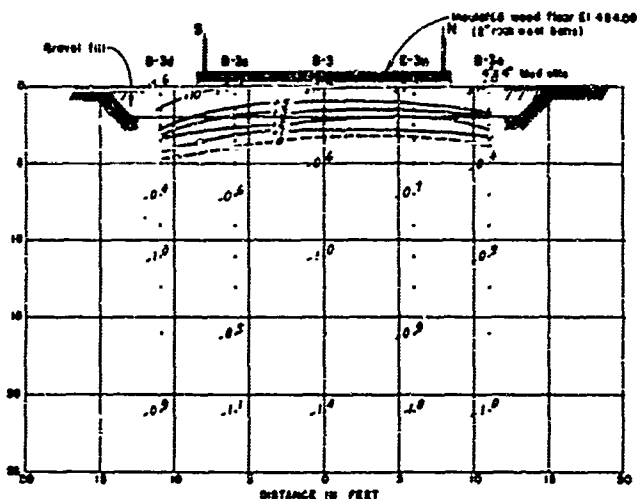
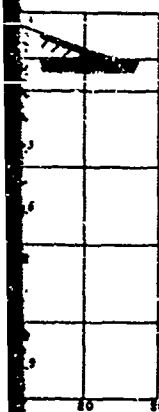
PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
GROUND TEMPERATURE OBSERVATIONS
1 APRIL 1947

SCALE: AS SHOWN
CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1949
DRAWN BY: R. R. G. TRACED BY: R. R. G. CHECKED BY:

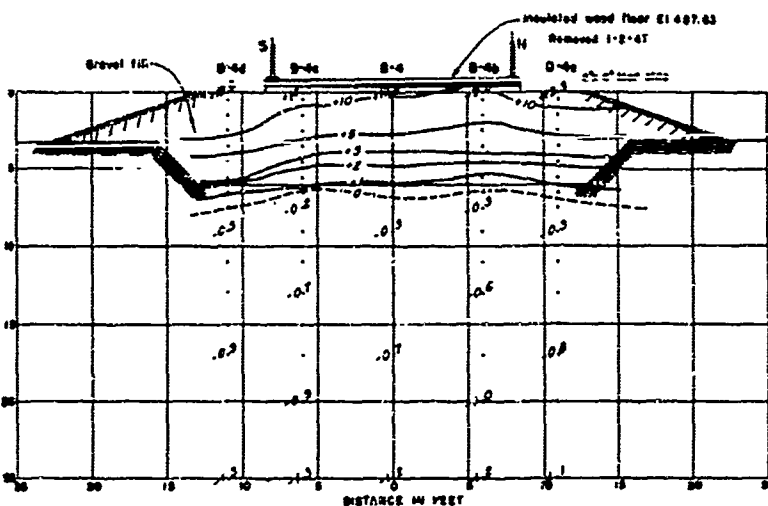


H

See E1 486.43
10)
2a

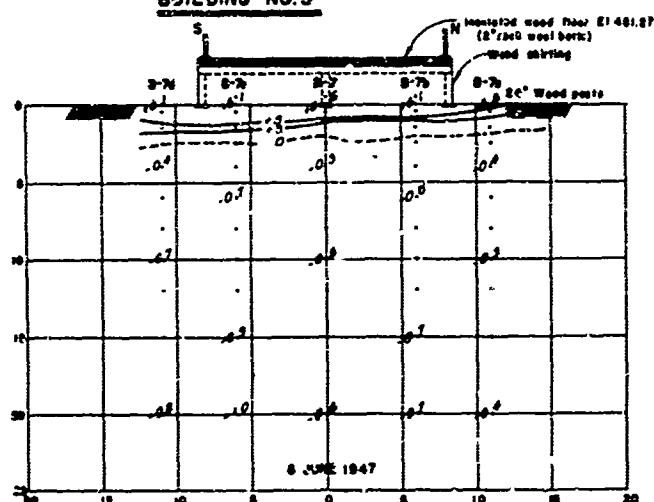
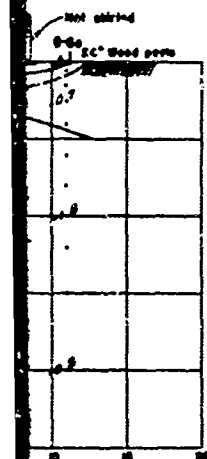


BUILDING NO. 3

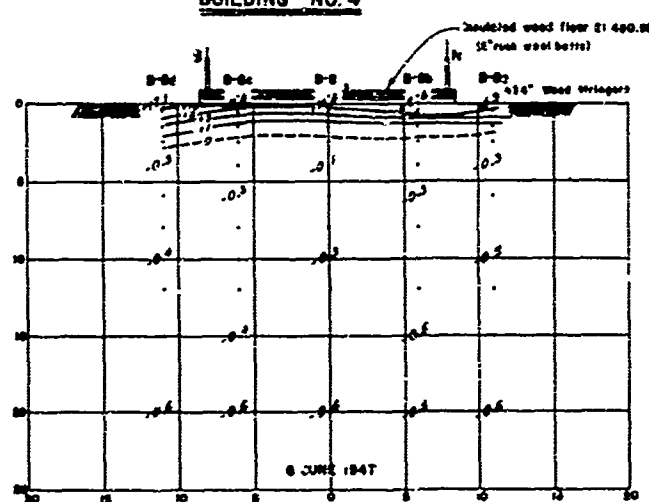


BUILDING NO. 4

Insulated wood floor E1 485.80
(8" run wool batts)



BUILDING NO. 7



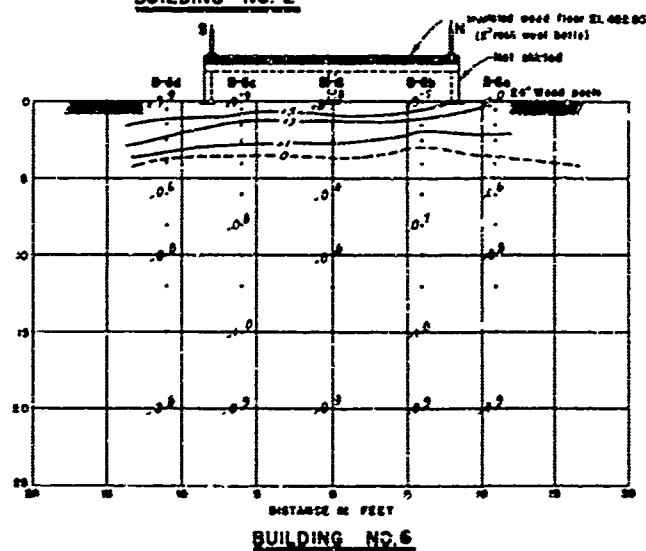
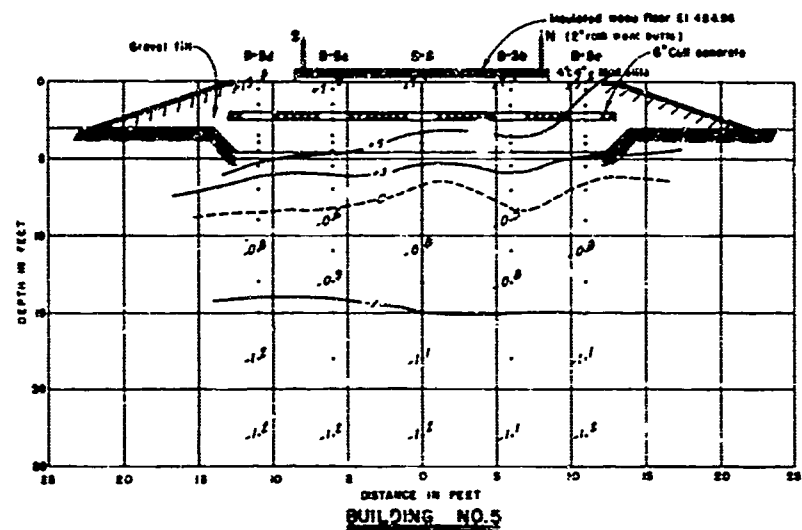
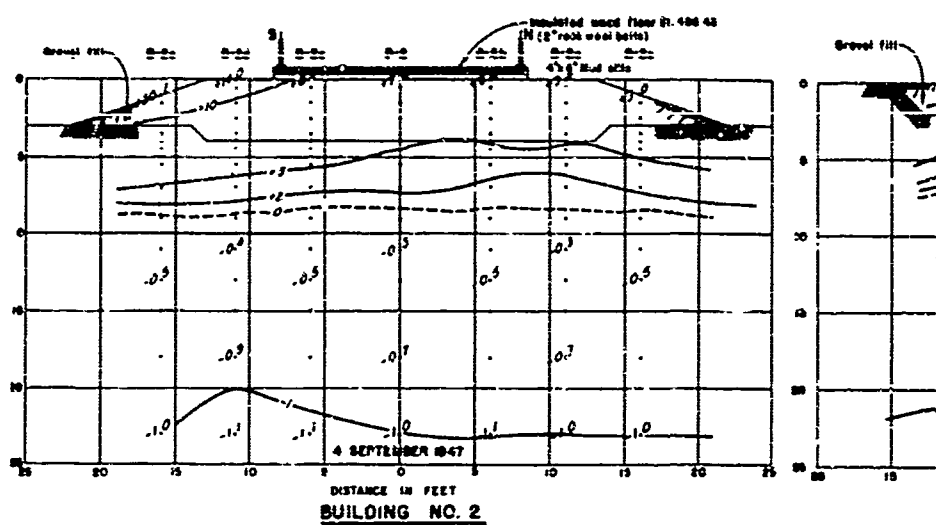
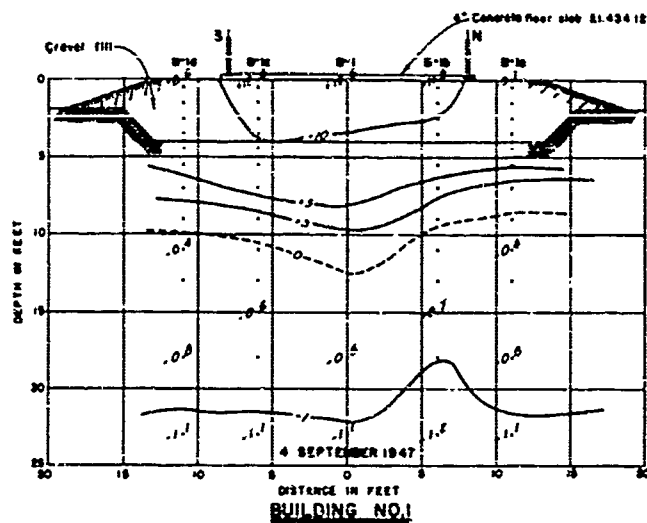
BUILDING NO. 8

B

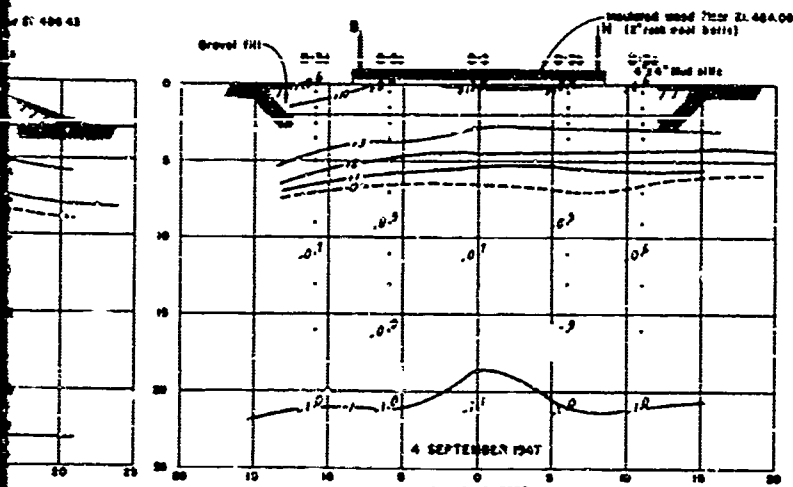
Note:
Building locations shown on plate III-7
Temperature observations shown
in degrees centigrade
----- Zero isotherm
———— Isotherms other than zero

**PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
GROUND TEMPERATURE OBSERVATIONS
5 JUNE 1947**

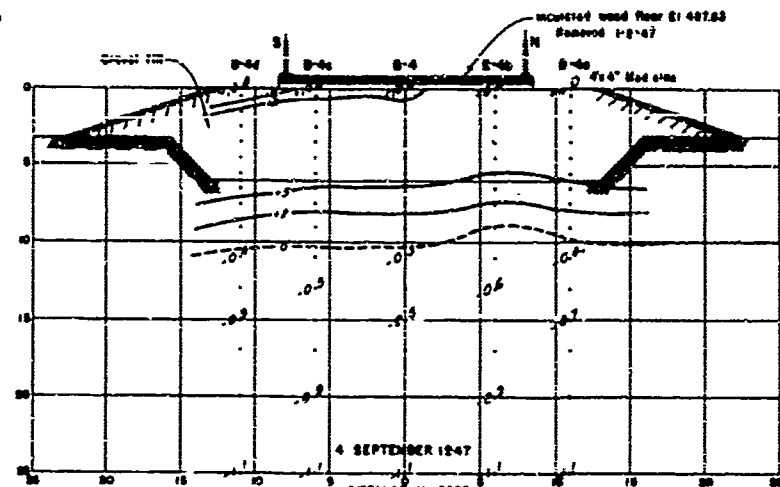
SCALE AS SHOWN
CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1950
DRAWN BY P. S. C. TRACED BY P. S. C. CHECKED BY



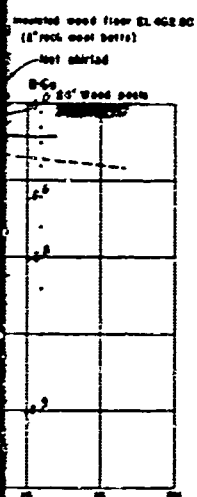
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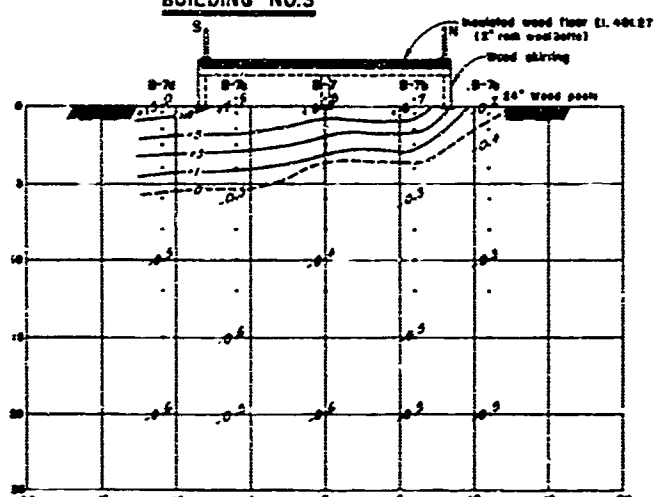
BUILDING NO. 3



BUILDING NO. 4



BUILDING NO. 7

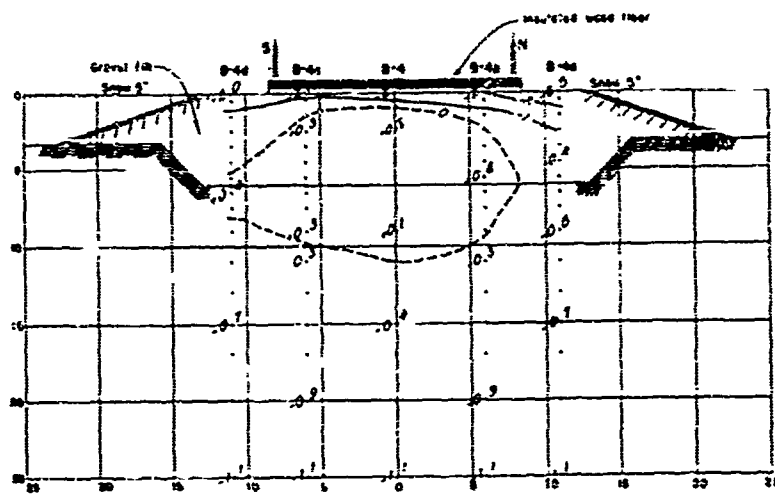


BUILDING NO. 8

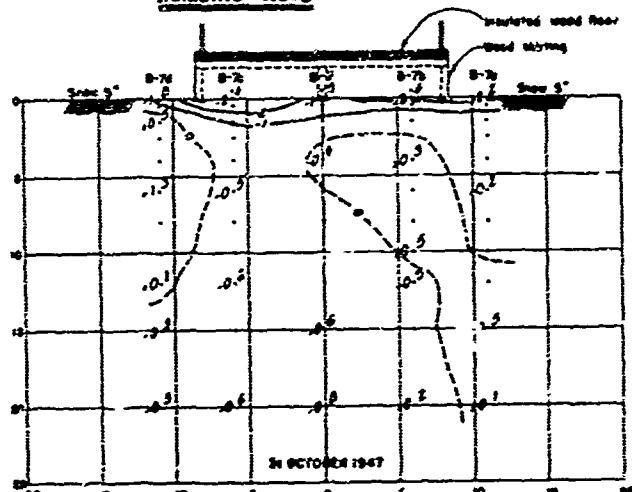
Note:
Boring locations shown on plate III-7
Temperature observations shown
in degrees centigrade
--- Zero isotherm
— Isotherms other than zero

**PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
GROUND TEMPERATURE OBSERVATIONS
5 SEPTEMBER 1947**

SCALE AS SHOWN
CORPS OF ENGINEERS ET AL., 1947 MAY 1950
DRAWN BY P. G. TRACED BY P. G. CHECKED BY



BUILDING NO. 4

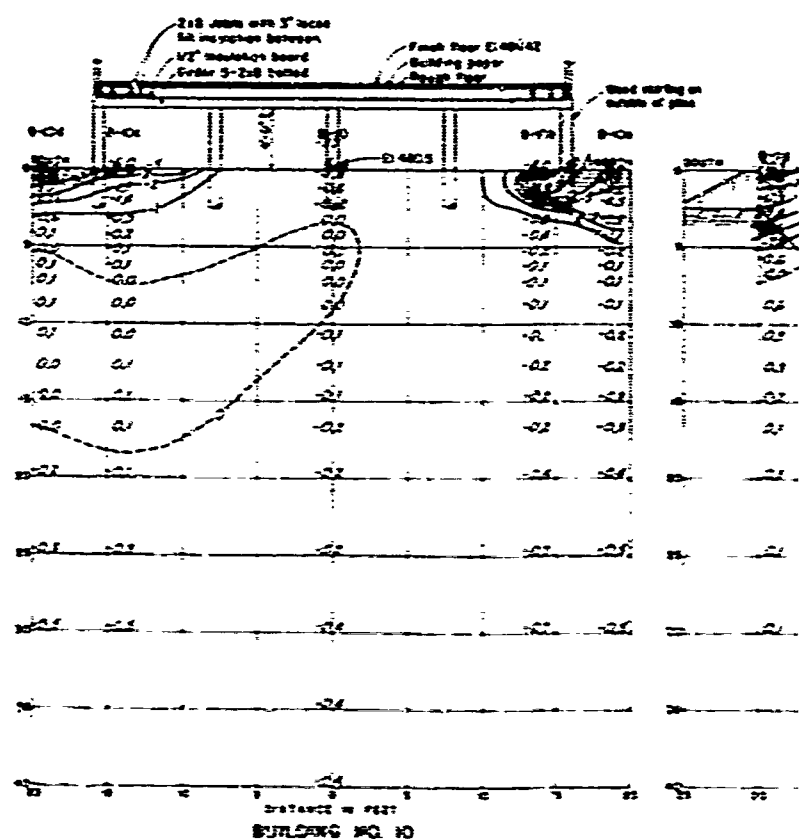
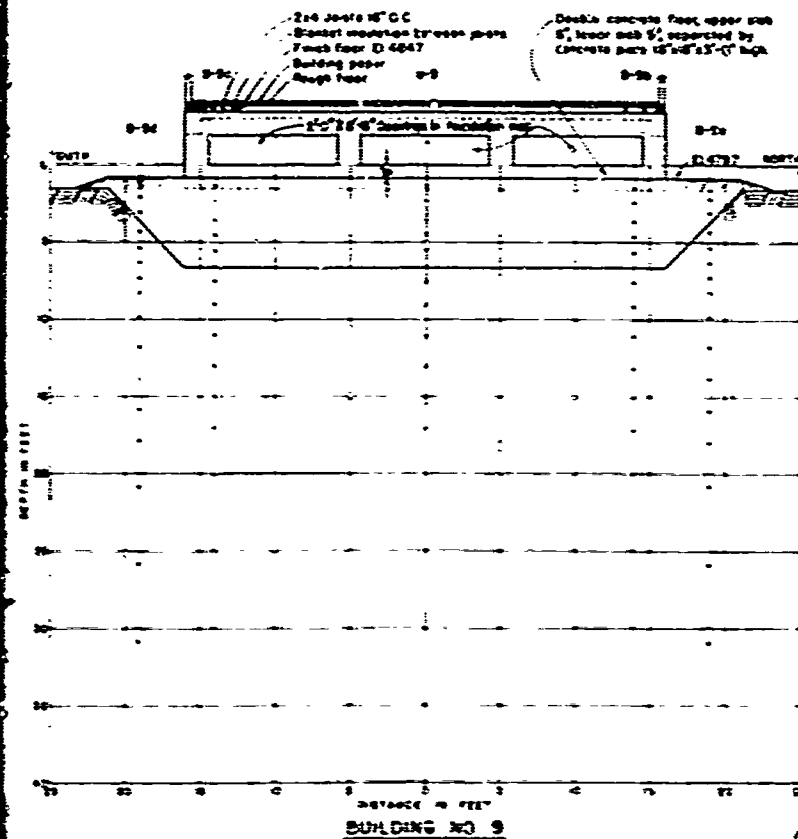
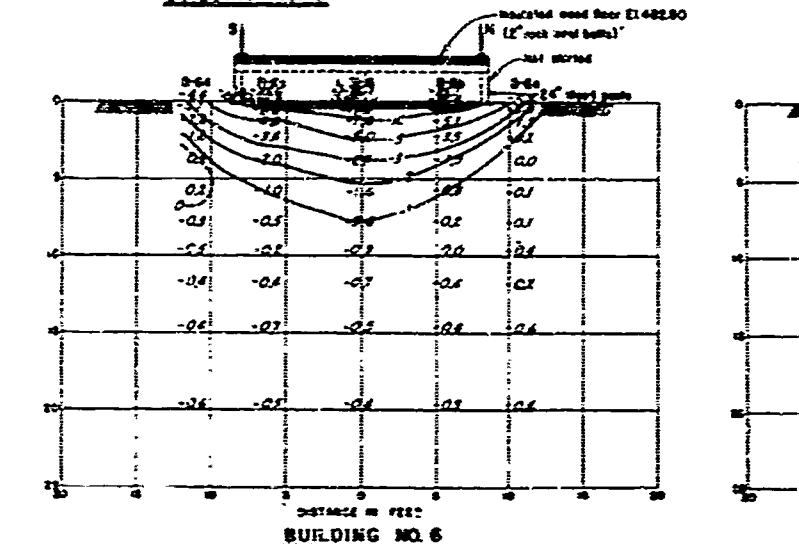
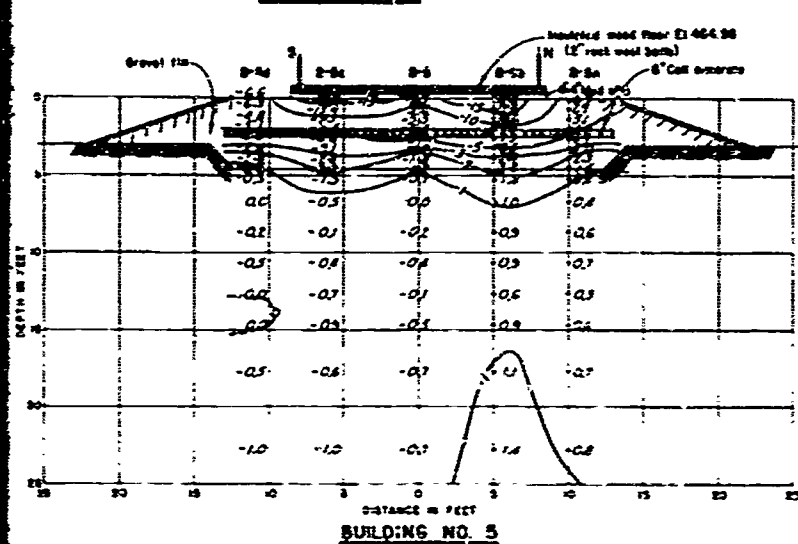
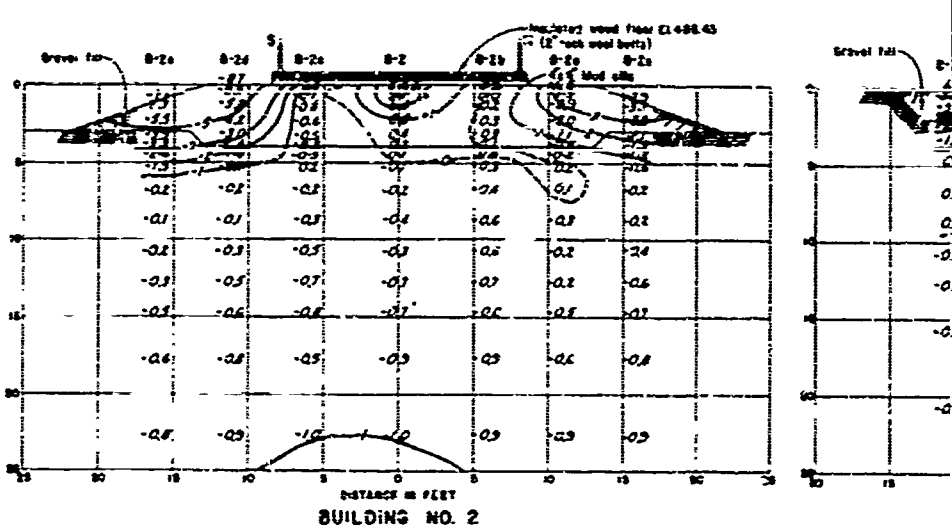
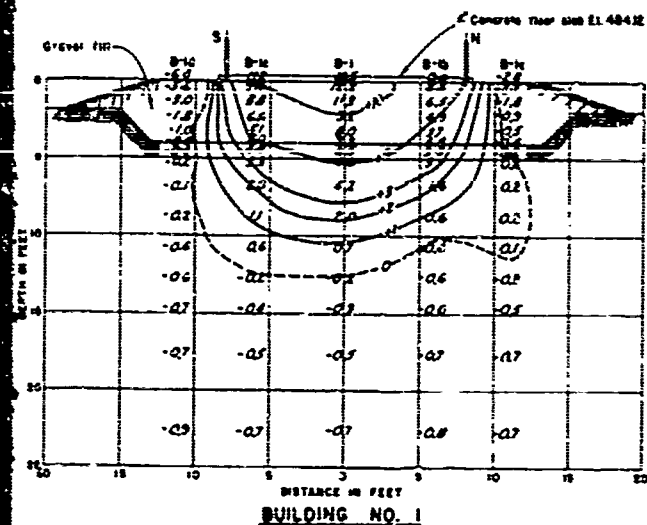


BUILDING NO.

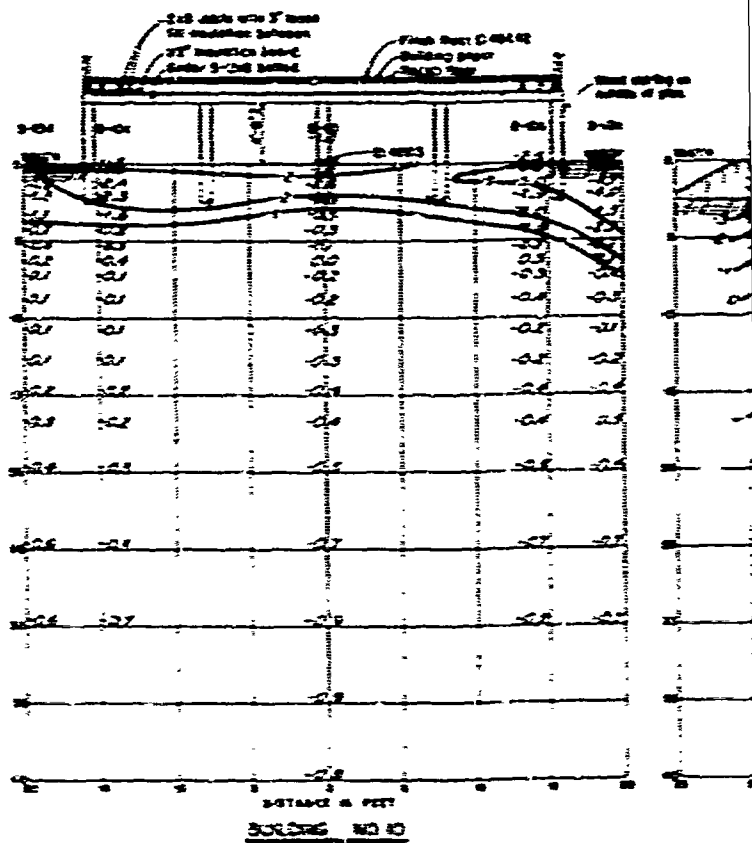
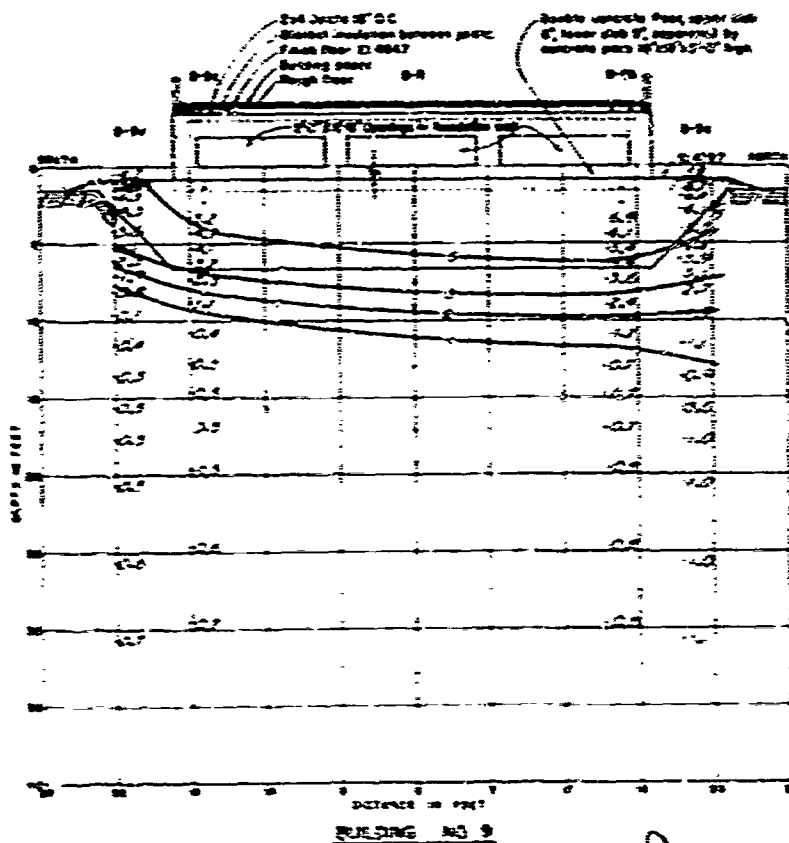
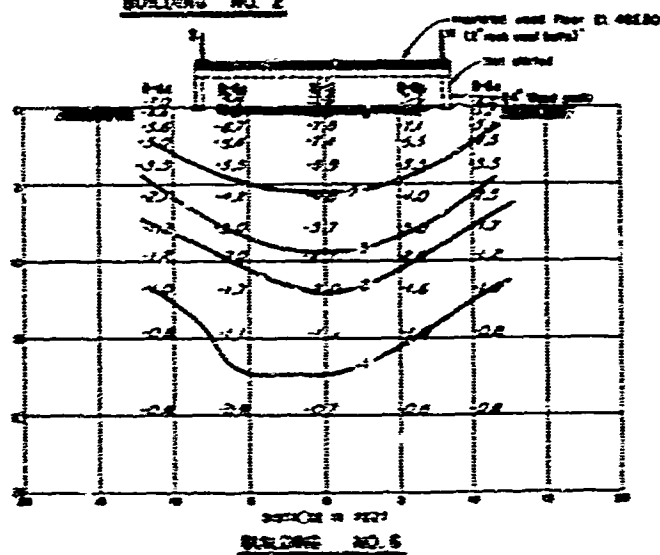
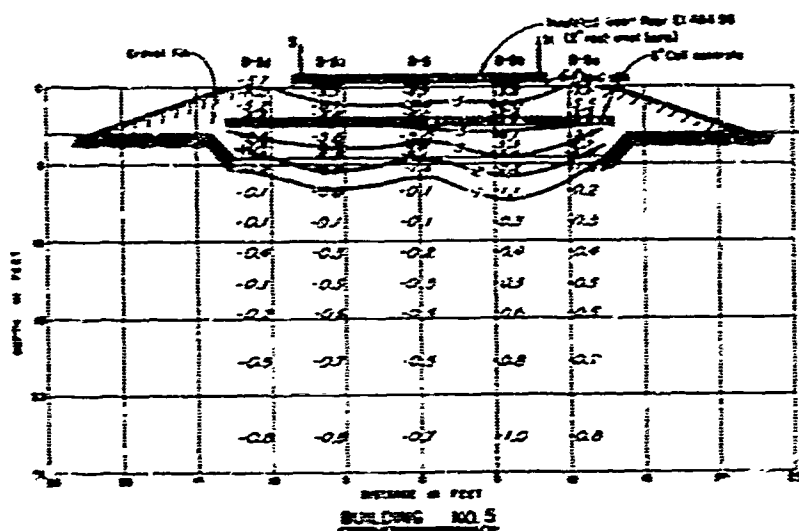
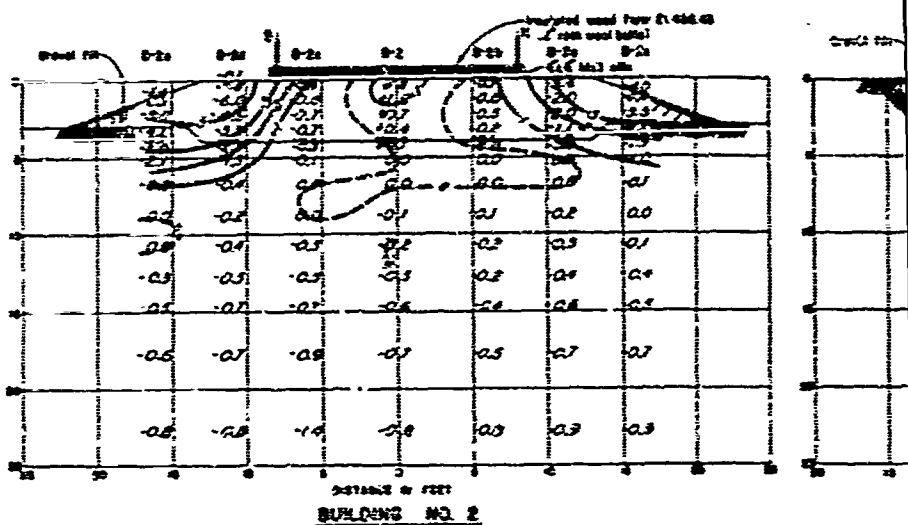
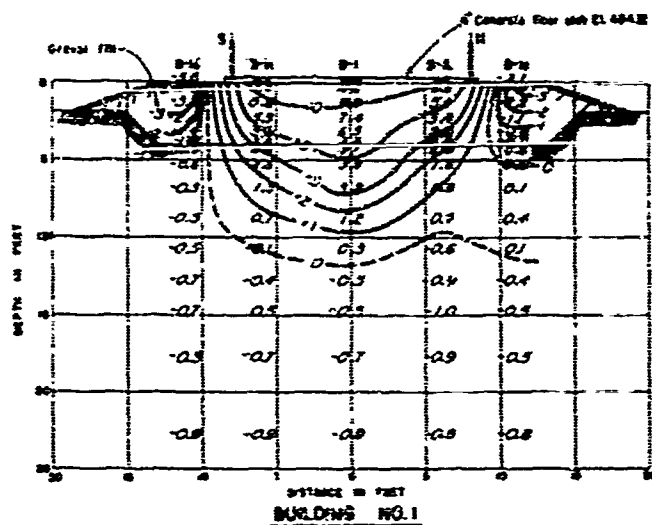
Notes.
Bottom locations shown on plate 22-7
Temperature observations shown in
degrees centigrade
----- Zero isotherm.
----- Isotherm after they are

PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
GROUND TEMPERATURE OBSERVATIONS
30 OCTOBER 1947

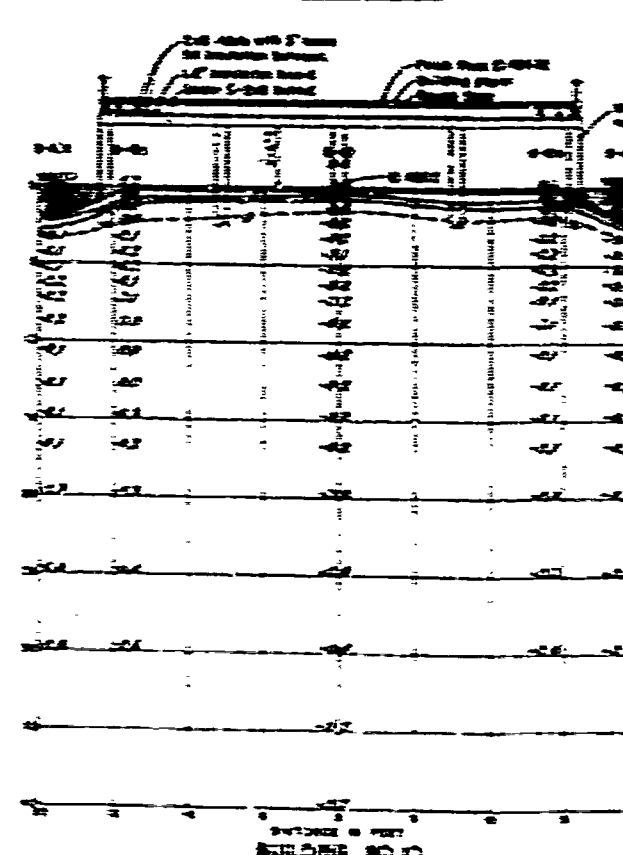
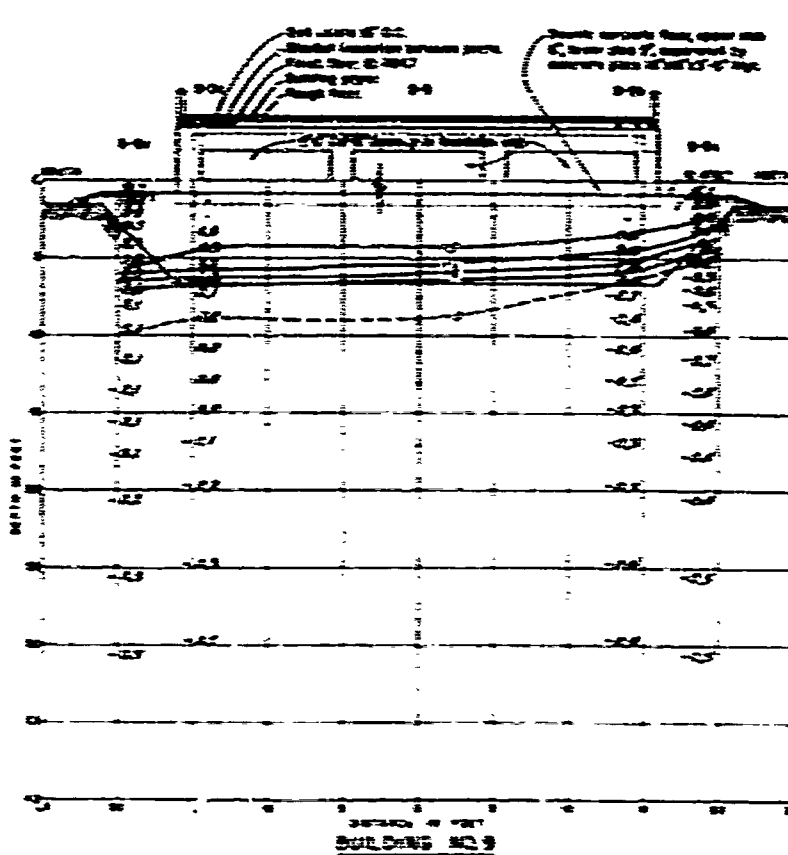
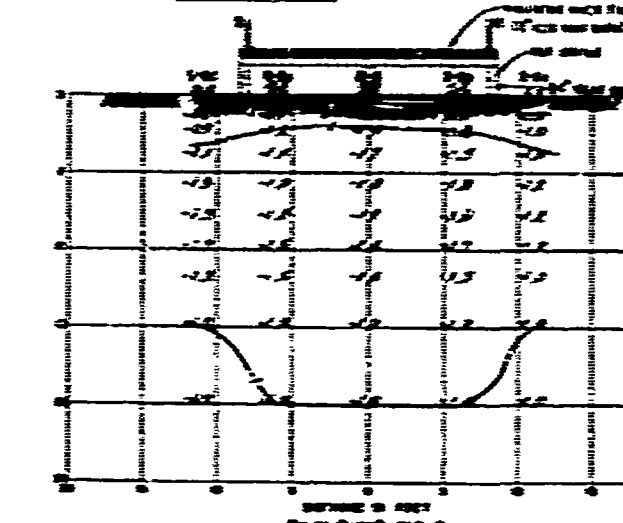
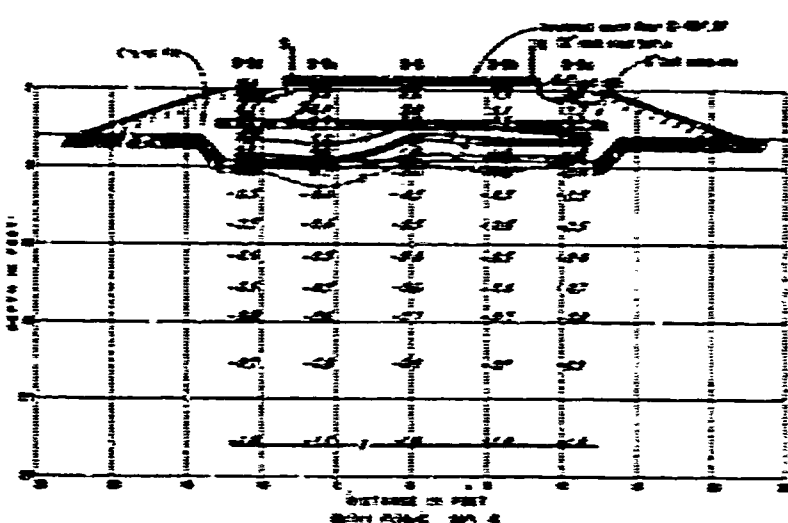
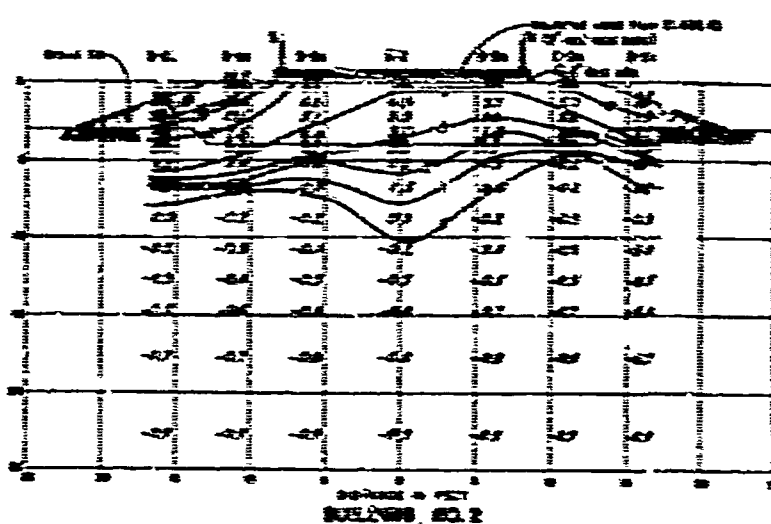
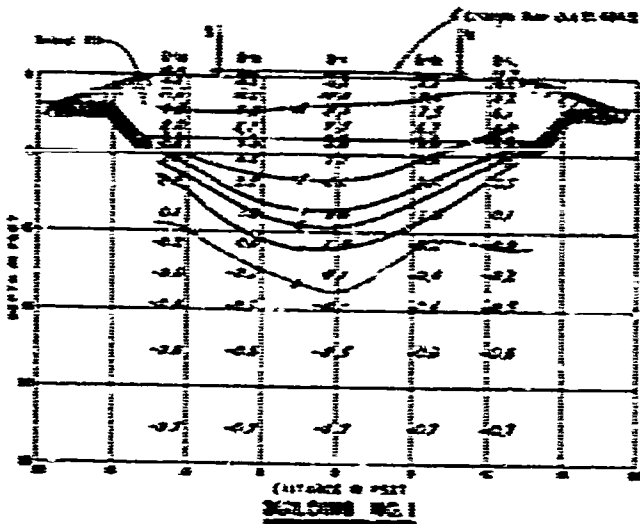
PAGE 03 BOMB
 COMPT 20 TWENTY 27 FIVE SEVEN MAY 1950
 2020 27 000 THREE 27 000 CLEVELAND OH



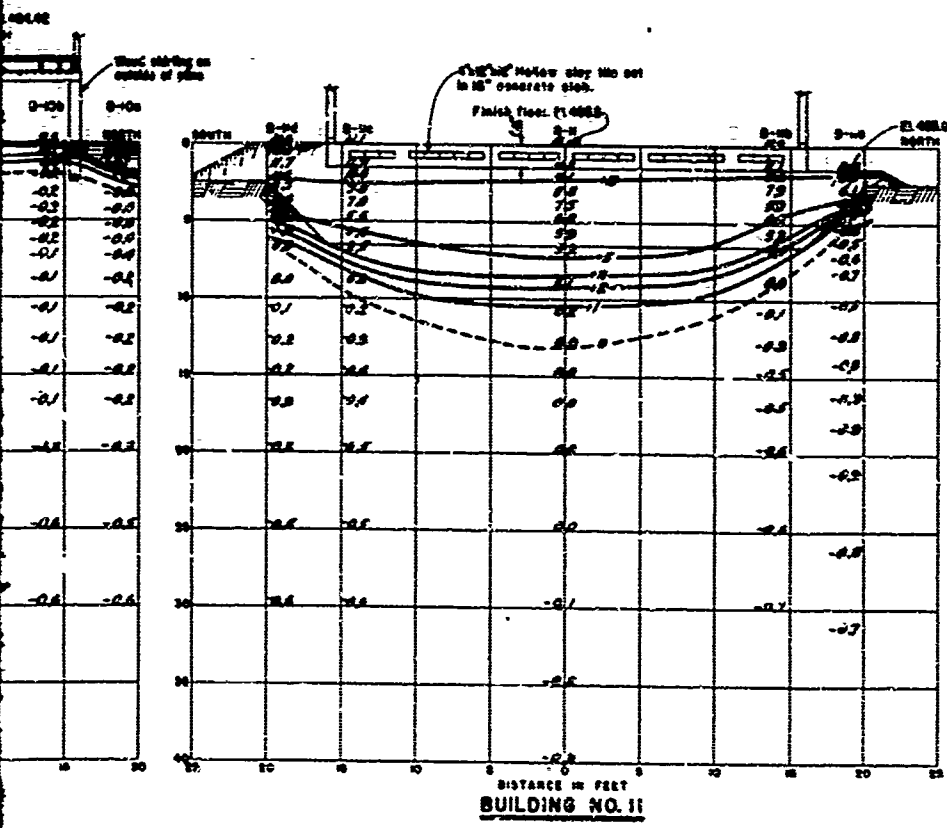
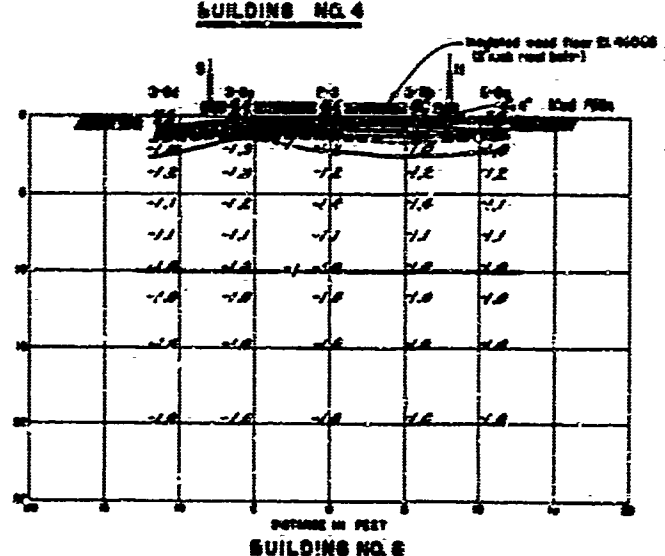
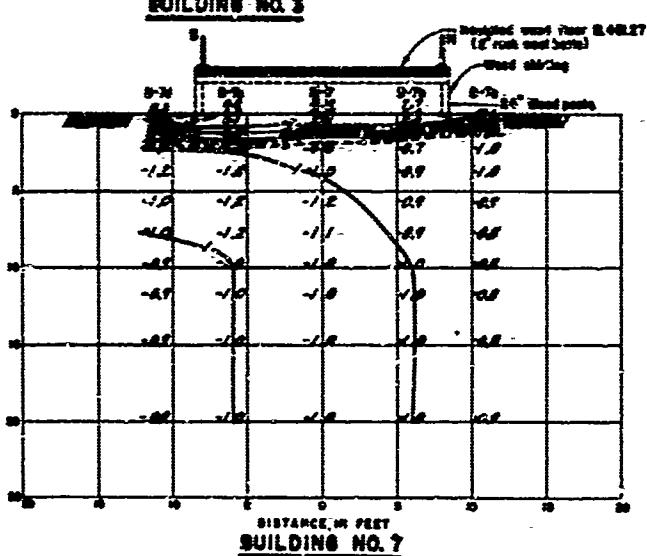
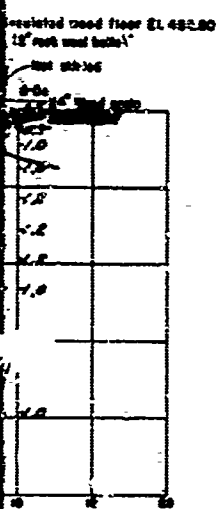
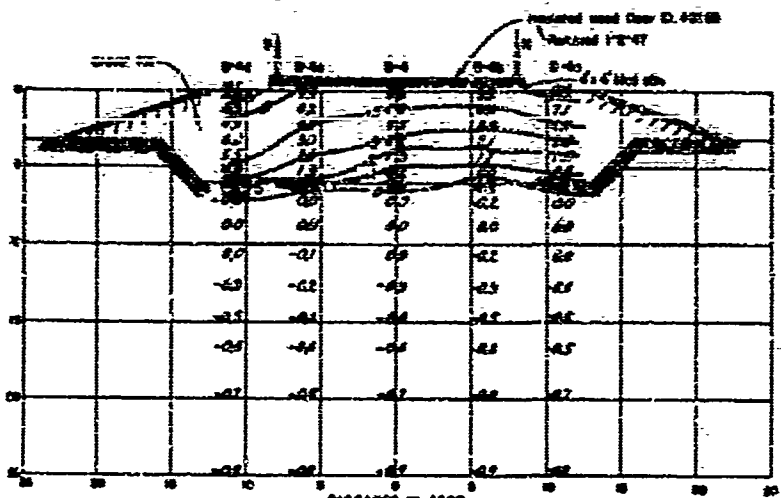
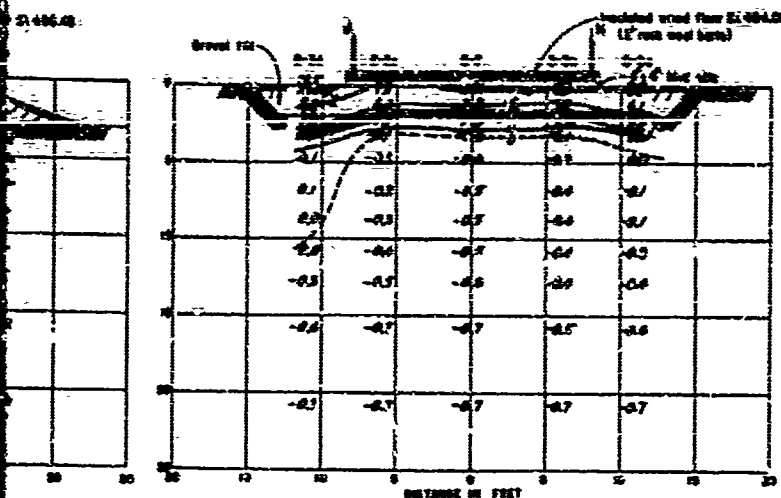
A



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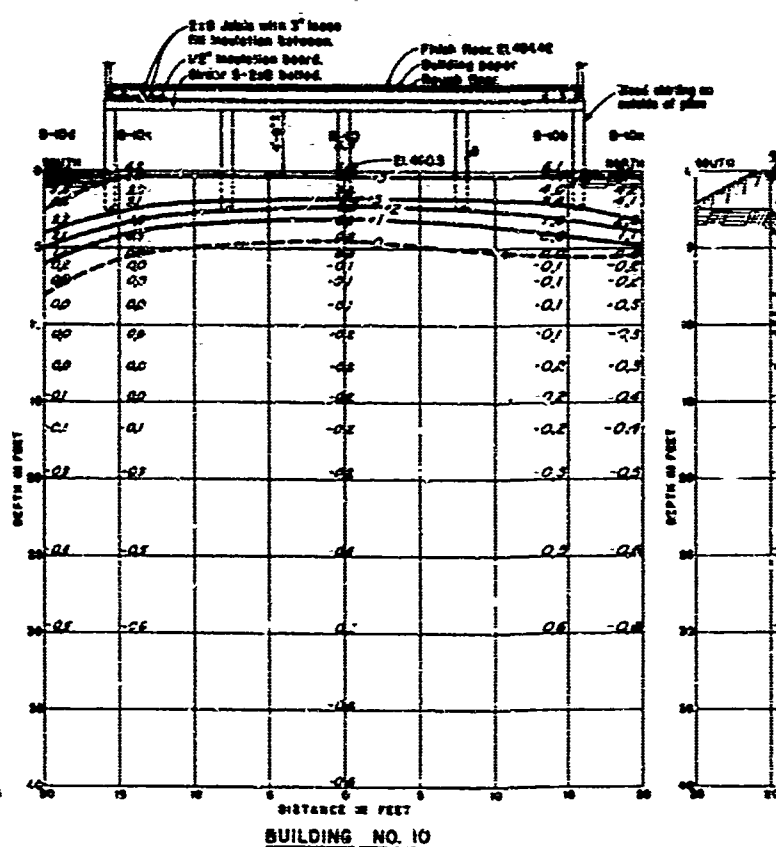
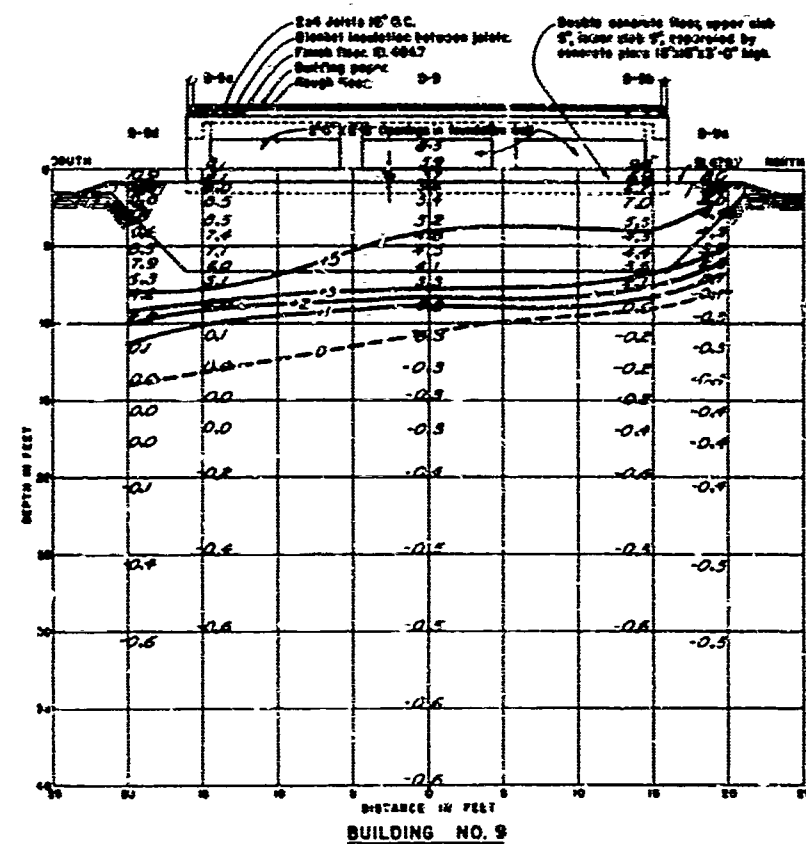
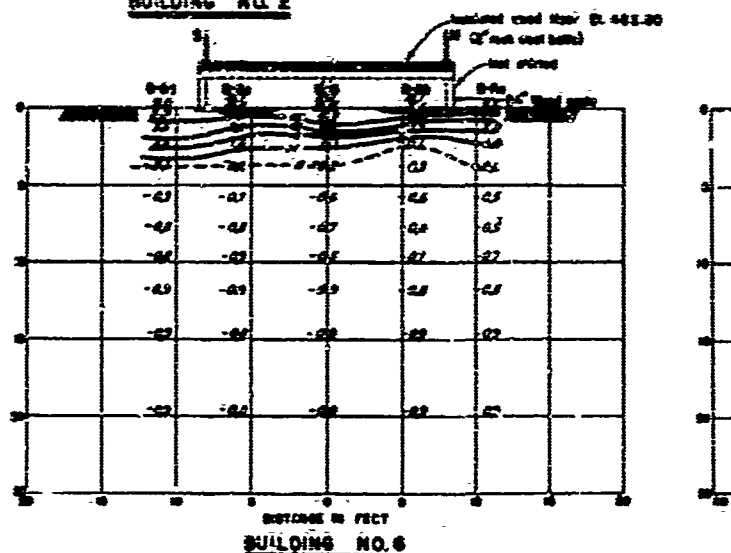
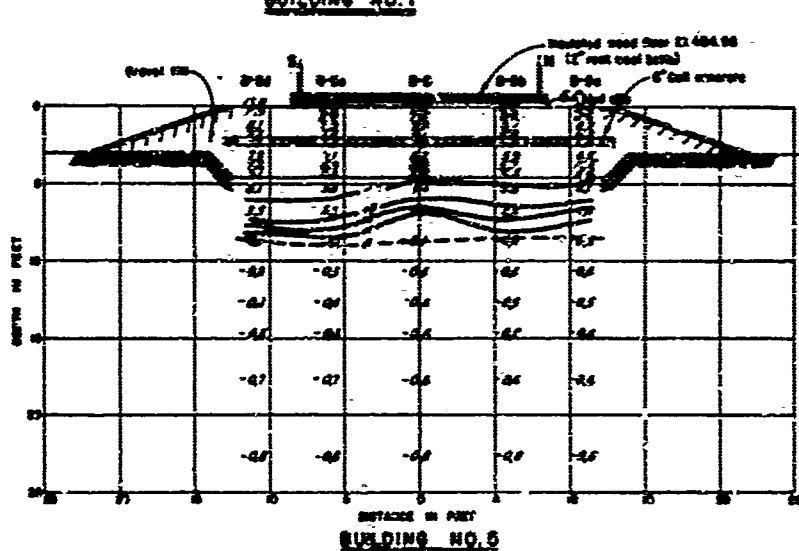
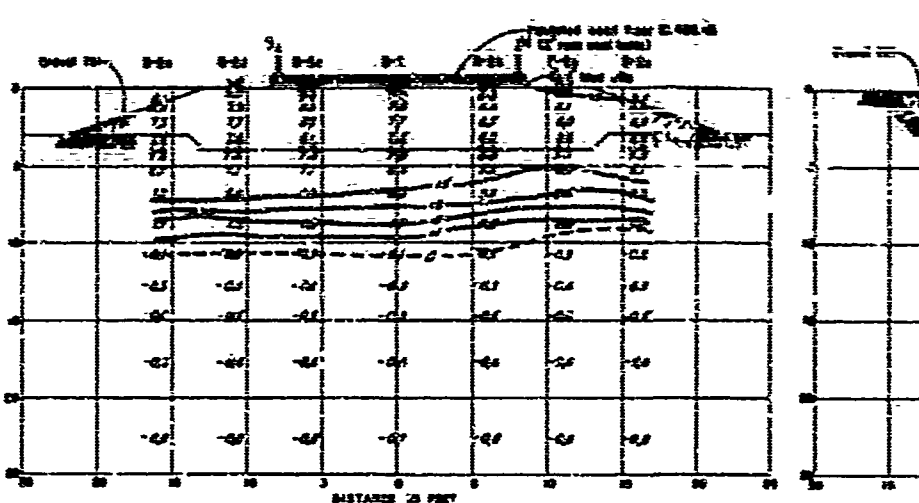
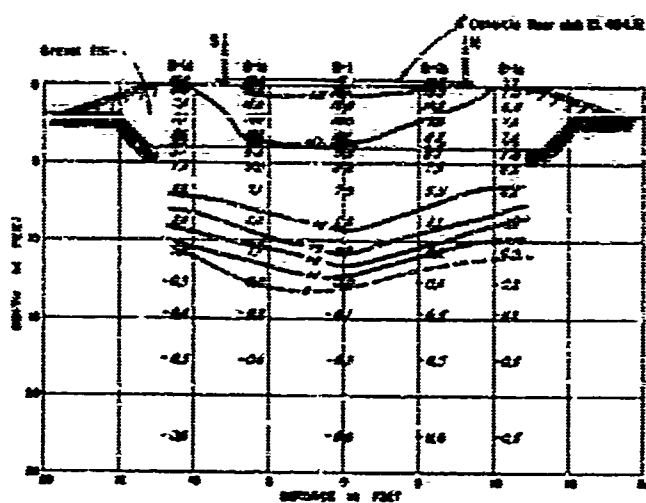


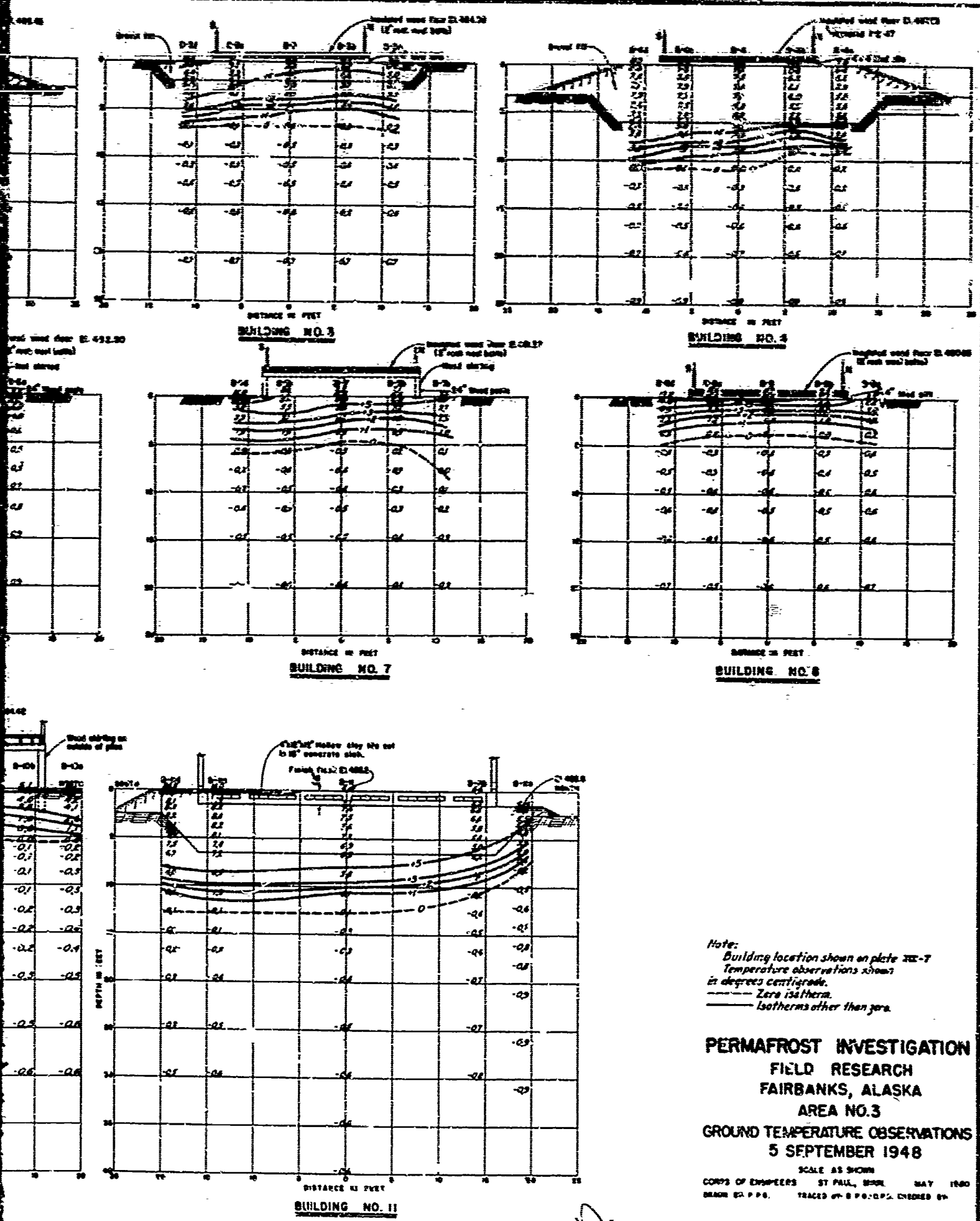
Note:
 Building location shown on plate III-47
 Temperature observations shown
 in degrees centigrade.
 — Zero isotherm
 — Isotherms other than zero.

PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
GROUND TEMPERATURE OBSERVATIONS
5 JUNE 1948

SCALE AS SHOWN
 CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1950
 DRAWN BY R. F. G. TRACED BY R. F. G. CHECKED BY

B

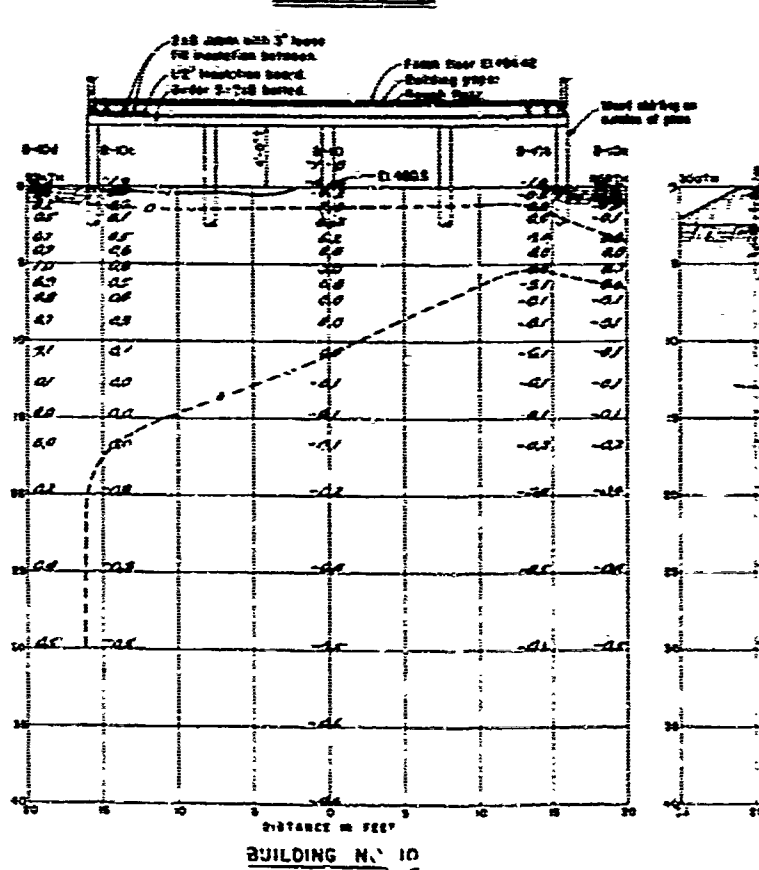
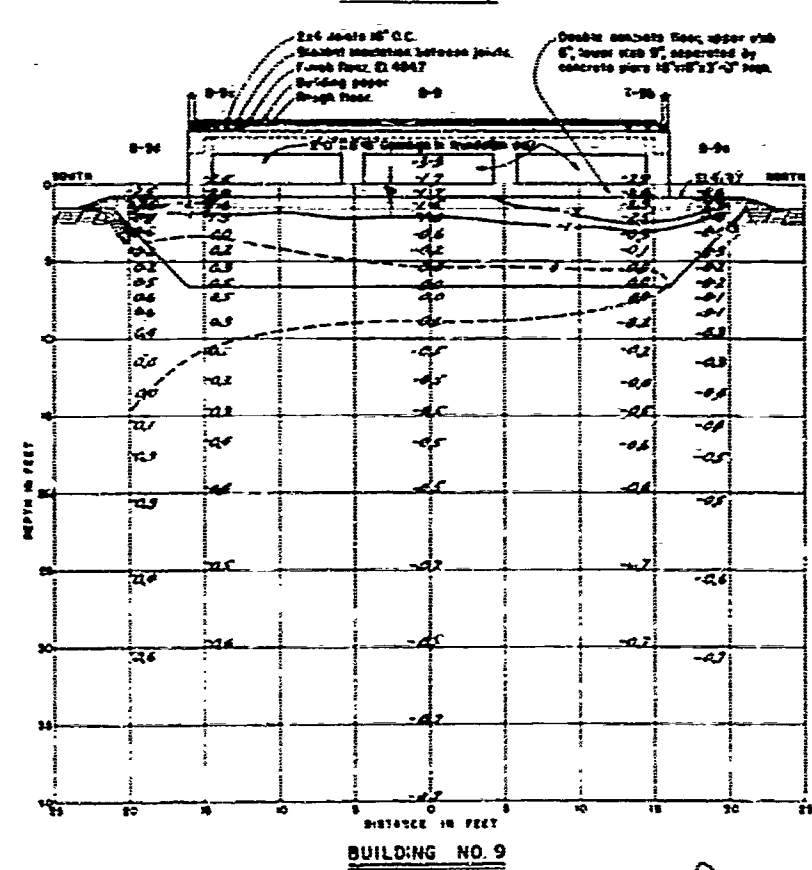
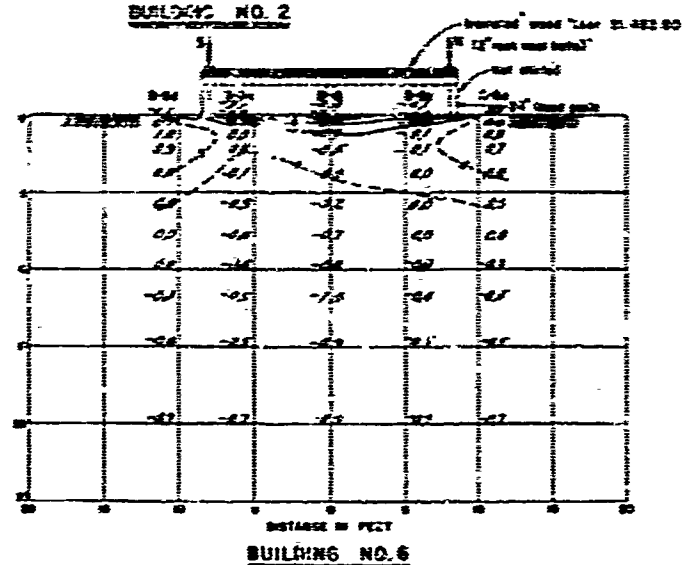
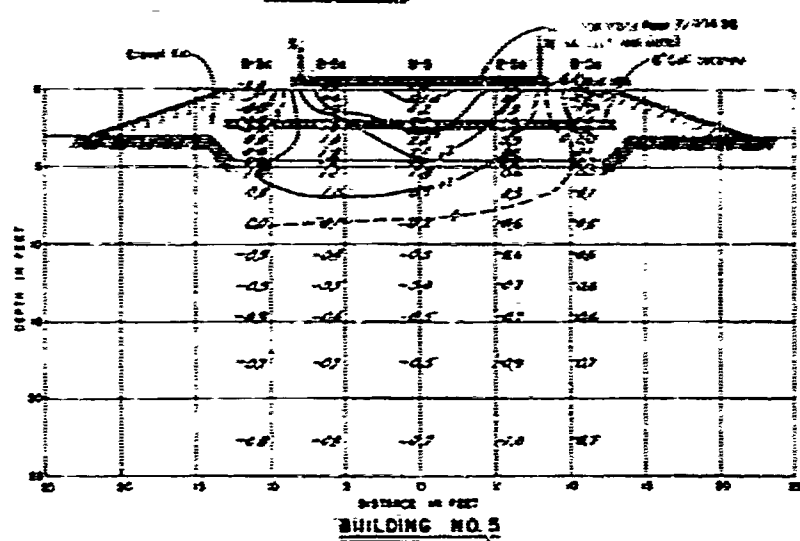
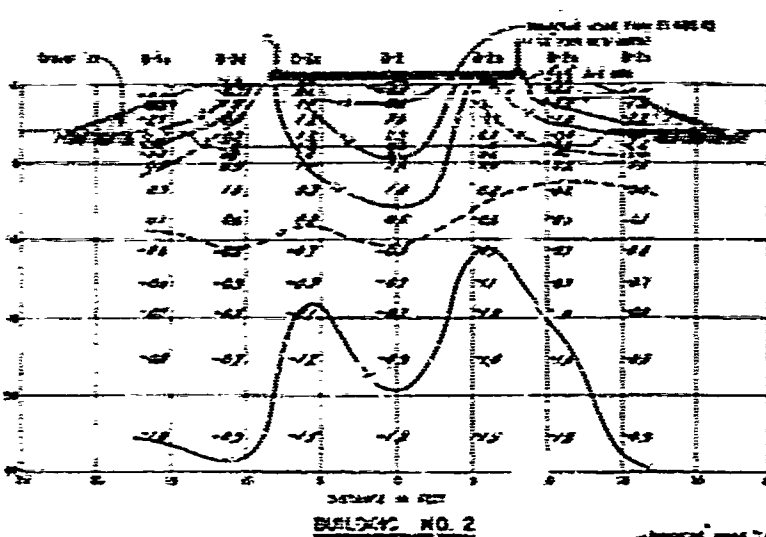
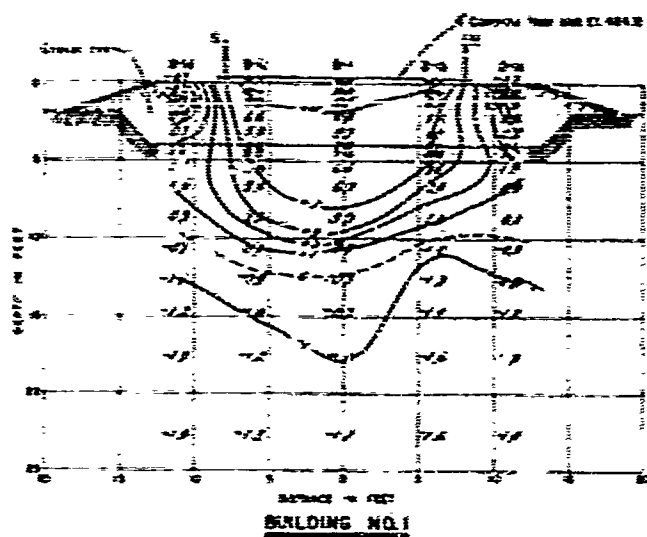




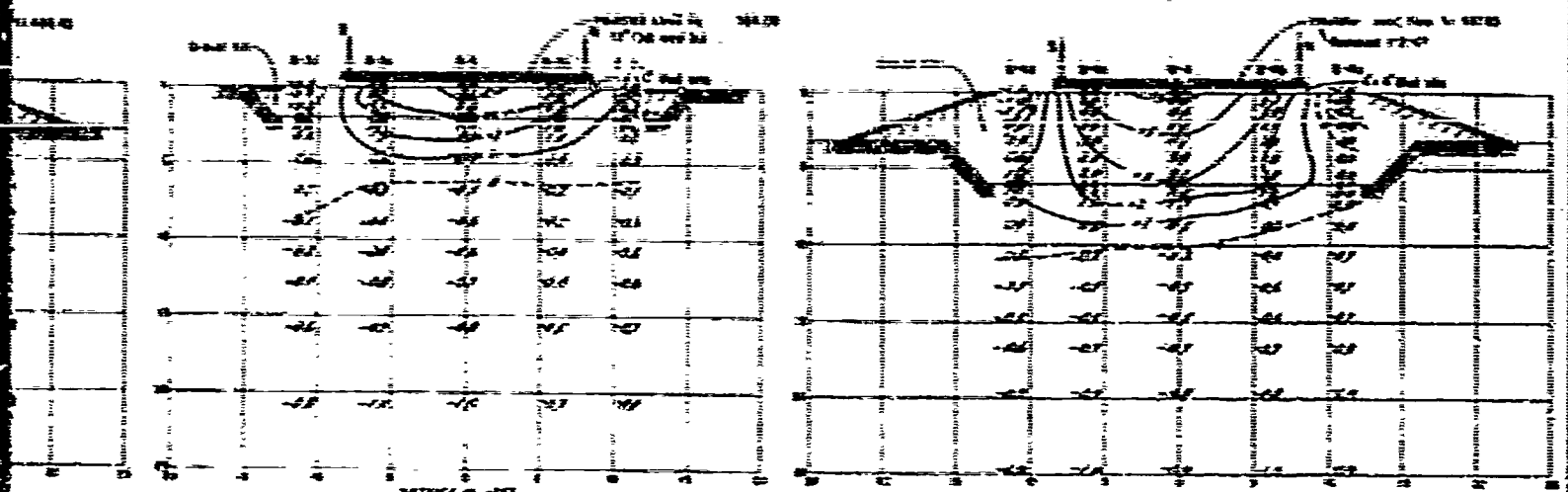
Note:
 Building location shown on plate XII-7
 Temperature observations shown
 in degrees centigrade.
 --- Zero isotherm.
 — Isotherms other than zero.

PERMAFROST INVESTIGATION
 FIELD RESEARCH
 FAIRBANKS, ALASKA
 AREA NO. 3
 GROUND TEMPERATURE OBSERVATIONS
 5 SEPTEMBER 1948
 SCALE AS SHOWN
 CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1950
 DRAWN BY P.P.B. TRACED BY P.P.B. CHECKED BY

A

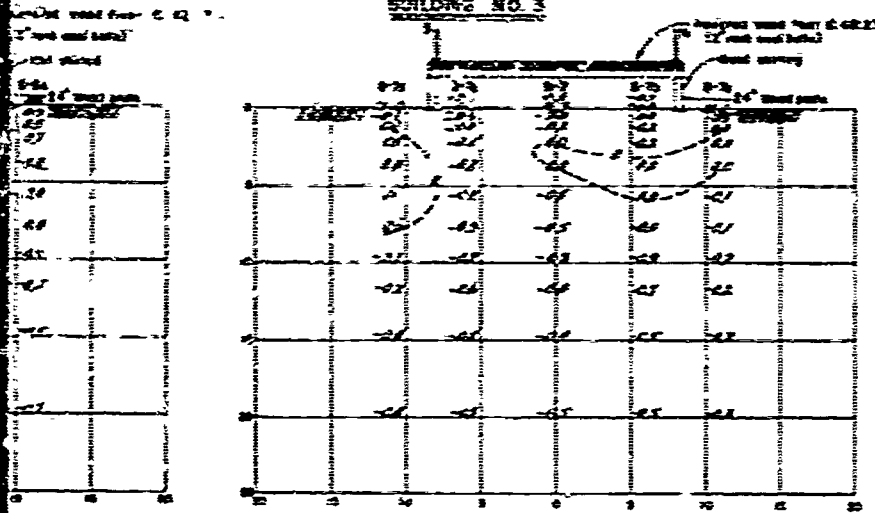


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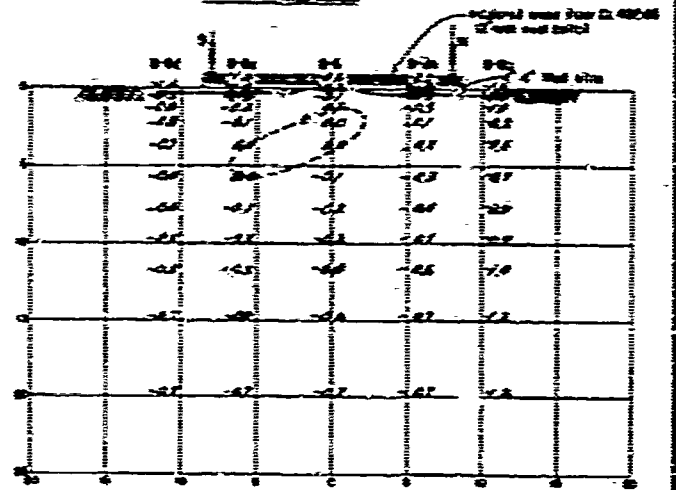


BUILDING NO. 3

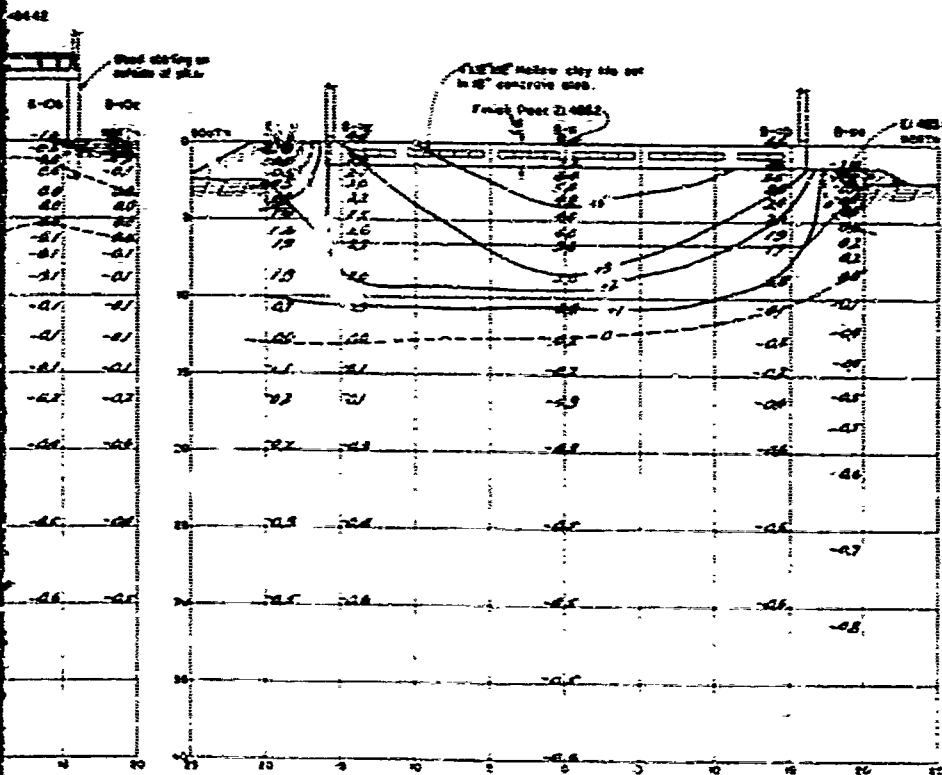
BUILDING NO. 4



BUILDING NO. 7



BUILDING NO. 8



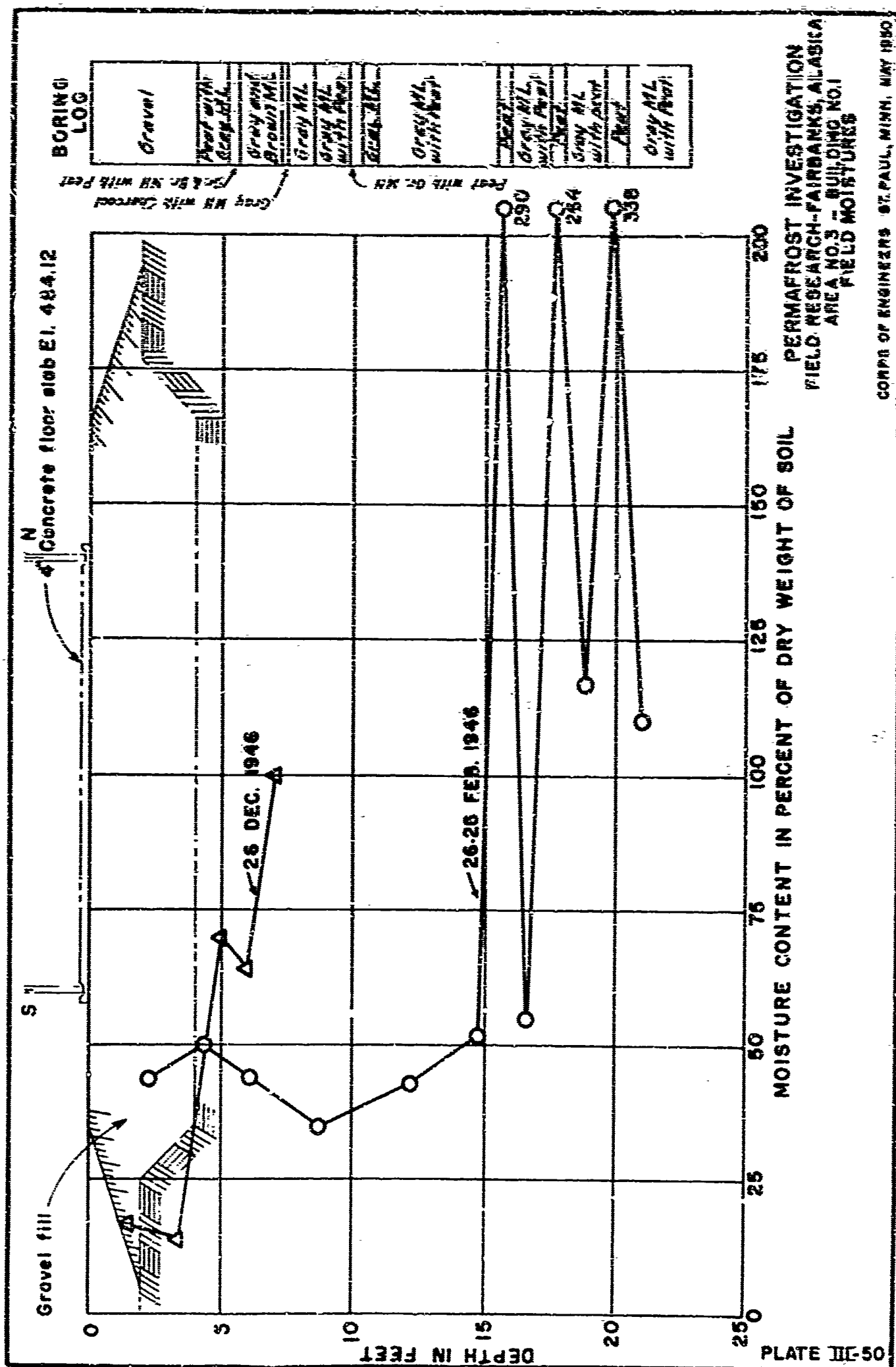
BUILDING NO. 11

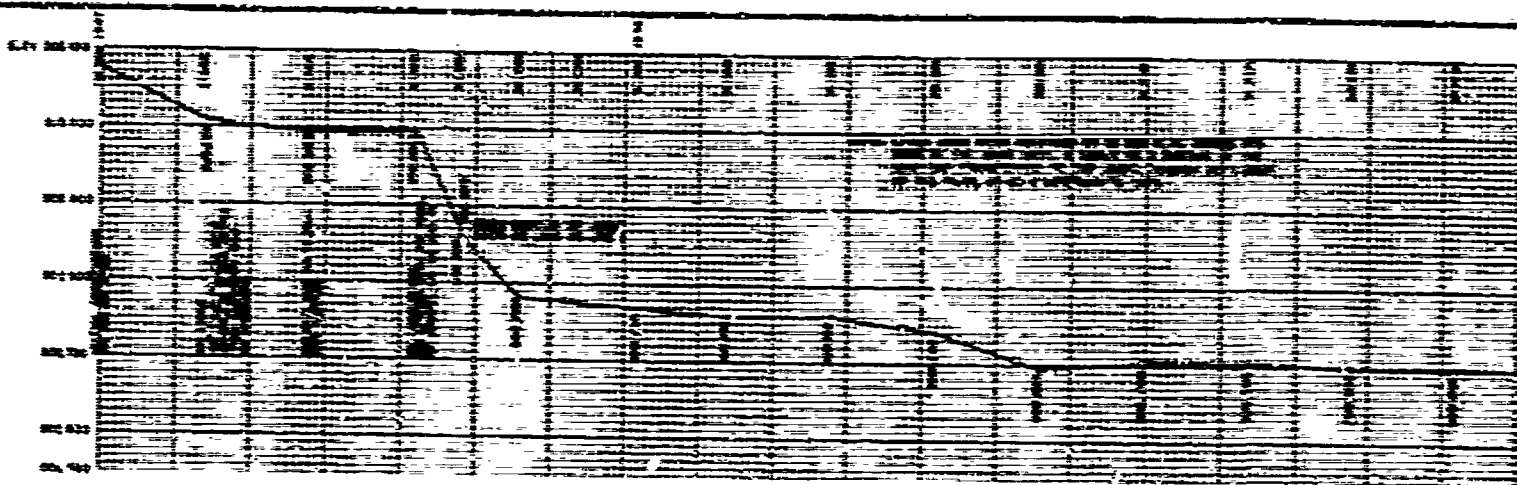
Note:
 Building locations shown on plate III-7
 Temperature observations shown
 in degrees centigrade.
 — Zero isotherm
 — Isotherms other than zero.

PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
GROUND TEMPERATURE OBSERVATIONS
30 OCTOBER 1948

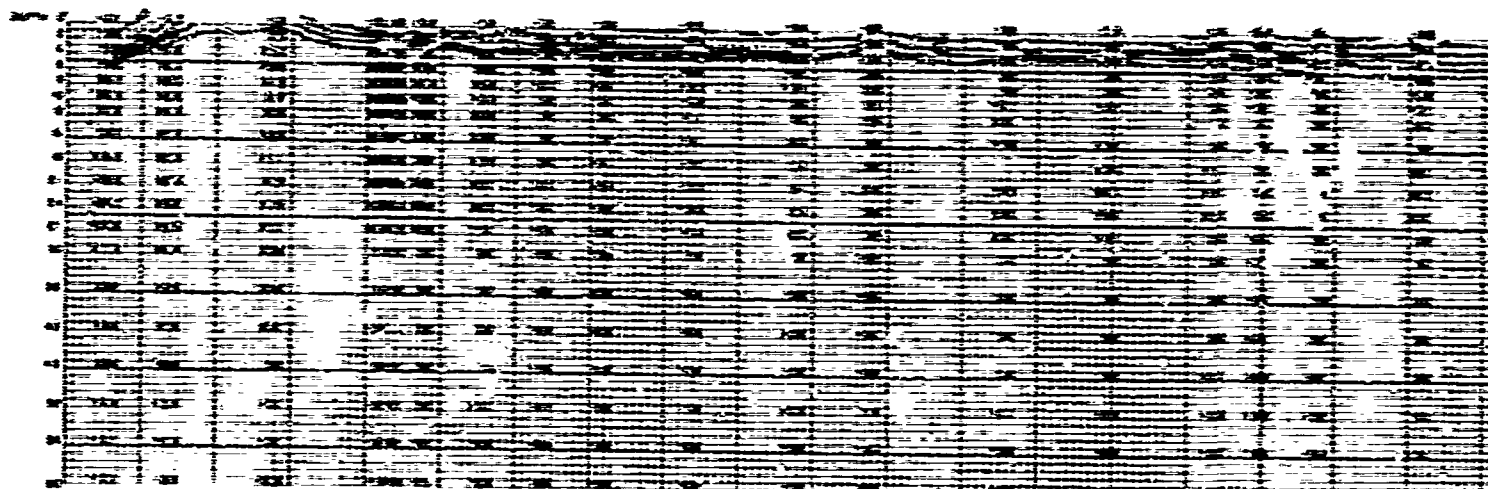
SCALE AS SHOWN
 CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1950
 DRAWN BY S.P.L. TRACED BY S.P.L. CHECKED BY

B

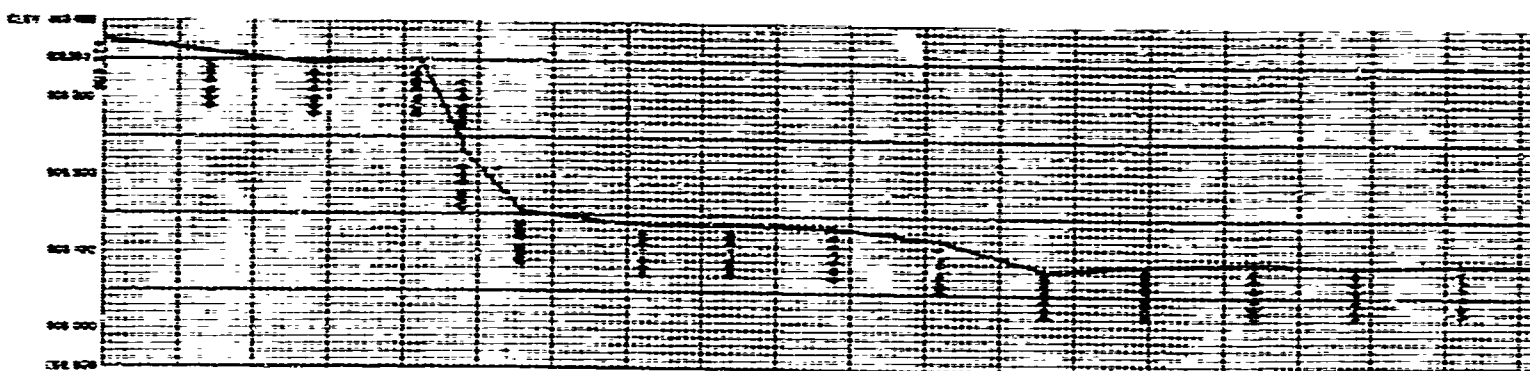




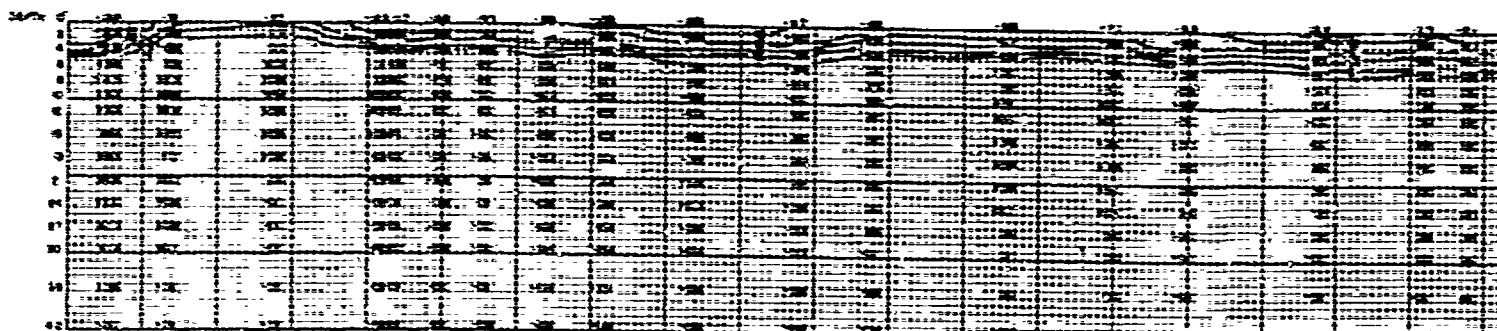
37' PILE-VERTICAL MOVEMENT



37' PILE-GROUND TEMPERATURES

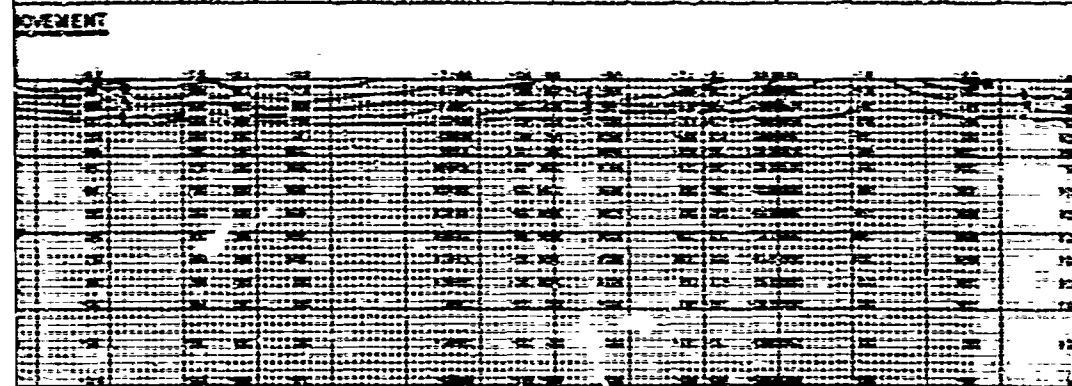
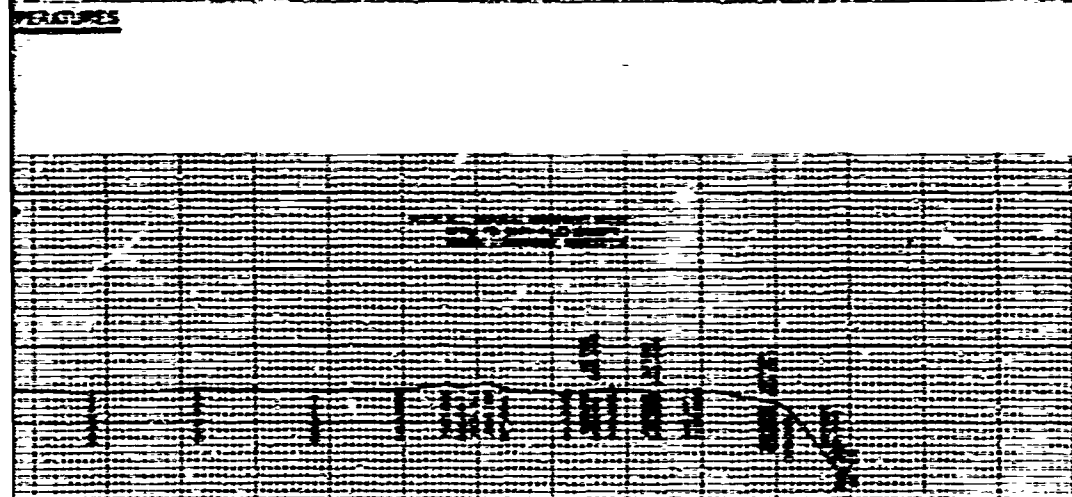
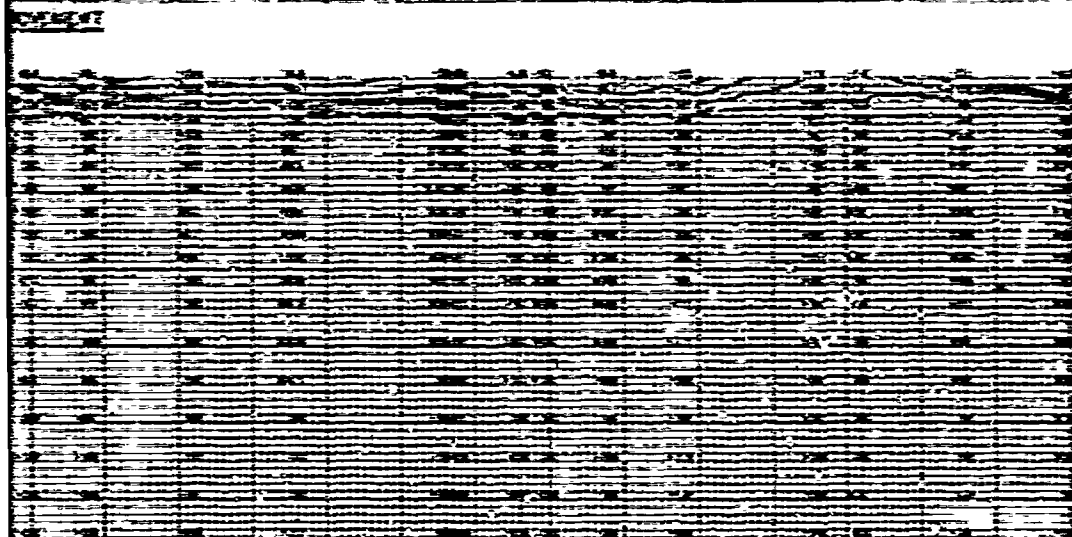


40' PILE-VERTICAL MOVEMENT



40' PILE-GROUND TEMPERATURES

A



TEMPERATURES

PERMAFROST INVESTIGATION
FIELD RESEARCH
FAIRBANKS, ALASKA
AREA NO. 3
MONOTUBE TEST PILES
VERTICAL MOVEMENT AND
GROUND TEMPERATURE CHART

SCALE AS SHOWN
CORPS OF ENGINEERS ST. PAUL, MINN. MAY 1950
DRAWN BY P.P.B. CHECKED BY P.P.C.

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